LADNER

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SUBSURFACE INVESTIGATION

FOR

B.A.P.S. INC.
SWAMI NARAYAN TEMPLE EXPANSION
2390 GREENWAY DRIVE
JACKSON, MISSISSIPPI

JUNE 2016

BY

GEOTECHNICAL ASSOCIATES NETWORK, LLC
110 BEECHTREE ROAD
VICKSBURG, MISSISSIPPI 39183-7464

SUBSURFACE INVESTIGATION FOR
B.A.P.S. INC.
SWAMI NARAYAN TEMPLE EXPANSION
GREENWAY DRIVE
JACKSON, MISSISSIPPI

PURPOSE

The purposes of this subsurface investigation are as follows:

a. To determine the general characteristics of the subsurface soils within the area of the proposed construction;

b. To determine by field and laboratory testing, the physical characteristics of the foundation soils and the soil samples collected; and

c. To make recommendations for foundation construction at this particular location.

FIELD INVESTIGATION

Four subsurface borings were made at the site of the proposed Hindu Temple Expansion for B.A.P.S. Inc. on Greenway Drive, Jackson, Mississippi. The borings were advanced with a truck mounted, powered continuous flight auger. Auger cuttings of the soil medium were collected at changes in strata and at intervals not exceeding five feet in depth. All samples were stored in sealed containers for later classification and testing. In addition, standard penetration resistance values (see ASTM D-1586-84) were determined and recorded on the boring logs for the various materials encountered. The Standard Penetration Test gives an indication of the in-place shear strength of cohesive soils and the relative density of cohesionless soils by recording the number of blows required, by a 140-pound hammer falling 30 inches, to drive a 2-inch O.D. splitspoon sampler one foot. Static water levels were recorded on the borings after completion of drilling and sampling operations at the site.

LABORATORY INVESTIGATION

Laboratory testing of selected soil samples included visual classification, Atterberg limits on cohesive soils with determination of the plasticity index (PI), grain size analyses, and in situ moisture contents. Atterberg limits (ASTM D-4318-93) were run on the clayey soils in an effort to estimate the susceptibility of these soils to shrink and swell with changes in moisture content. Liquid and plastic limits were run on samples selected from some of the various materials encountered. The liquid limit (LL) is the moisture content at which a soil changes from a plastic state to a viscous liquid state. The plastic limit is the moisture content at which a soil changes from a solid state to a plastic state. The plasticity index is the numerical difference between the liquid limit and the plastic limit and is indicative of the relative activity or sensitivity of a cohesive soil.

Grain size analyses (ASTM D-422-63) were conducted on representative samples of the various soils encountered to determine the particle size distribution of materials comprising the strata. Results of these tests were utilized in classifying the soils by the Unified Soil Classification System. Classifications for each of the soil samples are shown on the boring logs attached to this report.

To aid in the general interpretation of the soil conditions at the site, in situ moisture contents were determined for samples selected from the various soils encountered. This determination was made possible by placing extracted samples in sealed containers immediately upon removal from each interval. The results of these and other tests are recorded on the attached boring logs.

SUBSURFACE CONDITIONS

The site of the proposed new construction is at 2390 Greenway Drive in Section 11, Township 5 North, Range 1 West in Jackson, Hinds County, Mississippi. Physiographically, the location is in the Gulf Coastal Plain Province of North America and in the Jackson Prairie Province of Mississippi. Structurally, it is southwest of the Jackson Dome and in the Mississippi Interior Salt Basin. Stratigraphically, soils are derived from the Eocene Yazoo Formation, which was deposited in a marine environment. The Yazoo Formation can present significant engineering problems

because of its expansive property. Fill materials may have been placed at this site during previous construction.

