

STRUCTURAL NOTES

BASIS OF DESIGN
IBC 2012
ASCE 7-10

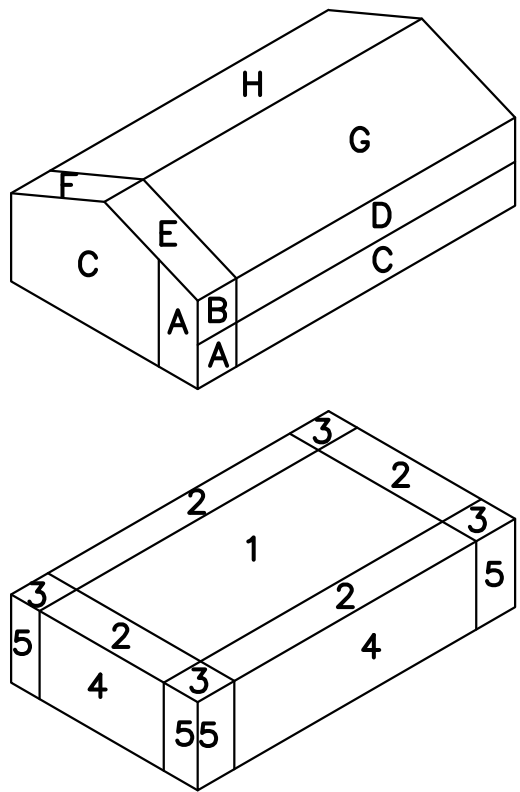
DESIGN LOADS

- DL 1 LIVE LOADS
a.FIRST FLOOR 100 psf
b.STORAGE 125 psf
c.MECHANICAL ROOMS
EQUIPMENT WEIGHT NOT LESS THAN 125 psf
- DL 2 ROOF 20 psf
- DL 3 GROUND SNOW LOAD 10 psf
a.SNOW EXPOSURE FACTOR Ce 0.9
b.THERMAL FACTOR Ct 1.0
c.SNOW IMPORTANCE FACTOR I 1.0
d.FLAT ROOF SNOW LOAD 0.7*pg*Ce*Ct*I 6.3 psf

- DL 4 WIND LOAD
a.WIND SPEED 90 mph
b.WIND SPEED 115 mph ULTIMATE
c.WIND IMPORTANCE FACTOR 1.00
d.RISK CATEGORY III
e.WIND EXPOSURE B
f.INTERNAL PRESSURE COEFFICIENT-ASD
ZONE A 16.8 psf
ZONE B -10.0 psf
ZONE C 11.0 psf
ZONE D -10.0 psf
ZONE E -17.7 psf
ZONE F -10.8 psf
ZONE G -12.3 psf
ZONE H -10.0 psf
OVERHANG
ZONE Eoh -24.8 psf Goh -19.4 psf
PARAPET LOAD -19.0 psf
g.COMPONENTS AND CLADDING - ASD
ZONE 1 10.0 psf -16.8 psf
ZONE 2 10.0 psf -28.1 psf
ZONE 3 10.0 psf -42.3 psf
ZONE 4 16.8 psf -18.2 psf
ZONE 5 16.8 psf -22.4 psf
ROOF OVERHANG ZONE 2 -24.1 psf
ZONE 3 -39.8 psf
- h.WIND BASE SHEAR
Vxult= 26 kips Vyult =42 kips
Vxasd=16 kips Vyasd =26 kips

- DL 5 SEISMIC DESIGN DATA
a.SEISMIC IMPORTANCE FACTOR 1.00
b.MAPPED SPECTRAL RESPONSE ACCELERATION Ss=0.158
S1=0.87
c.SITE CLASS D
d.SPECTRAL RESPONSE COEFFICIENTS Sds=0.168
Sd1=0.140
C
e.SEISMIC DESIGN CATEGORY
f.BASIC SEISMIC FORCE RESISTING SYSTEM
ORDINARY STEEL MOMENT FRAME
g.DESIGN BASE SHEAR 16 KIPS
h.RESPONSE MODIFICATION FACTORS R 3.5
i. SEISMIC RESPONSE COEFFICIENT Cd 3.0
j. ANALYSIS PROCEDURE -- EQUIVALENT FORCE METHOD

- DL 6 DESIGN DEAD LOADS
a.2nd FLOOR DEAD LOAD
METAL FRAMING = 54 psf
5" DRYWALL CEILING = 2 psf
MECHANICAL DUCTS & PIPING = 4 psf
TOTAL DEAD LOAD = 60 psf
- b.ROOF DEAD LOAD
PE METAL BUILDING = 5 psf
SUSPENDED CEILING = 2 psf
MECHANICAL DUCTS & LIGHTING = 4 psf
6" INSULATION AVERAGE = 2 psf
TOTAL DEAD LOAD = 13 psf



FOUNDATIONS

- FD 1 SOIL BEARING PRESSURE..... 2,100 psf CONT FOOTINGS
2,400 psf COLUMN FOOTINGS

GEOTECHNICAL INVESTIGATION REPORT PREPARED BY:
GEOTECHNICAL ASSOCIATES NETWORK, LLC
110 BEECHTREE ROAD
VICKSBURG, MS 39183-7464
PHONE: 601-362-5421

- FD 2 THE CONTRACTOR SHALL REFER TO THE GEOTECHNICAL INVESTIGATION REPORT FOR RECOMMENDATIONS FOR SUB-GRADE BUILDING PAD AND FOUNDATION BEARING PREPARATIONS. CONTRACTOR SHALL FOLLOW REPORTS RECOMMENDATIONS FOR BUILDING PAD AND OPEN FOUNDATION PROTECTION FROM WEATHER. EXTERIOR AND PERIMETER BUILDING FOUNDATION SHALL NOT BEAR ABOVE THE LOCAL FROST LINE. REFER TO ARCHITECTURAL DRAWINGS FOR PERIMETER SLAB/FOUNDATION INSULATION REQUIREMENTS.

- FD 3 BACK FILL

CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO BRACE WALLS DURING BACKFILLING. CARE SHALL BE TAKEN DURING PLACEMENT OF BACKFILL ALONG THE WALL SO AS TO NOT OVERLOAD THE WALL DUE TO HEAVY EQUIPMENT. ONLY LIGHTWEIGHT (A MAXIMUM OF ONE TON TOTAL WEIGHT) EQUIPMENT SHALL BE PERMITTED WITHIN THE CRITICAL ZONE DEFINED AS BEGINNING AT THE BASE OF THE WALL ON A 1:1 SLOPE.

STRUCTURAL CONCRETE

- CO 1 CONCRETE SHALL BE IN ACCORDANCE WITH ACI MANUAL 315 AND STANDARD 318. CONCRETE SHALL BE OF REGULAR AGGREGATE AND SHALL HAVE DESIGN COMPRESSIVE STRESS AT 28 DAYS AS FOLLOWS:

- a.f'c = 3,000 psi FOR FOUNDATIONS AND FOUNDATION WALLS.
b.f'c = 3,000 psi FOR SLAB ON GRADE
c.f'c = 3,500 psi AIR ENTRAINED FOR EXTERIOR CONCRETE
d.f'c = 4,000 psi FOR ELEVATED SLABS

- CO 2 PROVIDE AIR ENTRAINED CONCRETE FOR CONCRETE EXPOSED TO WEATHER.

- CO 3 SLAB ON GRADE
a.PROVIDE A 5" CONCRETE SLAB WITH #3@12" o.c. EACH WAY OVER 8 MIL POLY VAPOR RETARDER OVER 4" OF DRAINAGE FILL.
b.DRAINAGE FILL SHALL BE WASHED SAND, WASHED #57 STONE, SCREENINGS, CRUSHER RUN OR OTHER MATERIAL APPROVED BY THE SOIL ENGINEER.

- CO 4 REFER TO ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS FOR ALL MOLDS, GROOVES, ETC. AND FOR LOCATIONS OF SLEEVED AND INSERTS TO BE CAST IN CONCRETE SLABS AND FLOORS.

- CO 5 FOR SIZE, NUMBER AND LOCATIONS OF ALL SLAB OPENINGS AND MECHANICAL HOUSEKEEPING PADS SEE ARCHITECTURAL AND MECHANICAL DRAWINGS. PROVIDE MECHANICAL HOUSEKEEPING PADS AS REQUIRED AND REINFORCE WITH #4@12" EACH WAY U.N.O. DOWEL PADS INTO SUPPORTING SLAB.

- CO 6 REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60. DETAILING SHALL BE IN ACCORDANCE WITH ACI MANUAL 315 AND STANDARD 318. LAP SPLICES IN CONCRETE SHALL BE IN ACCORDANCE WITH CHAPTER 12 OF ACI 318-08. REINFORCING STEEL SHALL HAVE A MINIMUM CONCRETE COVER AS TABULATED BELOW UNLESS OTHERWISE NOTED:

- WALLS 1 1/2"
FOUNDATIONS 2" FOR FORMED CONCRETE
3" WHERE CONCRETE IS CAST AGAINST GROUND

BAR SIZE	MINIMUM SPLICE & EMBEDMENT LENGTHS		
	MIN EMBEDMENT, (IN)		
	MIN LAP (IN)	STRAIGHT	STD HOOK
	OTHER-TOP	OTHER-TOP	ALL BARS
#3	12 - 14	12 - 14	6
#4	15 - 19	15 - 19	7
#5	18 - 24	18 - 23	9
#6	22 - 28	22 - 28	10
#7	25 - 33	25 - 33	12
#8	29 - 37	29 - 37	14

STRUCTURAL STEEL

- ST 1 STRUCTURAL STEEL SHALL BE IN ACCORDANCE AISC 360-10:

- a.ALL STRUCTURAL A992
b.MISC STEEL, CHANNELS, ANGLES, PLATES A36
c.STEEL TUBING ASTM A500 GRADE B
d.SHOP AND FIELD WELDS E70XX ELECTRODES
e.BOLTS (3/4" DIAMETER MIN) A325 HIGH STRENGTH
f. ANCHOR BOLTS F1554 GR. 36
g.COLUMN BASE PLATES 3/4" HOOKED UNO
5" PROJECTION + 9" EMBED + 3" HOOK

- ST 2 CONTRACTOR MAY USE EXPANSION BOLTS OR EPOXY BOLTS IN LIEU OF ANCHOR BOLTS FOR WOOD SILL PLATES. USE 5/8" EXPANSION BOLTS w/ 4" EMBED w/ 1/4"x3"x3" PLATE WASHER AT SAME SPACING.

- ST 3 CONTRACTOR MAY USE EPOXY ALL THREAD BOLTS FOR COLUMN BASE PLATE ANCHOR BOLTS. EMBEDMENT TO BE 9".

- ST 4 GROUT UNDER BASE AND BEARING PLATES SHALL BE NON-SHRINK, NON-METALLIC.

- ST 5 ANCHOR BOLTS, BASE PLATES AND COLUMNS SHALL BE PROTECTED FROM DIRECT CONTRACT w/ THE GROUND. COAT HEAVILY AND VOID FREE WITH ASPHALTIC MASTIC.

- ST 6 ALL WELDING SHALL BE IN ACCORDANCE WITH THE AMERICAN WELDING SOCIETY (AWS) BY CERTIFIED WELDERS.

- ST 7 HOLES SHALL NOT BE CUT THROUGH BEAMS UNLESS INDICATED OR APPROVED BY THE ENGINEER.

- ST 8 PRIME STRUCTURAL AND MISC. STEEL WITH MANUFACTURES STANDARD IRON OXIDE PRIMER -- COLOR GRAY. PRIMER SHALL BE COMPATIBLE WITH FINISH COAT OF PAINT WHEN PROVIDED.

- ST 9 STRUCTURAL STEEL DETAILING, FABRICATION, AND ERECTION SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE AISC "MANUAL OF STEEL CONSTRUCTION".

- ST10 HSS TUBE COLUMNS SHALL HAVE ONE-EIGHTH INCH CAP PLATES.

COMPOSITE METAL DECK

- CD 1 COMPOSITE DECK SHALL BE IN ACCORDANCE WITH THE STEEL DECK INSTITUTE ANSI/SDI-C1.0 STANDARD FOR COMPOSITE STEEL DECK.

- CD 2 COMPOSITE DECK SHALL BE FABRICATED USING 33 KSI OR 50 KSI STEEL AS DETERMINED BY THE SUPPLIER BASED ON THE SPAN AND LOADING CONDITIONS.

- CD 3 DECK SHALL HAVE A G60 GALVANIZED FINISH.