Four borings were placed at the site. One was drilled to a depth of 41½ feet (Boring No. B-3) and three were drilled to depths of 10 feet (Boring Nos. B-1, B-2, and B-4). The soils encountered were lean clays (CL), heavy clays (CH), clayey sands (SC), silt with sand (ML), and silty clay (CL-ML). As inferred from the SPT data, the consistencies of the clays and silts ranged from soft to stiff.

Soils Data Table
B.A.P.S., Inc.
Swami Narayan Temple Expansion
2390 Greenway Drive
Jackson, Mississippi

Boring Numbers	Clayey Sand (SC)	Silt w/Sand (ML)	Silty Clay (CL-ML)	Lean Clay (CL)	Heavy Clay (CH)	Water (WD)	Total Depth (FT)
B-1	0 - 1	1 - 2½			2½ - 10	3	10
B-2			0 - 1	1 - 5	5 - 10		10
B-3				0 - 5	5 - 41½		41½
B-4				0 - 5	5 - 10		10

Depths are in feet below the surface.
WD = While Drilling.

Asphalt (3½ inches thick) and red and gray clayey sand (SC) were encountered in the top one foot of Boring No. B-1. The field moisture content was 10.9 percent, and 40.8 percent of the material passed the #200 grain-size sieve. As expected, this is a low plasticity material with plasticity index of 13 percent and liquid limit of 27 percent. The shrink/swell potential is low and only very small changes in volume would be expected with changes in moisture content.

One 1½-foot-thick stratum of tan and gray silt with sand (ML) was encountered from 1 foot to 2½ feet in Boring No. B-1 below the clayey sand (SC). This silt had a soft consistency, as suggested by a SPT blow count of 4 blows. The field moisture content was 25.1 percent, and 72.1 percent of the material passed the #200 grain-size sieve. As expected, this is a low plasticity material with plasticity index of 9 percent and liquid limit of 37 percent. The shrink/swell potential is low and only very small changes in volume would be expected with changes in moisture content.

One five-foot-thick stratum of tan and brown silt clay (CL-ML) with roots was encountered at the surface in Boring No. B-2. The field moisture content was 14.9 percent, and 70.2 percent of the material passed the #200 grain-size sieve. As expected, this is a low plasticity material with plasticity index of 7 percent and liquid limit of 28 percent. The shrink/swell potential is low and only very small changes in volume would be expected with changes in moisture content.

The lean clays (CL) were encountered at shallow depths in two out of the four borings at the site. Rootlets were encountered in Boring No. B-2 between 1 and 3½ feet deep. The colors of these lean clays were brown and tan; brown; tan; brown and gray; and tan and gray. The consistencies ranged from soft to stiff, as suggested by SPT blow counts ranging from 3 blows to 13 blows (averaging 8.5 blows). These are medium plasticity materials with plasticity indices ranging from 13 percent to 25 percent (averaging 19 percent) and liquid limits ranging from 35 percent to 46 percent (averaging 40.4 percent). These materials have a moderate shrink/swell potential suggesting that they are subject to moderate changes in volume with changes in moisture content. The field moisture contents ranged from 12.4 percent to 25.5 percent and averaged 19.6 percent. The percentages of these lean clays passing through the #200 grain-size sieve ranged from 88.0 percent to 97.4 percent and averaged 94.8 percent.

Heavy clays (CH) were encountered in the lower part of each boring at the site, as noted in the Soils Data Table above. Descriptions of these materials were heavy clays, heavy clays with gravel, heavy clays with calcareous nodules, and heavy clays with quartz. The colors of these heavy clays were tan and gray; tan; and blue. The tan and gray to tan heavy clays are weathered Yazoo Clay, and the blue materials are unweathered Yazoo Clay. The percent of these materials passing the #200 grain-size sieve ranged from 83.4 percent to 99.8 percent (averaging 95.6 percent). The field moisture contents ranged from 16.9 percent to 42.2 percent and averaged 34.7 percent. These are high to very high plasticity clays that had liquid limits ranging from 50 percent to 122 percent (averaging 82.6 percent) and plasticity indices ranging from 30 percent to 96 percent (averaging 58.7 percent). These heavy clays have a high to very high shrink/swell potential suggesting that they could be subject to large to very large changes in volume with changes in moisture content.

Water was noted while drilling (WD) at a depth of 3 feet in Boring No. B-1. No water was observed in the borings after

completion of drilling at the site. The actual water table at the site can only be determined with long-term observations. We note that groundwater conditions in this area do fluctuate during the year with variations in rainfall and other environmental factors. Therefore, the groundwater levels and soil moisture contents in the near-surface materials will vary throughout the year and will probably be different if tested at a different time.

RECOMMENDATIONS FOR SITE PREPARATION AND FOUNDATIONS

We understand that this project will consist of a Hindu Temple Expansion for B.A.P.S. Inc. located at the corner of Greenway Drive and Causey Drive, Jackson, Mississippi. The following conclusions and recommendations are based on our understanding of the proposed construction, information gathered during the exploration, accepted geotechnical engineering principles and practices, and our experience with similar sites and subsurface conditions. This report has been prepared for the exclusive use of B.A.P.S. Inc., Attn: Mr. Roy Patel, Brandon, Mississippi in the planning and design of the building. We request that we be informed of any significant changes to the proposed construction so we might review our recommendations in light of the new information. We should also be given an opportunity to review the final foundation and grading plans, as well as applicable portions of the project specifications, prior to construction.

Final plans and specifications were not available at the time of this geotechnical report, but it is our understanding that the construction will be on a lot that has been graded essentially flat. Our interpretation of the soil conditions was determined from borings located inside the building footprint. Based upon the anticipated foundation strengths at the site, and the assumption that no large or unusual loads are anticipated, it is our opinion that the building could be supported by a combination of foundation units, such as grade beams and spread footings. The foundation could consist of a monolithically cast, reinforced concrete, slab on-grade with turned-down, continuous grade beams and interior stiffeners to produce a beam diaphragm system. Column loads could be supported by isolated spread footings or thickened sections. Because of the expansive heavy clay near the surface, the building could also be supported on a pile foundation. Further details of our recommendations are discussed below.

SITE PREPARATION

The near-surface material occurring under the construction area consisted of lean clays (CL), silty clay (CL-ML), clayey sand (SC), and silt (ML) overlying heavy clay (CH). The lean clay ranged from a thickness of 2 ½ to 5 feet over the heavy clay and would not provide enough cover to minimize any shrinking and swelling that could cause differential movement. These materials had consistencies that ranged from soft to very stiff, as inferred from Standard Penetration Test (SPT) blow counts.

SLAB ON-GRADE FOUNDATION

If the slab on-grade is chosen, we recommend excavating the soil to a depth of 9 feet and to a minimum of 4 feet outside the building footprint to remove topsoil, any wet material, and expansive heavy clay. The site should be brought up to construction grade with compacted select fill. Any lean clay that is excavated could be placed aside and used back in the foundation if the engineer approves and if it meets select-fill specifications described below.

Following any excavation, we recommend that the subgrade in all areas be evaluated by a geotechnical engineer or his representative prior to fill placement. The engineer may recommend proof-rolling the areas as a means of evaluating the suitability of the subgrade for fill. Proof-rolling consists of systematically patrolling the area, preferably in perpendicular directions, utilizing a heavily loaded dump truck (minimum 20 tons) or other suitable vehicle approved by the engineer. Any areas which pump or rut excessively, and which cannot be densified by continued rolling, should be undercut to suitable material and properly backfilled. If proof-rolling is not possible, the sub-grade could be evaluated at selected locations with a hand-held Humbolt Cone Penetrometer or equivalent. The measured penetration resistance at each location can be subsequently converted to an in situ bearing capacity for the foundation.