- CD 4 COMPOSITE DECK TYPE SHALL BE AS INDICATED ON THE DRAWINGS AND REINFORCED WITH 6x6 W2.0xW2.0 WELDED WIRE MESH U.N.O.

- CD 5 ATTACHMENT OF METAL DECK TO WALLS AND STEEL SHALL BE AS RECOMMENDED BY THE DECK MANUFACTURER. MINIMUM BEARING 2" ON STEEL

- CD 6 14 GAGE SDI POUR STOPS SHALL BE PROVIDED AT ALL DECK EDGES.

- CD 7 COMPOSITE DECK IS TO BE SHORED DURING CONCRETE PLACEMENT. SHORING SHALL BE DETERMINED BY THE CONTRACTOR.

LOAD BEARING METAL STUDS

- MS 1 GENERAL CONTRACTOR TO PROVIDE A LEVEL BEARING SURFACE FOR BOTTOM TRACK OF LOAD-BEARING METAL STUDS AT EVER FLOOR. METAL STUDS TO HAVE SOLID BEARING ON SUPPORTING SURFACES.

- MS 2 ALL COLD-FORMED STEEL MEMBERS (STUDS, TRACKS & ACCESSORIES) SHALL BE MANUFACTURED BY SSMA CERTIFIED SUPPLIERS. ALL COMPONENTS SHALL COMPLY WITH THE LATEST EDITION OF AISI (AMERICAN IRON AND STEEL INSTITUTE SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS.

- MS 3 STUDS, TRACKS AND ACCESSORIES SHALL BE ASTM A525 G90 GALVANIZED MINIMUM.

- MS 4 FABRICATOR MAY USE SCREWS OR WELDS TO ATTACH MATERIALS PROVIDED WELDER IS ANSI/AWI D1.3-98 CERTIFIED. WELDED MATERIAL SHALL BE EQUAL TO OR GREATER THAN 18 GAGE. ALL WELDING SHALL BE TOUCHED UP WITH ZINC RICH PAINT.

- MS 5 WALLS SHALL BE CONSTRUCTED SUCH THAT THE WALL STUDS WILL HAVE UNIFORM BEARING ON THE TOP AND BOTTOM TRACK.

- MS 6 WALL STUDS SHALL VERTICALLY ALIGN FROM FLOOR TO FLOOR.

- MS 7 TRACKS GAGE SHALL MATCH WALL STUDS BUT NOT LESS THAN 54x50.
MS 8 UNLESS NOTED OTHERWISE TRACK SHALL BE ATTACHED TO FLOORS AS FOLLOWS:

- a.EXTERIOR WALLS: 0.117 POWDER ACTUATED FASTENERS (PAF) AT 8" o.c. WITH 1 1/2" EMBED.

- b.INTERIOR WALLS: 0.117 PAF AT 16" o.c. WITH 1 1/2" EMBED

- MS 9 CUTTING OF LOAD-BEARING STUDS OR TRACK IS NOT PERMITTED WITHOUT SPECIFIC WRITTEN PERMISSION FROM THE MANUFACTURER.

- MS 10 CUTTING OF HEADERS IS NOT PERMITTED.

- MS 11 FOR WALLS GREATER THAN 12FT TALL 4"x14 GAGE FLAT STRAPS SHALL BE ADDED TO THE EACH FACE OF THE WALL AT THE BRIDGING LOCATIONS

SUBMITTALS

- SB 1 SUBMITTALS SHALL BE PROVIDED THROUGH THE ARCHITECT IN ACCORDANCE WITH THE GENERAL CONDITIONS.

- SB 2 REVIEW IS ONLY FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT AND THE INFORMATION GIVEN IN THE CONSTRUCTION DOCUMENTS. CORRECTIONS OR COMMENTS MADE ON THE SHOP DRAWINGS DURING THE REVIEW DO NOT RELIEVE THE GENERAL CONTRACTOR FROM COMPLIANCE WITH THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. REVIEW OF A SPECIFIC ITEM SHALL NOT INCLUDE REVIEW OF AN ASSEMBLY OF WHICH THE ITEM IS A COMPONENT. THE CONTRACTOR IS RESPONSIBLE FOR DIMENSIONS TO BE CONFIRMED AND CORRELATED AT THE JOBSITE; INFORMATION THAT PERTAINS SOLELY TO THE FABRICATION PROCESSES AND PROCEDURES OF THE CONSTRUCTION; COORDINATION OF THE WORK WITH THAT OF ALL OTHER TRADES AND PERFORMING ALL WORK IN A SAFE AND SATISFACTORY MANNER.

- SB 3 REVIEW OF SHOP DRAWINGS DOES NOT WARRANT OR REPRESENT THAT THE INFORMATION WITHIN THE SUBMITTAL IS EITHER ACCURATE OR COMPLETE. SOLE RESPONSIBILITY FOR CORRECT DESIGN, DETAILS, DIMENSIONS AND QUANTITIES SHALL REMAIN WITH THE PARTY PROVIDING THE SUBMITTAL AND THE GENERAL CONTRACTOR.

- SB 4 CHANGES TO THE CONTRACT DOCUMENTS MUST BE DOCUMENTED SEPARATELY FROM SHOP DRAWINGS.

- SB 5 REQUIRED SUBMITTALS - (SUBMITTALS OTHER THAN THOSE LISTED SHALL NOT BE REVIEWED):

- SB 6 SUBMIT FOR REVIEW:
a.CONCRETE DESIGN MIX - EACH TYPE
b.STRUCTURAL STEEL ERECTION DRAWINGS
c.LOAD BEARING METAL STUD WALLS/FRAMING LAYOUT

DRAWING INDEX

- S001 STRUCTURAL NOTES
S002 STRUCTURAL SPECIAL INSPECTIONS
S101 DIMENSION PLAN
S102 FOUNDATION PLAN
S201 SECOND FLOOR FRAMING PLAN
S202 ROOF FRAMING PLAN
S203 CANOPY DIMENSION, FOUNDATION & ROOF FRAMING PLAN
S301 FOUNDATION SECTIONS
S401 METAL STUD DETAILS
S501 FRAMING SECTIONS

NO.	DATE	REVISION
1	08/14/17	2ND FLOOR FRAMING
2	12/06/17	REV FOR EXISTING CONDITIONS

SCHEDULE OF STRUCTURAL SPECIAL INSPECTIONS

REQUIRED VERIFICATION AND INSPECTION OF SOILS

VERIFICATION AND INSPECTION TASK	CONTINUOUS DURING TASK LISTED	PERIODICALLY DURING TASK LISTED
1. Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	-	X
2. Verify excavations are extended to proper depth and have reached proper material.	-	X
3. Perform classification and testing of compacted fill materials.	-	X
4. Verify use of proper materials, densities and lift thicknesses during placement and compaction of compacted fill.	X	-
5. Prior to placement of compacted fill, observe subgrade and verify that site has been prepared properly.	-	X

REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD
1. Inspection of reinforcing steel, including prestressing tendons, and placement.	-	X	ACI 318: 3.5, 7.1-7.7
2. Inspection of reinforcing steel welding in accordance with Table 1704.3, Item 5b.	-	-	AWS D1.4 ACI 318: 3.5.2
3. Inspection of bolts to be installed in concrete prior to and during placement of concrete where allowable loads have been increased or where strength design is used.	X	-	ACI 318: 8.1.3, 21.2.8
4. Inspection of anchors installed in hardened concrete.	-	X	ACI 318: 3.8.6, 8.1.3, 21.2.8
5. Verifying use of required design mix.	-	X	ACI 318: Ch. 4, 5.2-5.4
6. At the time fresh concrete is sampled to fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of the concrete.	X	-	ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8
7. Inspection of concrete and shotcrete placement for proper application techniques.	X	-	ACI 318: 5.9, 5.10
8. Inspection for maintenance of specified curing temperature and techniques.	-	X	ACI 318: 5.11, 5.13
9. Inspection of prestressed concrete:			
a. Application of prestressing forces.	N/A	-	ACI 318: 18.20 ACI 318: 18.18.4
b. Grouting of bonded prestressing tendons in the seismic-force-resisting system.	N/A	-	
10. Erection of precast concrete members.	N/A	-	ACI 318: Ch. 16
11. Verification of in-situ concrete strength, prior to stressing of tendons in posttensioned concrete and prior to removal of shores and forms from beams and structural slabs.	N/A	-	ACI 318: 6.2
12. Inspect formwork for shape, location and dimensions of the concrete member being formed.	N/A	-	ACI 318: 6.1.1

REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD
1. Material verification of high–strength bolts, nuts and washers:			
a. Identification markings to conform to ASTM standards specified in the approved construction documents.	–	X	AISC 360, Section A3.3 and applicable ASTM material standards
b. Manufacturer’s certificate of compliance required.	–	X	–
2. Inspection of high–strength bolting:			
a. Snug–tight joints.	–	X	AISC 360, Section M2.5
b. Pretensioned and slip–critical joints using turn–of–nut with matchmaking, twist–off bolt or direct tension indicator methods of installation.	–	X	
c. Pretensioned and slip–critical joints using turn–of–nut without matchmarking or calibrated wrench methods of installation.	X	–	
3. Material verification of structural steel and cold formed steel deck:			
a. For structural steel, identification markings to conform to AISC 360.	–	X	AISC 360, Section M5.5
b. For other steel, identification markings to conform to ASTM standards specified in the approved construction documents.	–	X	Applicable ASTM material standards
c. Manufacturer’s certified test reports.	–	X	–
4. Material verification of weld filler materials:			
a. Identification markings to conform to AWS specifications in the approved construction documents.	–	X	AISC 360, Section A3.5 and applicable AWS A5 documents
b. Manufacturer’s certificate of compliance required.	–	X	–
5. Material verification of load bearing metal stud:			
a. Verify metal strength, size, gage and spacing for studs, lintels and track.	–	X	–
b. Verify fastener size, length, spacing of mechanical connectors, floor, roof and shear wall sheathing.	–	X	–
c. Verify lateral bracing in load bearing walls and verify lintel stiffeners.	–	X	–
d. Verify size of spacing of attachment to the foundation.	–	X	–
e. Verify location of shear walls.	–	X	–
6. Inspection of welding:			
a. Structural steel and cold–formed steel deck:			
1) Complete and partial joint penetration groove welds.	X	–	AWS D1.1
2) Multipass fillet welds.	X	–	
3) Single–pass fillet welds > 5/16"	X	–	
4) Plug and slot welds.	X	–	
5) Single–pass fillet welds ≤ 5/16"	–	X	
6) Floor and roof deck welds.	–	X	AWS D1.3
b. Reinforcing steel:			
1) Verification of weld ability of reinforcing steel other than ASTM A 706.	–	X	AWS D1.4 ACI 318: Section 3.5.2
2) Reinforcing steel resisting flexural and axial forces in intermediate and special moment frames, and boundary elements of special structural walls of concrete and shear reinforcement.	X	–	
3) Shear reinforcement.	X	–	
4) Other reinforcing steel.	–	X	
7. Inspection of steel frame joint details for compliance:			
a. Details such as bracing and stiffening.	–	X	–
b. Member locations.	–	X	–
c. Application of joint details at each connection.	–	X	–

SPECIAL INSPECTIONS:

1. SPECIAL INSPECTIONS ARE REQUIRED FOR THIS PROJECT
2. SPECIAL INSPECTIONS SHALL BE DONE AS THE WORK PROGRESSES.
3. FOR EACH CONSTRUCTION ACTIVITY LISTED FOR INSPECTION THE INSPECTOR SHALL BE ON SITE PERIODICALLY OR CONTINUOUSLY AS INDICATED WHILE THE WORK IS BEING PERFORMED.
4. NON-COMPLIANT WORK SHALL REPORTED BY THE SPECIAL INSPECTOR AND CORRECTED BY THE GENERAL CONTRACTOR. A FOLLOW UP INSPECTION SHALL BE PERFORMED TO CONFIRM COMPLIANCE OF THE CORRECTIVE ACTION.
5. A FINAL INSPECTION ONCE THE ACTIVITY IS COMPLETED TO CONFIRM COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS.

WES

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NO.	DATE	REVISION
1	08/14/17	2ND FLOOR FRAMING
2	12/05/17	REV FOR EXISTING CONDITIONS

BAPS, INC.