Select structural fill material should then be placed in the foundation area in maximum loose lifts of 8 inches and be compacted to a minimum of 98% of the standard Proctor density (ASTM D-698-91) within 2 percentage points of optimum moisture content. Sufficient field density tests should be conducted to insure compaction requirements are met during construction. As a rule of thumb, we recommend a minimum of two density tests be

performed for each 2000 square feet of surface area per lift. In addition, monitoring of fill construction and compaction will result in minimizing future settlement of the fill and structure. Therefore, we believe that it is important that a qualified geotechnical engineer or certified technician monitor earthwork operations and that this work not be controlled by the earthwork contractor.

It is important that the select structural fill material should consist of a material having a liquid limit of less than 40 percent and a plasticity index between 8 percent and 20 percent. Any excavated materials that include topsoil and any debris, should not serve as select fill and should be disposed of outside the foundation area. Excavated material that meets select fill specifications could be used as fill.

FOUNDATION STRENGTHS

GRADE BEAMS AND SPREAD FOOTINGS: If the foundation system chosen is grade beams and spread footings, they should bear in the controlled, select fill at a minimum depth 24 inches below the finished grade elevation. Minimum depths needed to offset wind forces should be verified by your structural engineer. All foundation members should be reinforced both top and bottom, sufficient to resist differential movement, and the completed foundation system should provide for uniform distribution of applied loads to the bearing soils. After the placement of select fill, the maximum soil pressure under the foundation members should not exceed 2.1 kips per square foot for continuous foundation units or 2.4 kips per square foot for individual spread footings. Foundations sized in accordance with recognized criteria for the above stated allowable soil bearing pressure should provide a factor of safety of 2.0 - 3.0 against ultimate failure of the soil medium with total estimated settlements of 1 inch, more or less.

PILE FOUNDATIONS

PILE FOOTINGS: If heavy loads are anticipated, it is our opinion that the building should be founded on drilled-and-belled end bearing piles at a depth of 35 feet. The maximum soil pressure under the foundation members should not exceed 10 kips per square foot for individual piles. Foundations sized in accordance with recognized criteria for the above stated allowable soil bearing

pressure should provide a factor of safety of 2.0 against ultimate failure of the soil medium.

SITE MAINTENANCE

Note that the soils at this site contain lean clays and silt that can lose strength with increases in moisture content. The soils at this site also contain heavy clays (CH) that can lose strength and shrink and swell with changes in moisture content. It is important to properly control the moisture content of these soils during and after construction. Any foundation soils in exposed excavations that become wet or soft should be removed and replaced prior to construction. Recent inspections of several buildings that have had differential movement have noted gutters exiting beside the foundations and inadequate grades for the swales that should remove surface water. The final site-grading plan should provide for quick runoff of surface waters away from the building foundations in all directions. Any beds for flowers and shrubs should not be boxed in and should be sloped down away from the building foundation. Sprinkler systems located close to the building foundation should be controlled by nearby soil moisture content and not specific time schedules. The landscape plans should insure that large water-consuming trees and shrubs are not located within 50 feet of the perimeter of the foundation members. Where any large trees or stumps are removed or where any plumbing or electrical trenches are cut under the foundation, select-fill material should be used and compacted to specifications.

All foundation recommendations made in this report are contingent upon proper execution of the earthwork requirements noted herein. We believe that it is very important that a qualified geotechnical engineer familiar with working with these type soils be present after excavation and during fill placement. In addition, sufficient field density tests should be taken to insure that the compaction criteria are satisfied and to reduce the possibility of differential settlement at this location.

REPORT LIMITATIONS

The recommendations made in this report are based on the assumption that the borings are representative of the subsurface conditions throughout the site. Therefore, we cannot warrant that our boring logs represent subsurface conditions at other locations or times. If during construction, any unusual or

significantly different conditions are encountered, we should be advised in order to review the changed conditions and subsequently reconsider any of the above recommendations.

Further, we are available to review those portions of the plans and specifications relating to earthwork and foundations for this particular project and request that we be retained to do so in order to determine whether the plans and specifications are consistent with the recommendations contained within this report. In addition, we are available to observe foundation construction procedures, including interpretation of the use of on-site materials and compaction of the structural fill, quality control of concrete placement, and other field observations and quality control measures as required.

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PROJECT: BAPS, INC. RENOVATIONS OF THE SWAMI NARAYAN TEMPLE JACKSON, MS	**CLIENT:** SHS DEVELOPMENT, INC. 115 HOSPITALITY DRIVE FLOWOOD, MS 39232	**DATE:** 6/13/2016 **LAB NO:** 295-16-A **BORE NO:** B-1 **TECHNICIAN:** B.M.\S.W.

SAMPLES: | | **AUGER(ASTM D-1452)** | ■ **TUBE(ASTM D-1587)** | X **PENETRATION TEST(ASTM D-1586)**

DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0		ASPHALT (0 - 3 1/2")							
		RED & GRAY CLAYEY SAND (3 1/2" - 1')		10.9	27.0	13.0	40.8	SC	
	X	TAN & GRAY SILT W/SAND (1' - 2 1/2')	SOFT	25.1	37.0	9.0	71.1	ML	4
		TAN & GRAY HEAVY CLAY W/GRAVEL (2 1/2' - 3 1/2')		16.9	50.0	31.0	83.4	CH	
		WATER @ 3'							
		TAN HEAVY CLAY W/CALCAREOUS (YAZOO CLAY) (3 1/2' - 5')		36.5	88.0	63.0	94.5	CH	
5		TAN & GRAY HEAVY CLAY (YAZOO CLAY) (5' - 10')		41.9	97.0	69.0	97.8	CH	
10									
15									
		METAL BLDG. - FOOTINGS & GRADE BEAMS							
20									
25									
30									

WATER DEPTH 3 **FT.** **AFTER** 0.1 **HRS.** **BORING ELEVATION** 0 **FT.**

WATER DEPTH 0 **FT.** **AFTER** 0 **HRS.** **BORING TERMINATED AT** 10 **FT.**

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PROJECT:	CLIENT:	DATE: 6/13/2016
BAPS, INC. RENOVATIONS OF THE SWAMI NARAYAN TEMPLE JACKSON, MS	SHS DEVELOPMENT, INC. 115 HOSPITALITY DRIVE FLOWOOD, MS 39232	LAB NO: 295-16-A BORE NO: B-2 TECHNICIAN: B.M.\S.W.

SAMPLES: [ ] AUGER(ASTM D-1452) [ ] TUBE(ASTM D-1587) [X] PENETRATION TEST(ASTM D-1586)

DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0		TAN & BROWN SILTY CLAY W/ROOTS (0 - 1')		14.9	28.0	7.0	70.2	CL-ML	
	X	BROWN & TAN LEAN CLAY W/ROOTLETS (1' - 3 1/2')	MEDIUM	18.8	38.0	16.0	96.2	CL	8
	X	BROWN LEAN CLAY (3 1/2' - 5')	SOFT	21.7	35.0	13.0	97.4	CL	3
5		TAN HEAVY CLAY (5' - 7')		25.8	50.0	30.0	97.2	CH	
		TAN HEAVY CLAY W/CALCAREOUS (YAZOO CLAY) (7' - 10')		30.0	71.0	51.0	95.6	CH	
10									
15									
		METAL BLDG. - FOOTINGS & GRADE BEAMS							
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.

WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 10 FT.

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PROJECT:	CLIENT:	DATE: 6/13/2016
BAPS, INC. RENOVATIONS OF THE SWAMI NARAYAN TEMPLE JACKSON, MS	SHS DEVELOPMENT, INC. 115 HOSPITALITY DRIVE FLOWOOD, MS 39232	LAB NO: 295-16-A BORE NO: B-3 TECHNICIAN: B.M.\S.W.