RENOVATION OF SWAMI NARAYAN TEMPLE

2390 GREENWAY DRIVE

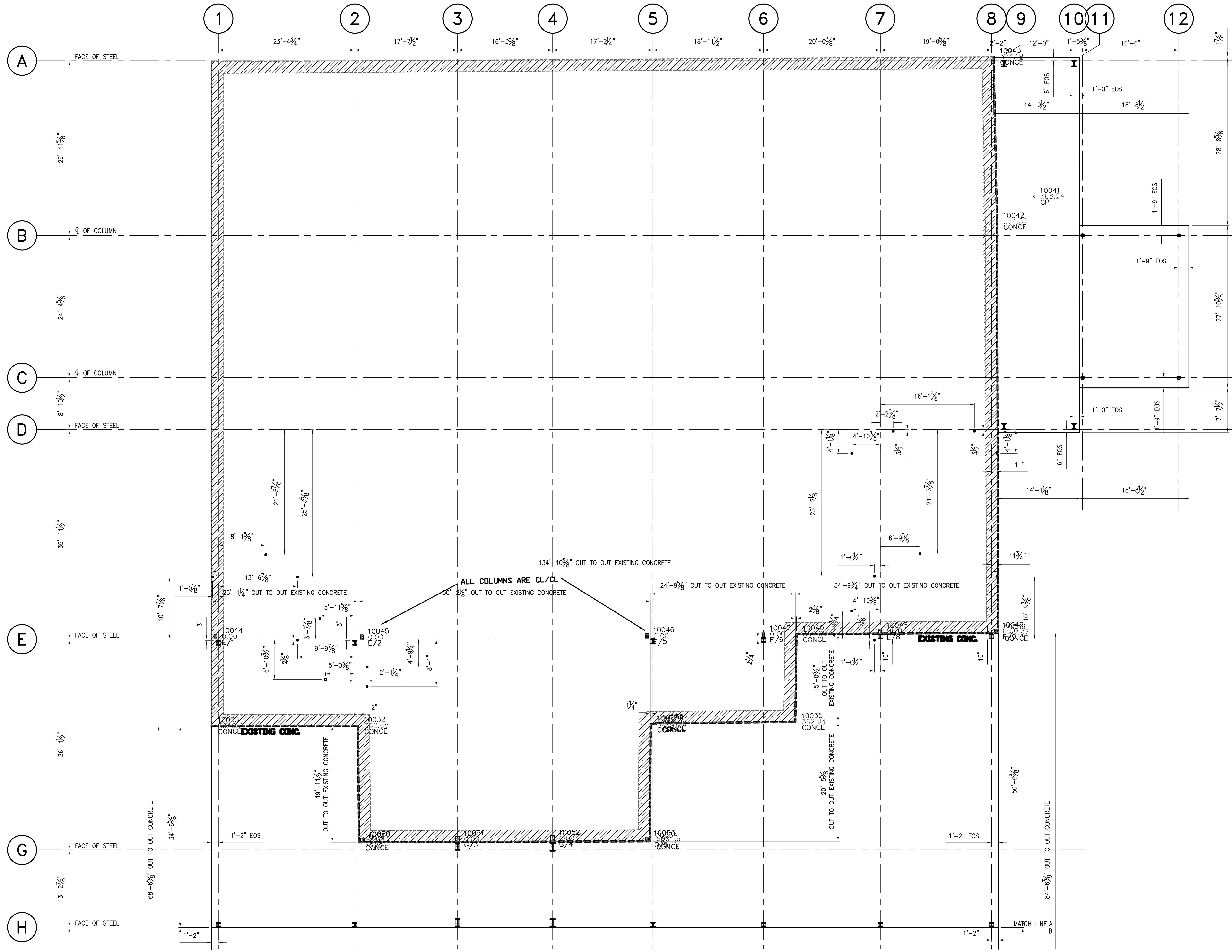
JACKSON, MS 39204

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☒ CONSTRUCTION
DATE : 03-13-17

DRAWING TITLE:
**STRUCTURAL
SPECIAL
INSPECTIONS**

SCALE:
PROJECT NO: 03TBN4
DATE: 03-13-17
DRAWN BY: HVS
CHECKED BY: WCVW
SHEET NO:

S002



DIMENSION PLAN

1/8" = 1'-0"

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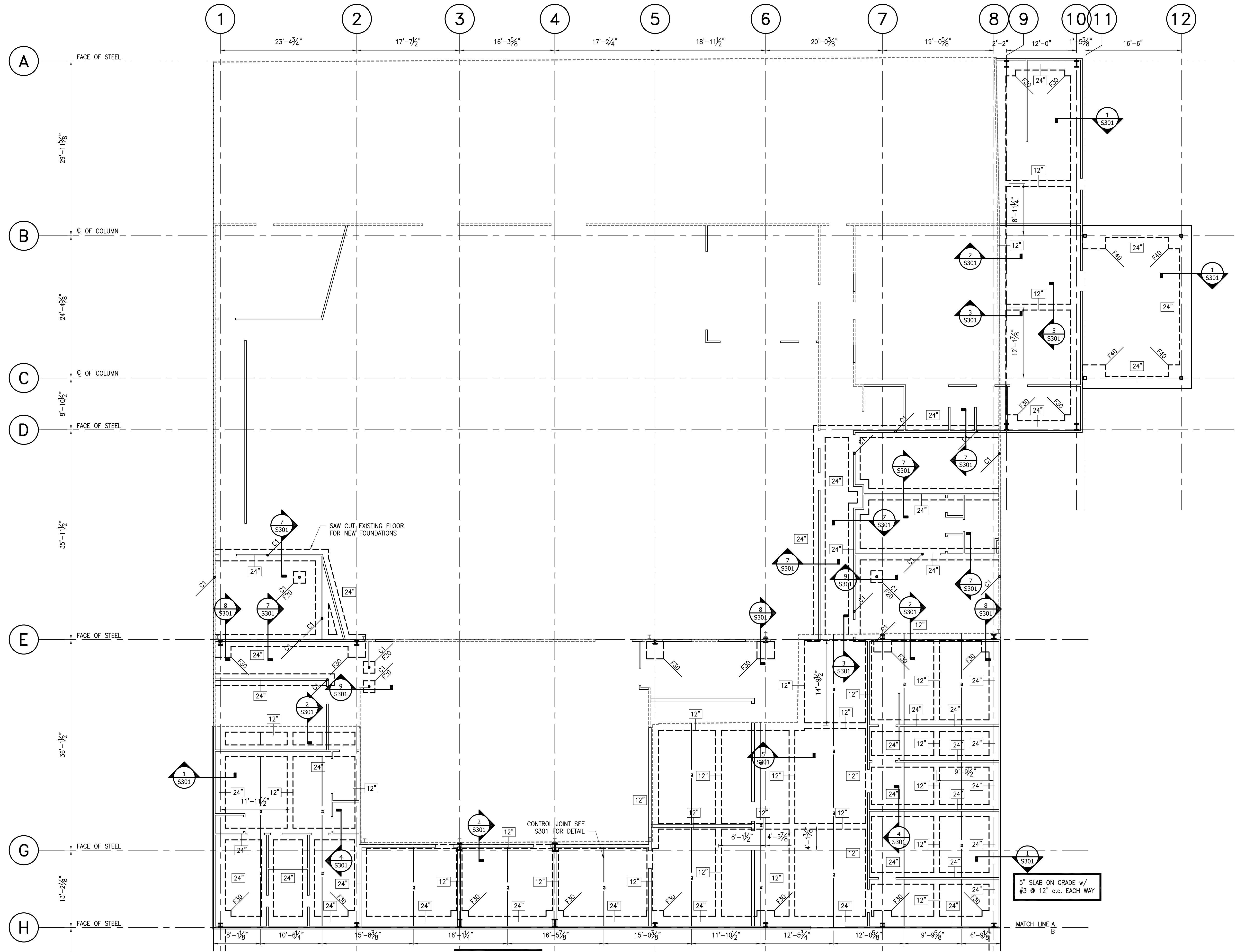
BAPS, INC.
RENOVATION OF SWAMI NARAYAN TEMPLE
2390 GREENWAY DRIVE
JACKSON, MS 39204

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☒ CONSTRUCTION
DATE : 03-13-17

DRAWING TITLE:
DIMENSION PLAN

SCALE: AS SHOWN
PROJECT NO: 037EN4
DATE: 03-13-17
DRAWN BY: HVS
CHECKED BY: WJW
SHEET NO:

S101



FOUNDATION PLAN

1/8" = 1'-0"

FOOTING SCHEDULE			
MARK	SIZE	REINFORCING	
F20	2'-0" x 2'-0" x 12"	3 #5 EW	
F30	3'-0" x 3'-0" x 12"	3 #5 EW	
F40	4'-0" x 4'-0" x 12"	4 #5 EW	

COLUMN SCHEDULE		
MARK	SIZE	BASE PLATE
C1	HSS 3½x3½x½	10x10½

WES
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WILLIAM O. WHISONANT
Professional Engineer
19784
STATE OF MISSISSIPPI
05/17/17

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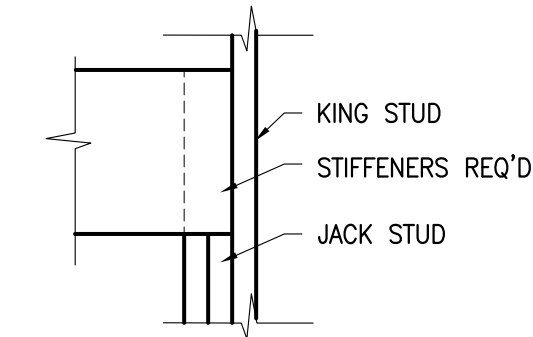
BAPS, INC.
RENOVATION OF SWAMI NARAYAN TEMPLE
2390 GREENWAY DRIVE
JACKSON, MS 39204

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DATE : 03-13-17

DRAWING TITLE:
**FOUNDATION
PLAN**

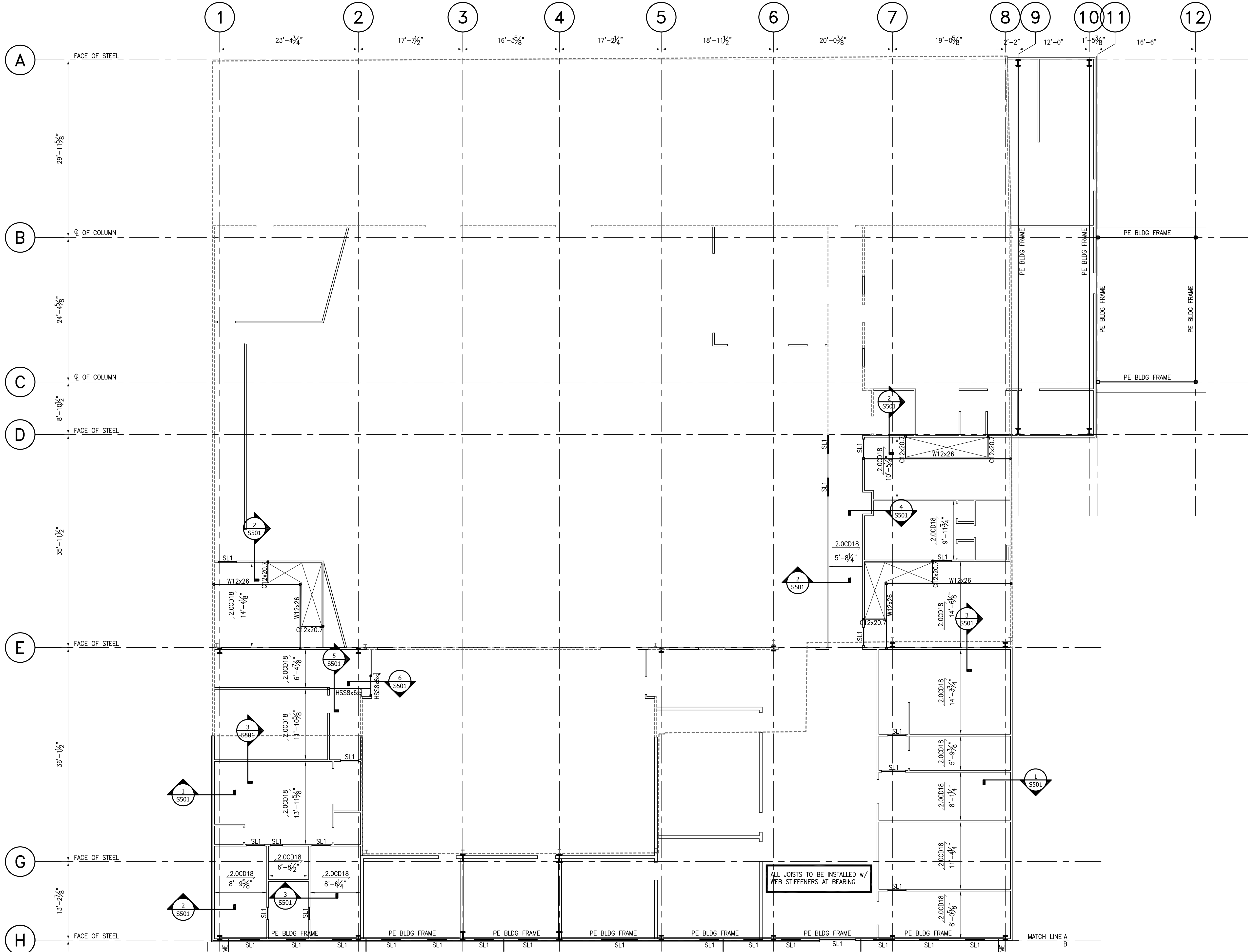
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PROJECT NO: 037EN4
DATE: 03-13-17
DRAWN BY: HVS
CHECKED BY: WCVW
SHEET NO:
S102

12/6/2017 8:11:41 AM



BEARING TYPE 1
ADD ONE KING POST AT FIRST FLOOR
OPENINGS TO THOSE LISTED IN SCHEDULE

LINTEL SCHEDULE				
MARK	SIZE	DETAIL	BEARING TYPE	SUPPORT
SL1	(2) 600S162-54x50		1	2 JACK STUDS 1 KING STUD
NON LOAD BEARING LINTELS	(2) 600S162-33x33		1	2 JACK STUDS 1 KING STUD



SECOND FLOOR FRAMING PLAN

1/8" = 1'-0"

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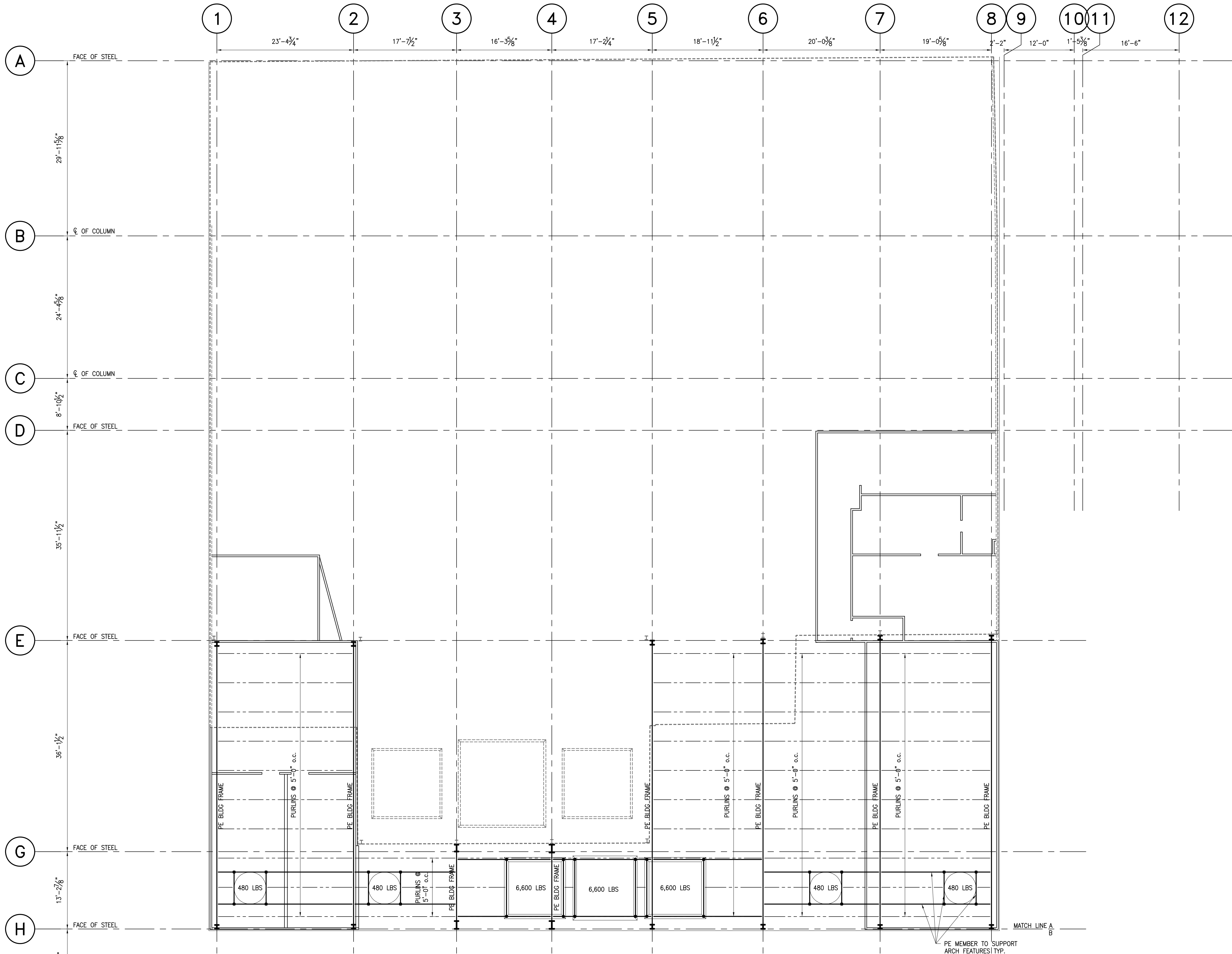
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RENOVATION OF SWAMI NARAYAN TEMPLE
2390 GREENWAY DRIVE
JACKSON, MS 39204

RELEASED FOR:
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☒ CONSTRUCTION
DATE : 03-13-17

DRAWING TITLE:
**SECOND FLOOR
FRAMING PLAN**

SCALE: AS SHOWN
PROJECT NO: 03TIN4
DATE: 03-13-17
DRAWN BY: HVS
CHECKED BY: WCVW
SHEET NO:

S201



ROOF FRAMING PLAN
1/8" = 1'-0"

WES WHISONANT
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SERVICES, LLC
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Lexington, South Carolina 29072
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And Any Infringement
Will Be Subject to
Legal Action.



NO.	DATE	REVISION
1	08/14/17	2ND FLOOR FRAMING
2	12/05/17	REV FOR EXISTING CONDITIONS

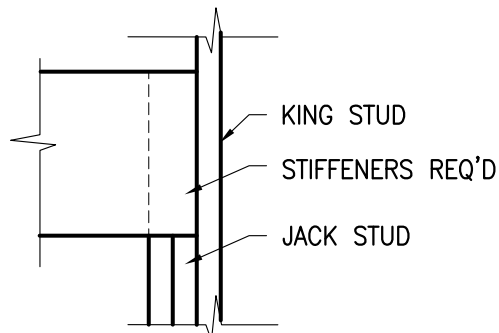
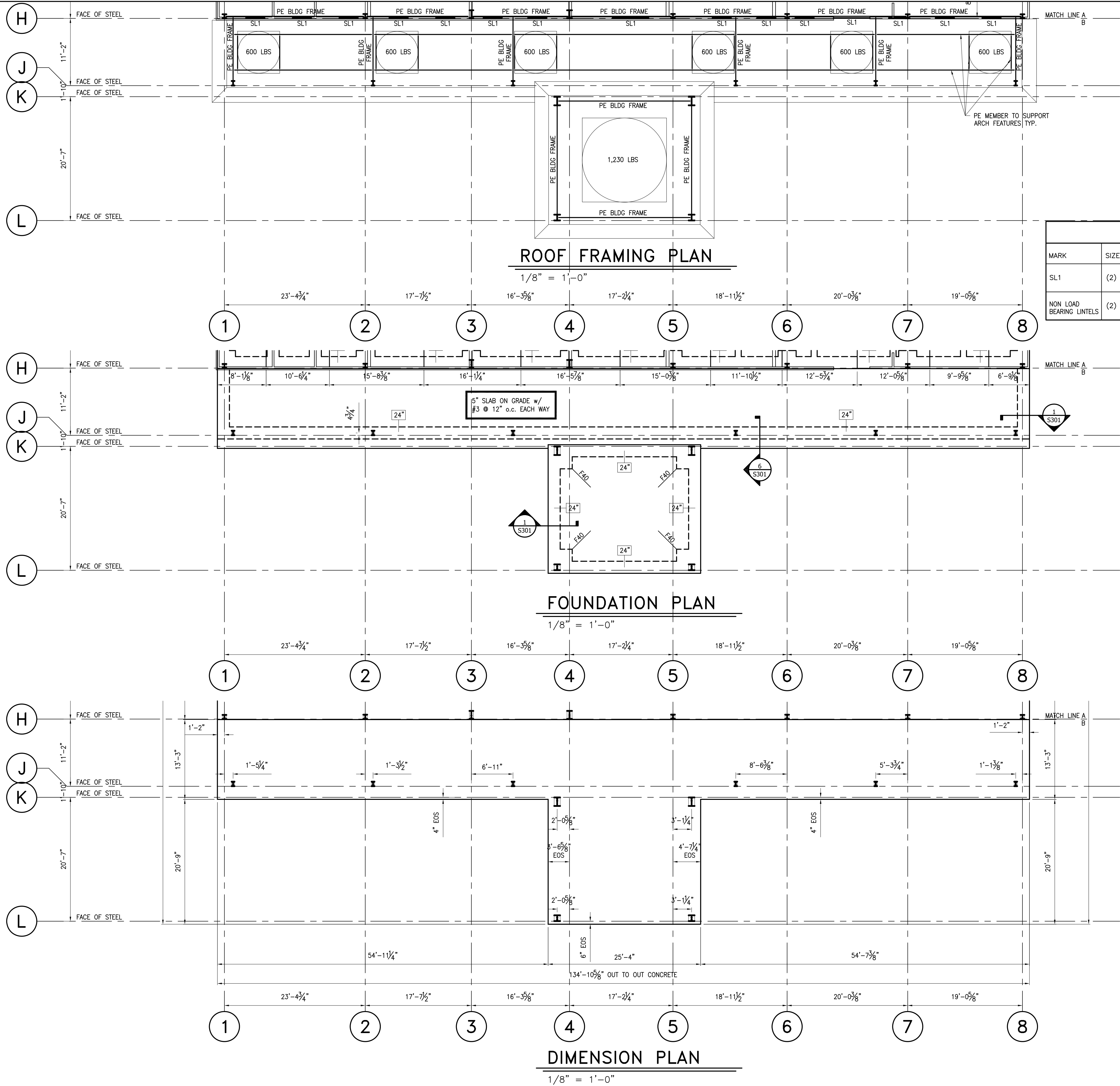
BAPS, INC.
RENOVATION OF SWAMI NARAYAN TEMPLE
2390 GREENWAY DRIVE
JACKSON, MS 39204

RELEASED FOR:
☐ PRELIMINARY ONLY
☐ PRICING
☒ PERMIT
☒ CONSTRUCTION
DATE : 03-13-17

DRAWING TITLE:
**ROOF
FRAMING PLAN**

SCALE: AS SHOWN
PROJECT NO: 037EN4
DATE: 03-13-17
DRAWN BY: HVS
CHECKED BY: WESW
SHEET NO:

S202



BEARING TYPE 1
ADD ONE KING POST AT FIRST FLOOR
OPENINGS TO THOSE LISTED IN SCHEDULE

LINTEL SCHEDULE				
MARK	SIZE	DETAIL	BEARING TYPE	SUPPORT
SL1	(2) 600S162-54x50		1	2 JACK STUDS 1 KING STUD
NON LOAD BEARING LINTELS	(2) 600S162-33x33		1	2 JACK STUDS 1 KING STUD

FOOTING SCHEDULE		
MARK	SIZE	REINFORCING
F20	2'-0" x 2'-0" x 12"	3 #5 EW
F30	3'-0" x 3'-0" x 12"	3 #5 EW
F40	4'-0" x 4'-0" x 12"	4 #5 EW

WES WHISONANT ENGINEERING SERVICES, LLC
122 Nut Tree Court
Lexington, South Carolina 29072
(803) 957-4008
Email: bill@weslex.com

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STAMP: WILLIAM O. WHISONANT
ENGINEER
19784
STATE OF MISSISSIPPI

NO.	DATE	REVISION
1	08/14/17	2ND FLOOR FRAMING
2	12/05/17	REV FOR EXISTING CONDITIONS

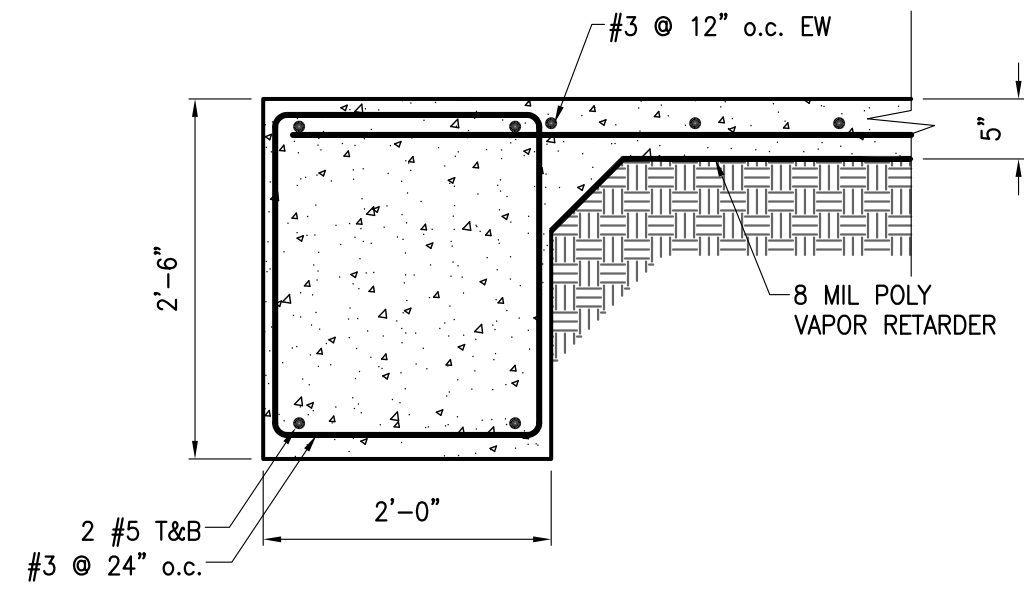
BAPS, INC.
RENOVATION OF SWAMI NARAYAN TEMPLE
2390 GREENWAY DRIVE
JACKSON, MS 39204

RELEASED FOR:
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☒ CONSTRUCTION
DATE : 03-13-17

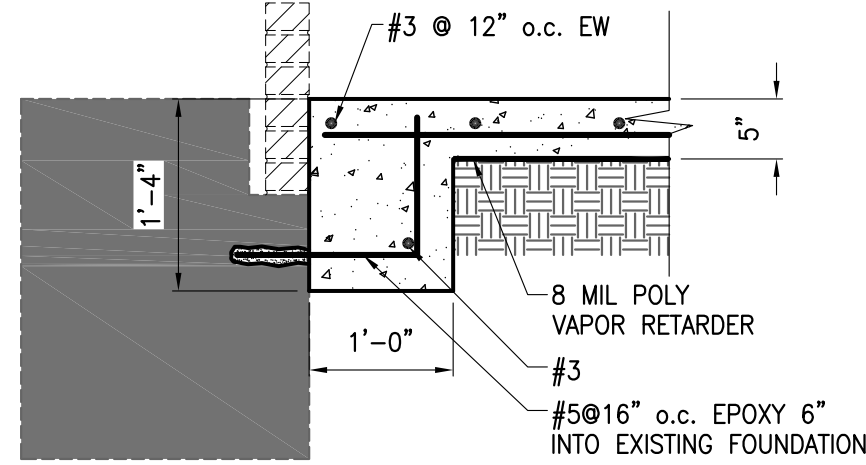
DRAWING TITLE:
**CANOPY DIMENSION,
FOUNDATION &
ROOF FRAMING
PLAN**

SCALE: AS SHOWN
PROJECT NO: 037EN4
DATE: 03-13-17
DRAWN BY: HVS
CHECKED BY: WJW
SHEET NO:

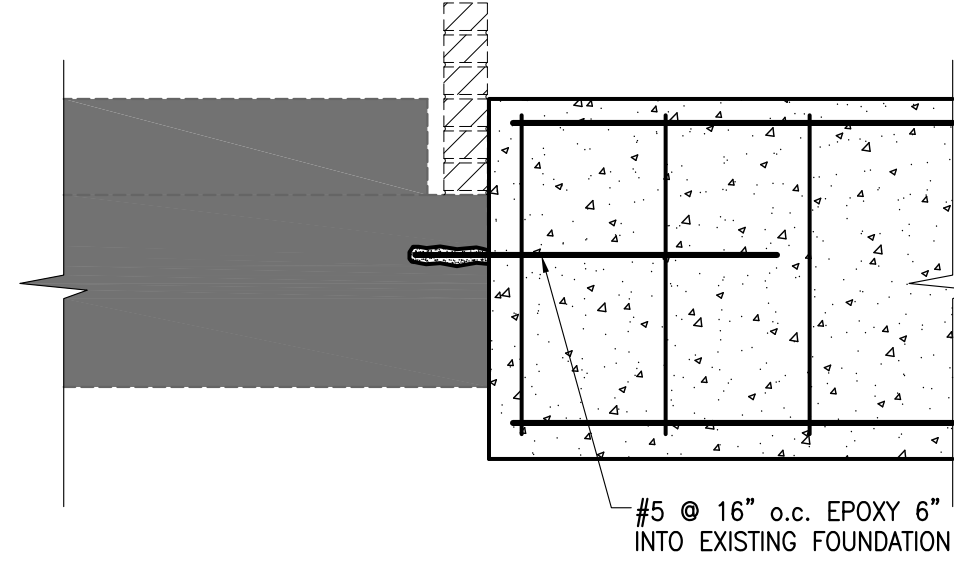
S203



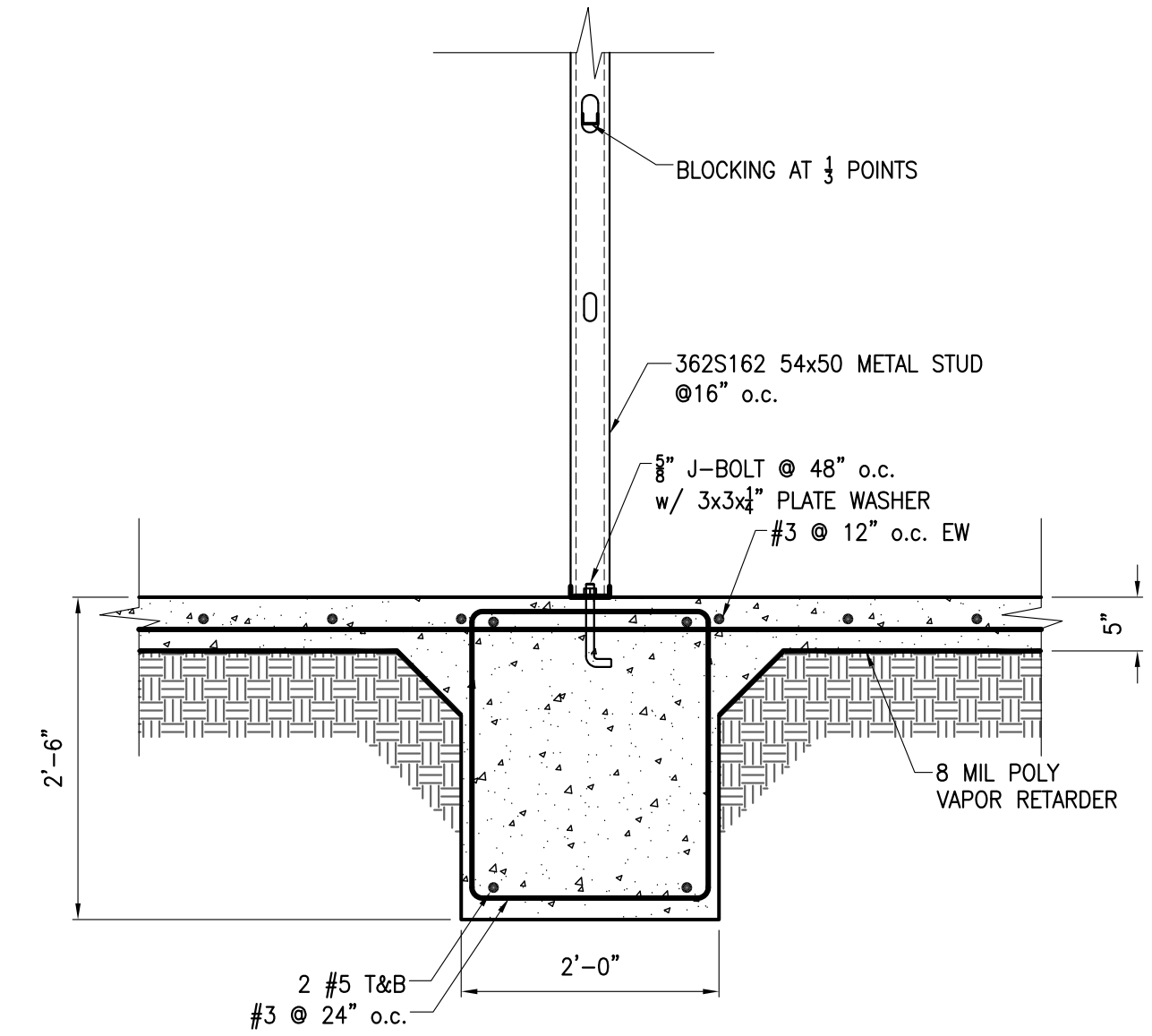
1 SECTION
S301 3/4" = 1'-0"



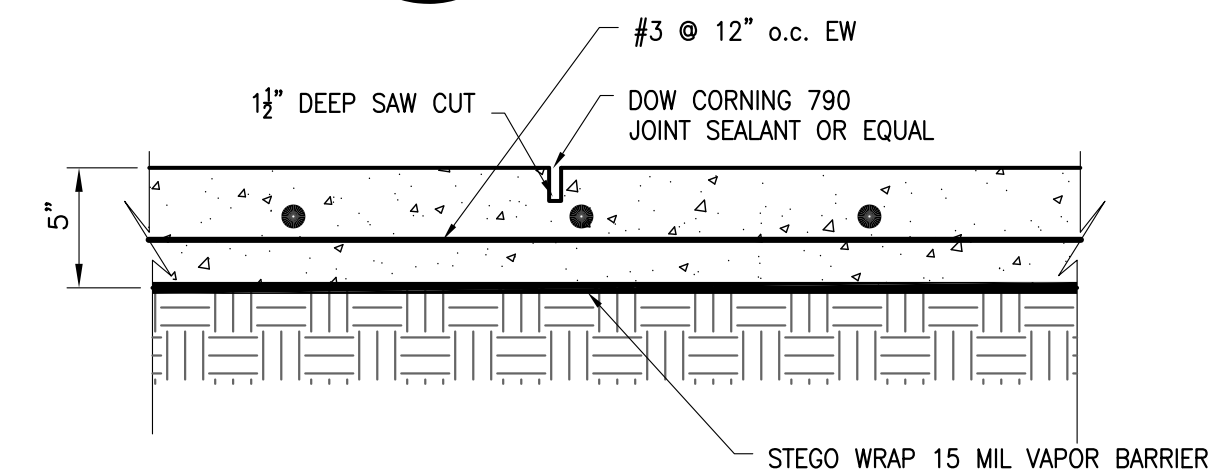
2 SECTION
S301 3/4" = 1'-0"



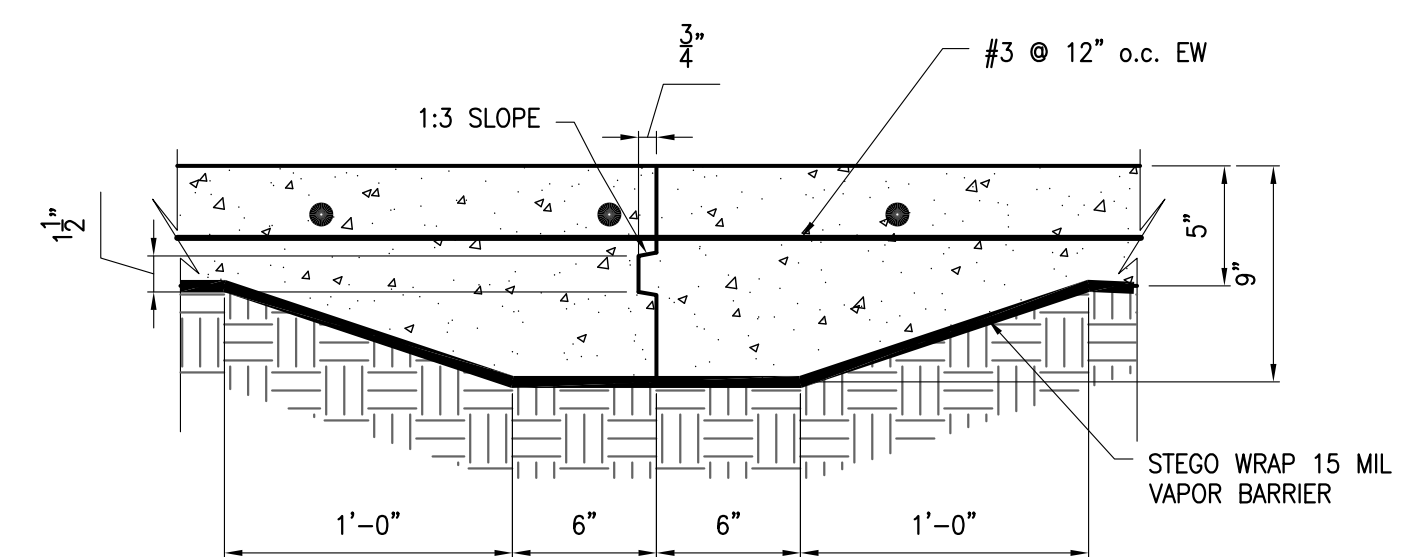
3 SECTION
S301 3/4" = 1'-0"



4 24" GRADE BEAM
S301 3/4" = 1'-0"



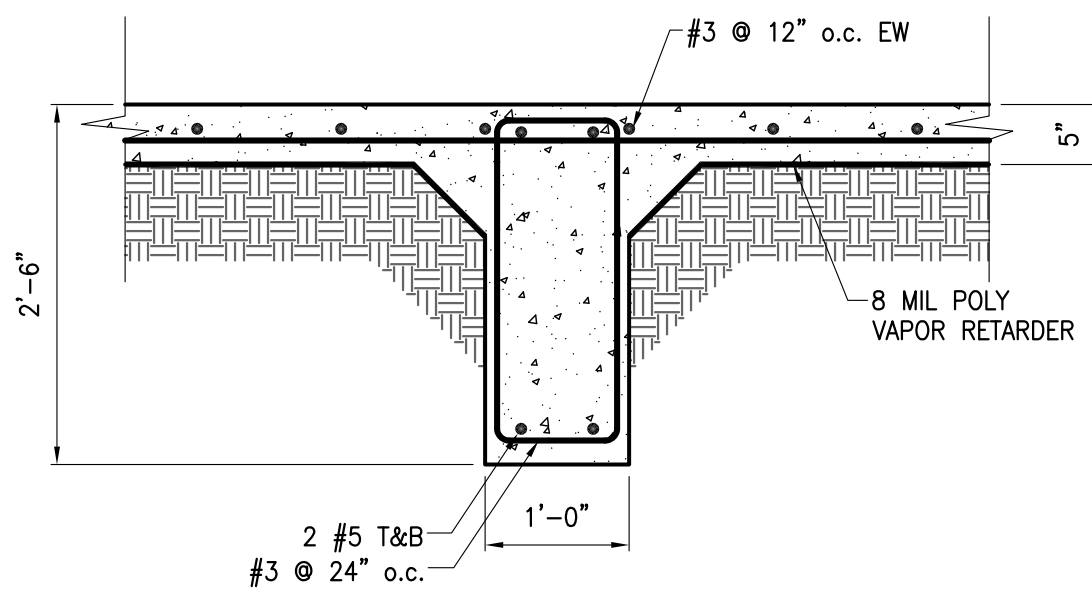
SAW JOINT



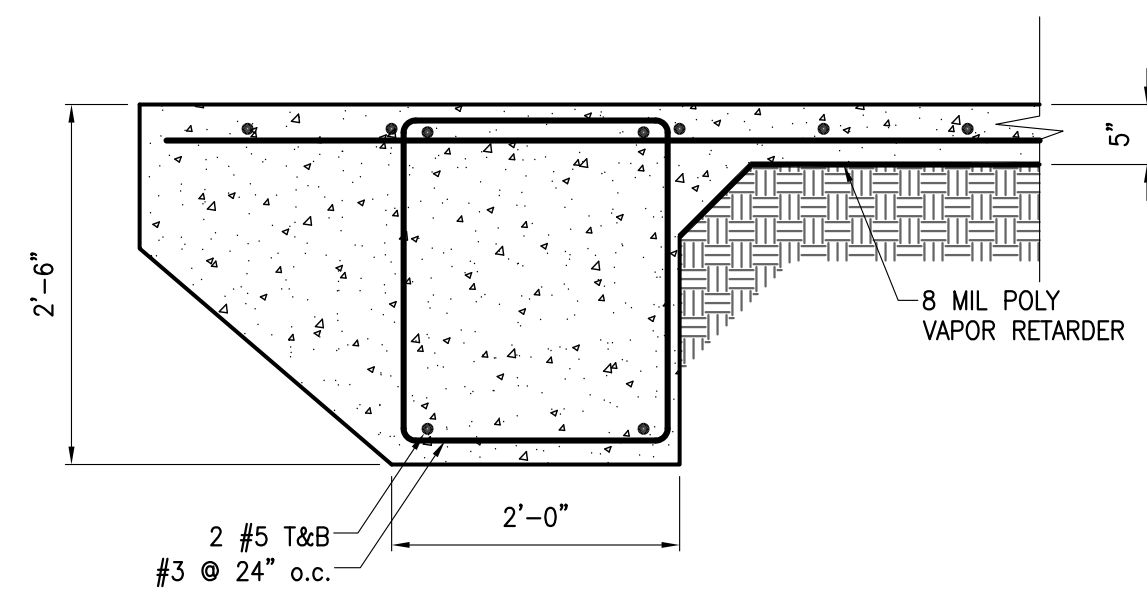
KEYED JOINT

CONTROL JOINT DETAIL - OPTIONS

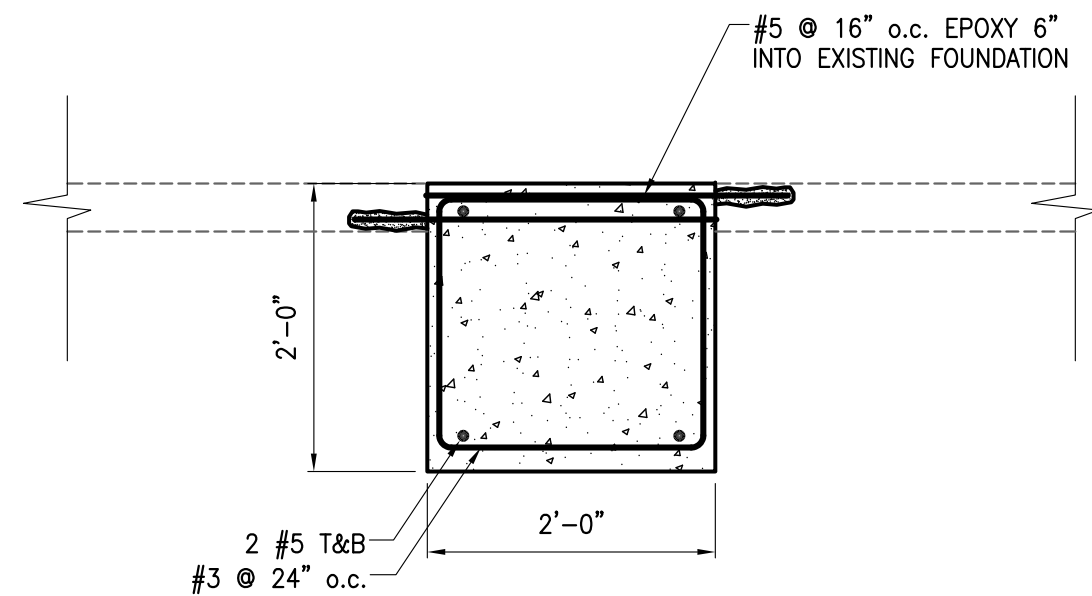
1 1/2" = 1'-0" EITHER SAWED OR KEYED JOINT MAY BE USED FOR CONTROL JOINTS SHOWN
PROVIDE KEYED JOINT ONLY AT FOUR STOPS



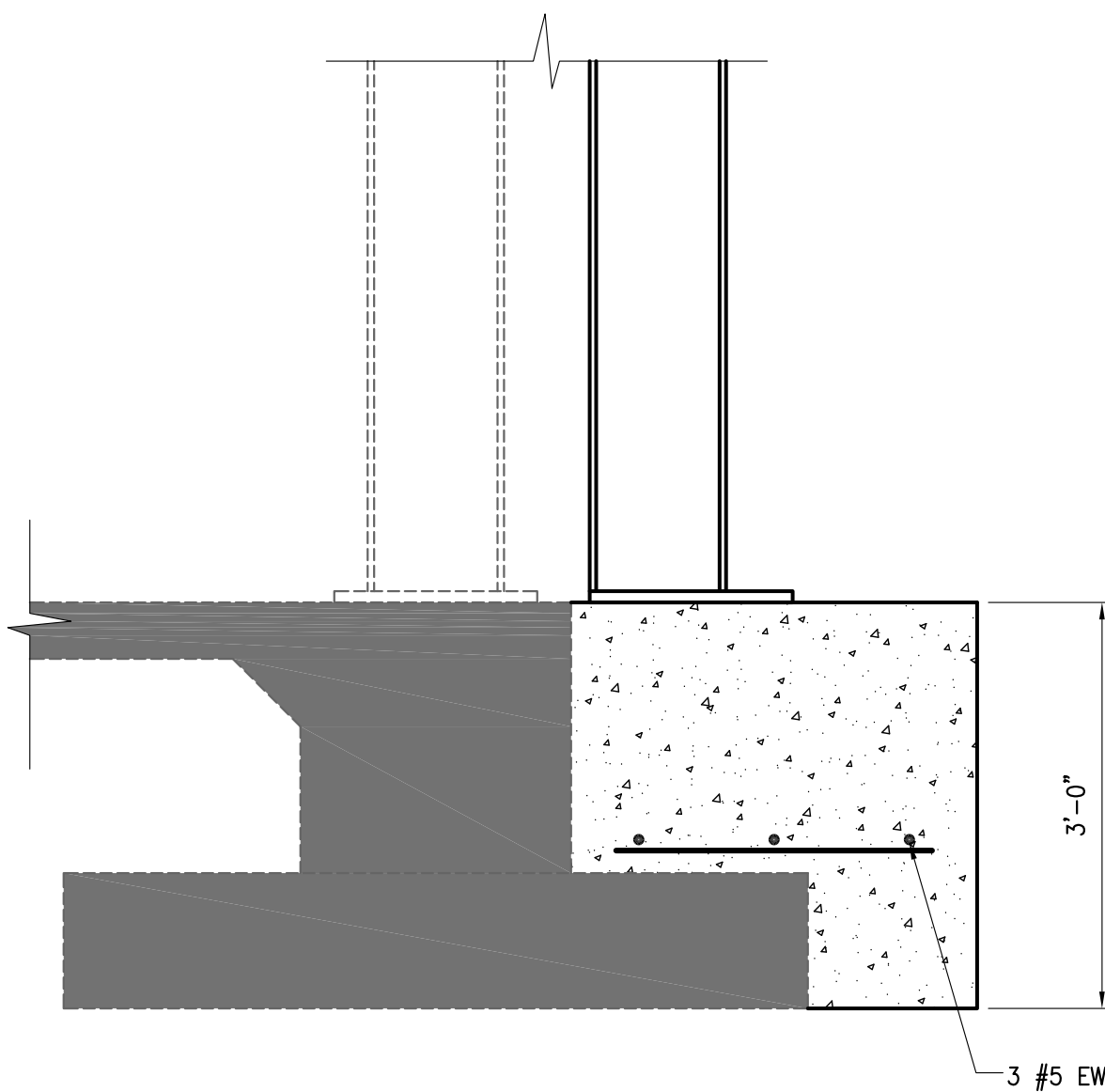
5 12" GRADE BEAM
S301 3/4" = 1'-0"



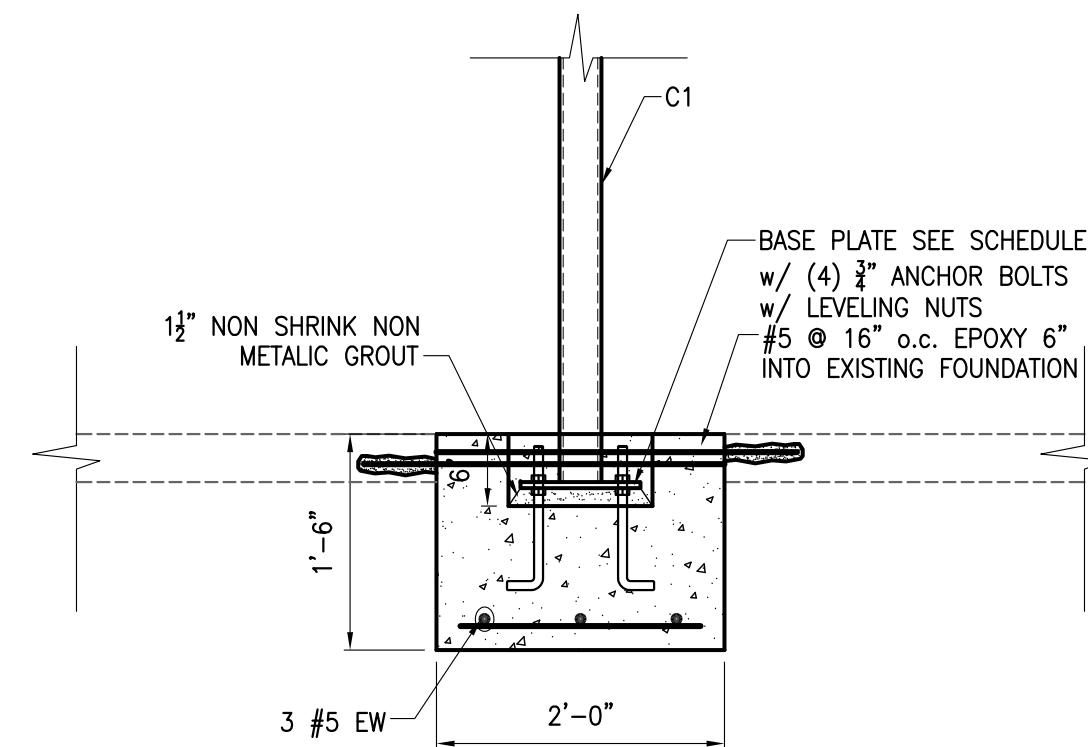
6 SECTION
S301 3/4" = 1'-0"



7 SECTION
S301 3/4" = 1'-0"



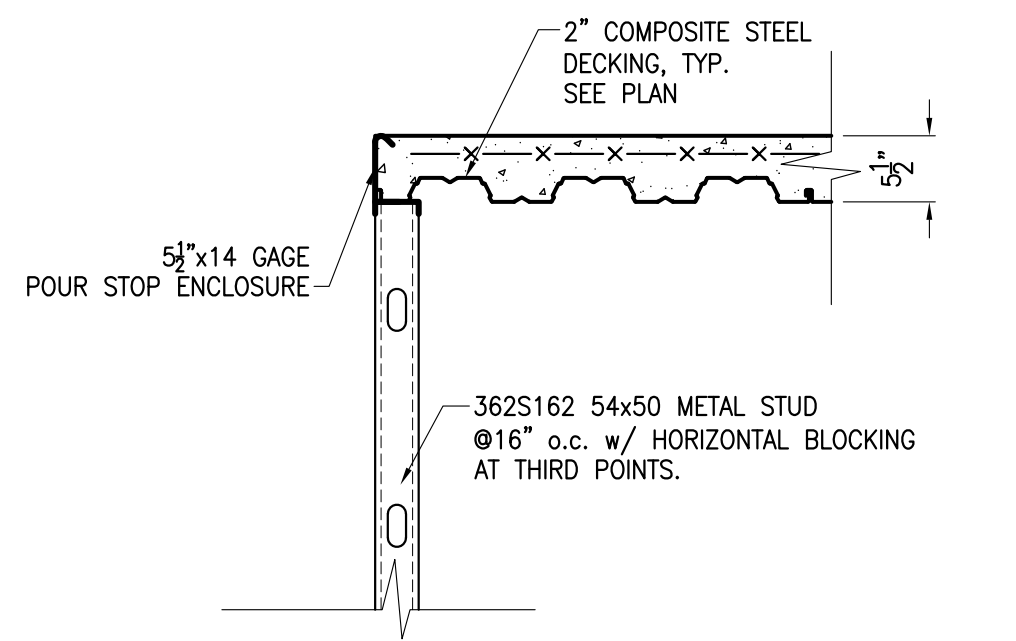
8 SECTION
S301 3/4" = 1'-0"



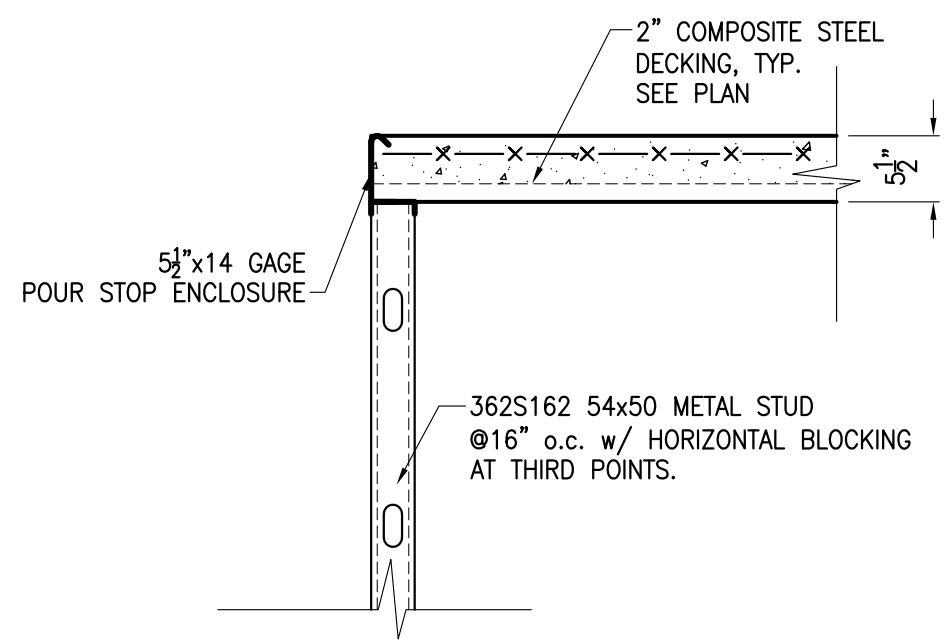
9 SECTION
S301 3/4" = 1'-0"

NO.	DATE	REVISION
1	05/15/17	2ND FLOOR FRAMING
2	12/06/17	REV FOR EXISTING CONDITION

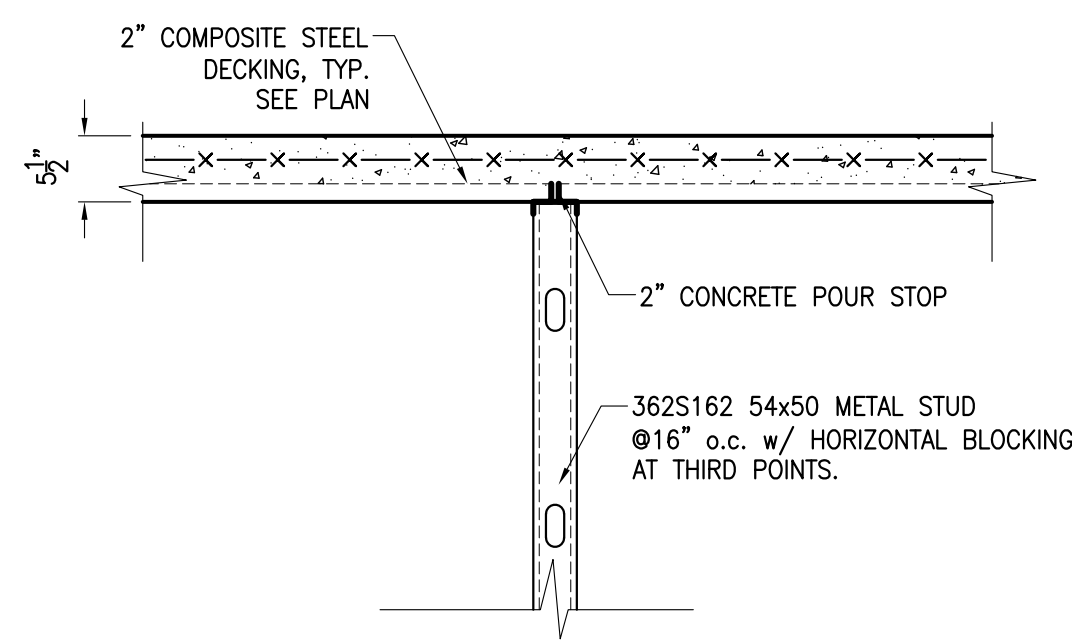




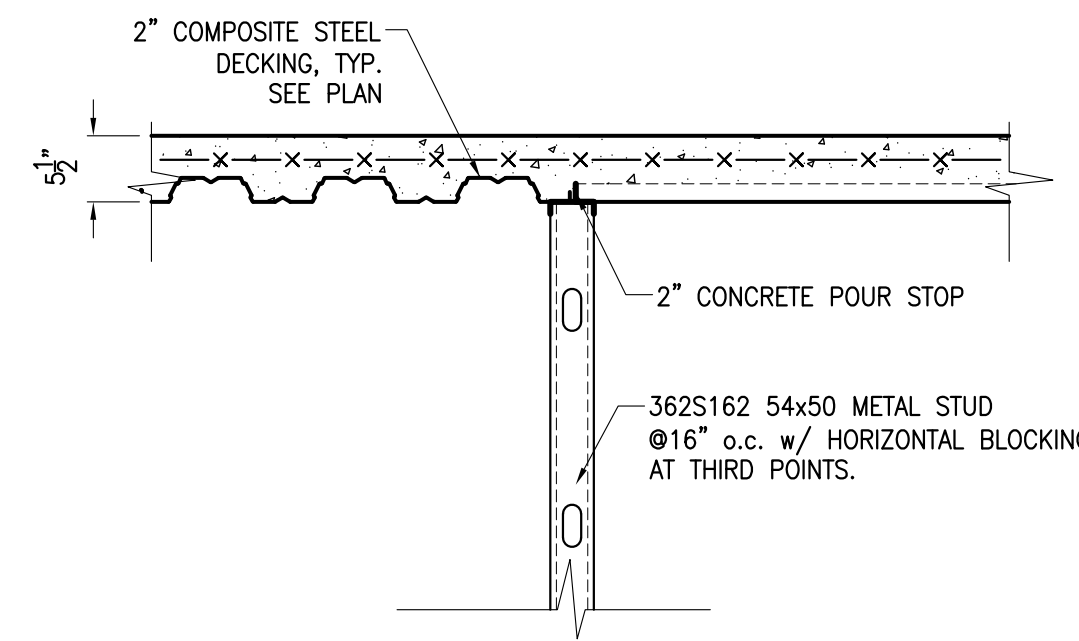
1 SECTION
S501 $\frac{3}{4}" = 1'-0"$



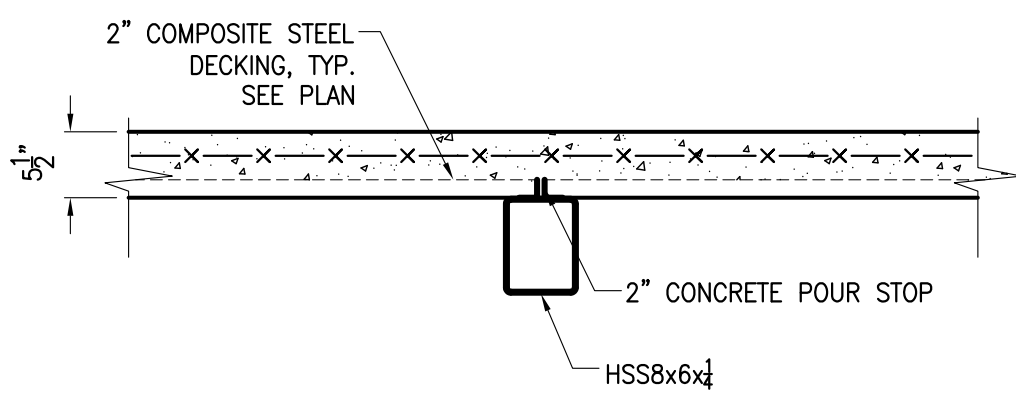
2 SECTION
S501 $\frac{3}{4}" = 1'-0"$



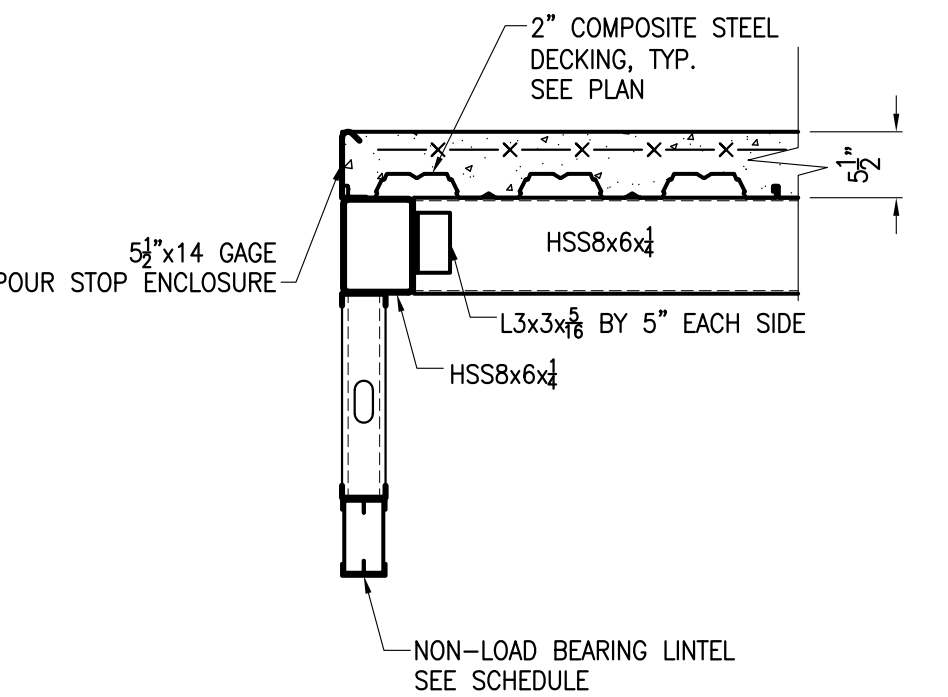
3 SECTION
S501 $\frac{3}{4}" = 1'-0"$



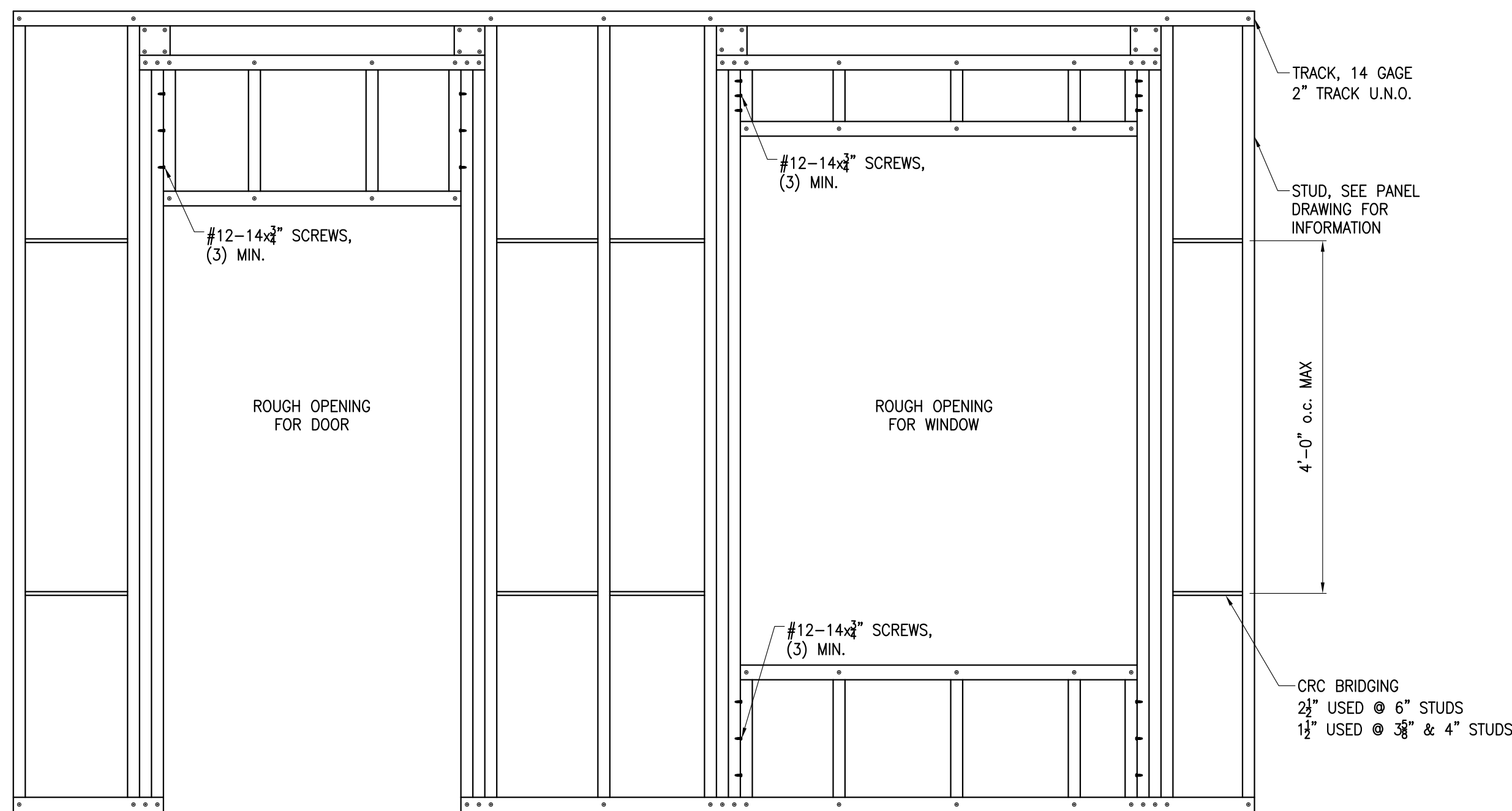
4 SECTION
S501 $\frac{3}{4}" = 1'-0"$



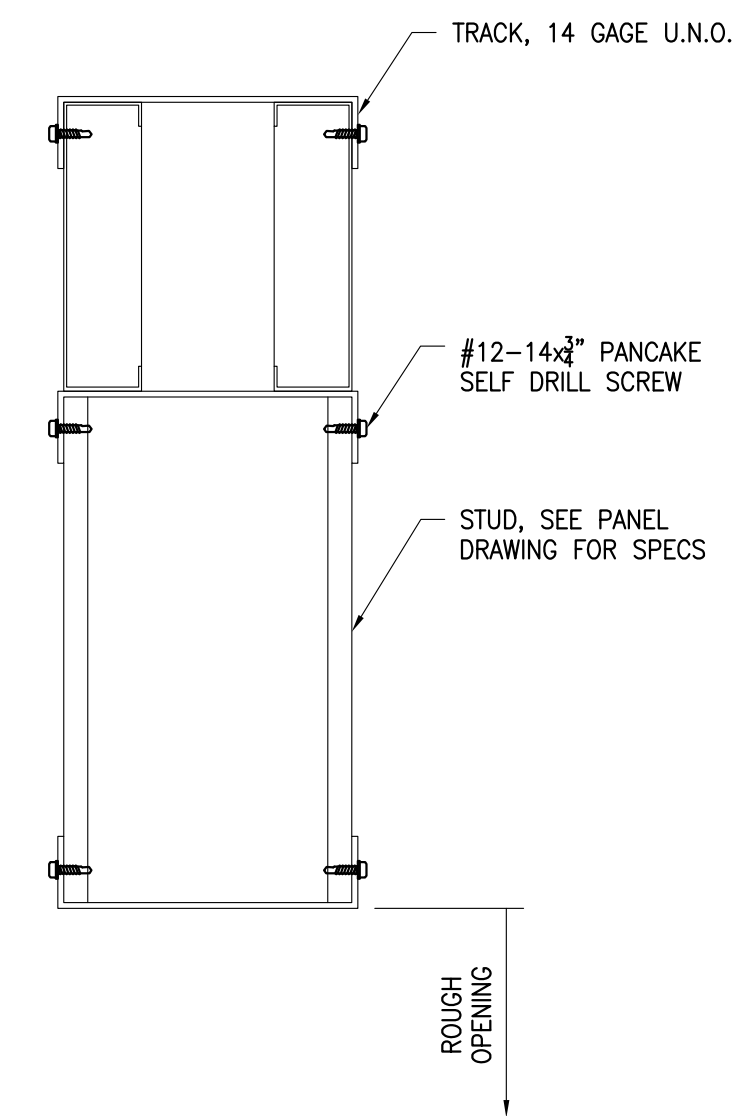
5 SECTION
S501 $\frac{3}{4}" = 1'-0"$



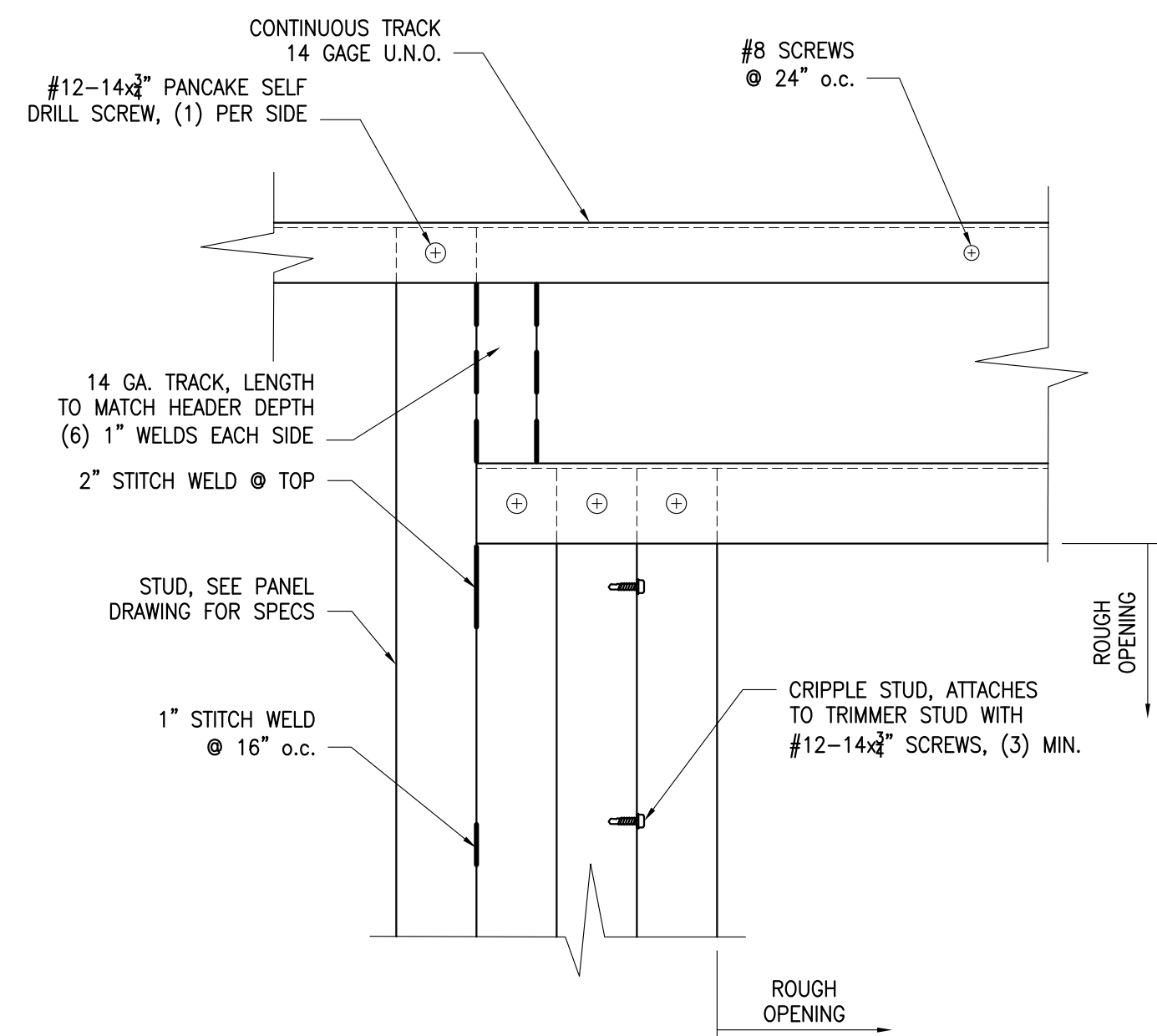
6 SECTION
S501 $\frac{3}{4}" = 1'-0"$



ROUGH OPENING ELEVATION
 $\frac{3}{4}" = 1'-0"$



HEADER SECTION
 $\frac{3}{4}" = 1'-0"$



HEADER DETAIL
 $\frac{3}{4}" = 1'-0"$

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