SAMPLES: | | AUGER(ASTM D-1452) | | TUBE(ASTM D-1587) | X | PENETRATION TEST(ASTM D-1586)

DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0		TAN LEAN CLAY (0 - 2 1/2')		15.5	39.0	17.0	94.8	CL	
	X		STIFF						11
		BROWN & GRAY LEAN CLAY (2 1/2' - 3 1/2')		22.8	37.0	16.0	96.8	CL	
	X	BROWN & TAN LEAN CLAY (3 1/2' - 5')	MEDIUM	25.5	46.0	25.0	93.4	CL	6
5		TAN & GRAY HEAVY CLAY (YAZOO CLAY) (5' - 7')		26.5	64.0	45.0	97.6	CH	
		TAN & GRAY HEAVY CLAY W/CALCAREOUS (7' - 15') (YAZOO CLAY)		42.2	122.0	96.0	99.1	CH	
15		TAN & GRAY HEAVY CLAY (15' - 25')		36.5			92.2	CH	
25		TAN HEAVY CLAY W/QUARTZ (25' - 30')		40.2			96.8	CH	
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.

WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 41.5 FT.

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PROJECT:	CLIENT:	DATE: 6/13/2016
BAPS, INC. RENOVATIONS OF THE SWAMI NARAYAN TEMPLE JACKSON, MS	SHS DEVELOPMENT, INC. 115 HOSPITALITY DRIVE FLOWOOD, MS 39232	LAB NO: 295-16-A BORE NO: B-3 TECHNICIAN: B.M.\S.W.

SAMPLES: | AUGER(ASTM D-1452) | TUBE(ASTM D-1582) | X | PENETRATION TEST(ASTM D-1596)

DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
30		TAN HEAVY CLAY (YAZOO CLAY) (30' - 35')		37.7			94.2	CH	
35		TAN & BLUE YAZOO CLAY (35' - 40')		42.7			98.2	CH	
40		BLUE UNWEATHERED YAZOO CLAY (40' - 41 1/2')		37.8			99.8	CH	
45									
50		- ENTRANCE \ CONCRETE STRUCTURE \ CONCRETE ROOF PILE FOUNDATION -							
55									
60									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.

WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 41.5 FT.

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PROJECT:	CLIENT:	DATE: 6/13/2016
BAPS, INC. RENOVATIONS OF THE SWAMI NARAYAN TEMPLE JACKSON, MS	SHS DEVELOPMENT, INC. 115 HOSPITALITY DRIVE FLOWOOD, MS 39232	LAB NO: 295-16-A BORE NO: B-4 TECHNICIAN: B.M.\S.W.

SAMPLES: | | AUGER(ASTM D-1452) | ■ TUBE(ASTM D-1587) | X PENETRATION TEST(ASTM D-1586)

DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0		BROWN & TAN LEAN CLAY (0 - 1')		12.4	41.0	18.0	88.0	CL	
	X	TAN & GRAY LEAN CLAY (1' - 3 1/2')	STIFF	21.1	45.0	23.0	95.2	CL	10
	X	TAN & GRAY LEAN CLAY (3 1/2' - 5')	STIFF	19.4	42.0	24.0	96.4	CL	13
5		TAN HEAVY CLAY W/CALCAREOUS (YAZOO CLAY) (5' - 8 1/2')		34.7	82.0	57.0	96.5	CH	
		TAN & GRAY HEAVY CLAY (YAZOO CLAY) (8 1/2' - 10')		36.3	119.0	86.0	95.4	CH	
10									
15									
		METAL BUILDING - FOOTINGS & GRADE BEAMS							
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.

WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 10 FT.

BAPS, INC. RENOVATIONS OF
THE SWAMI NARAYAN TEMPLE
JACKSON, MS

DATE: 6/13/2016

LAB NO.: 295-16-A

DEAD LOADS CALCULATED:

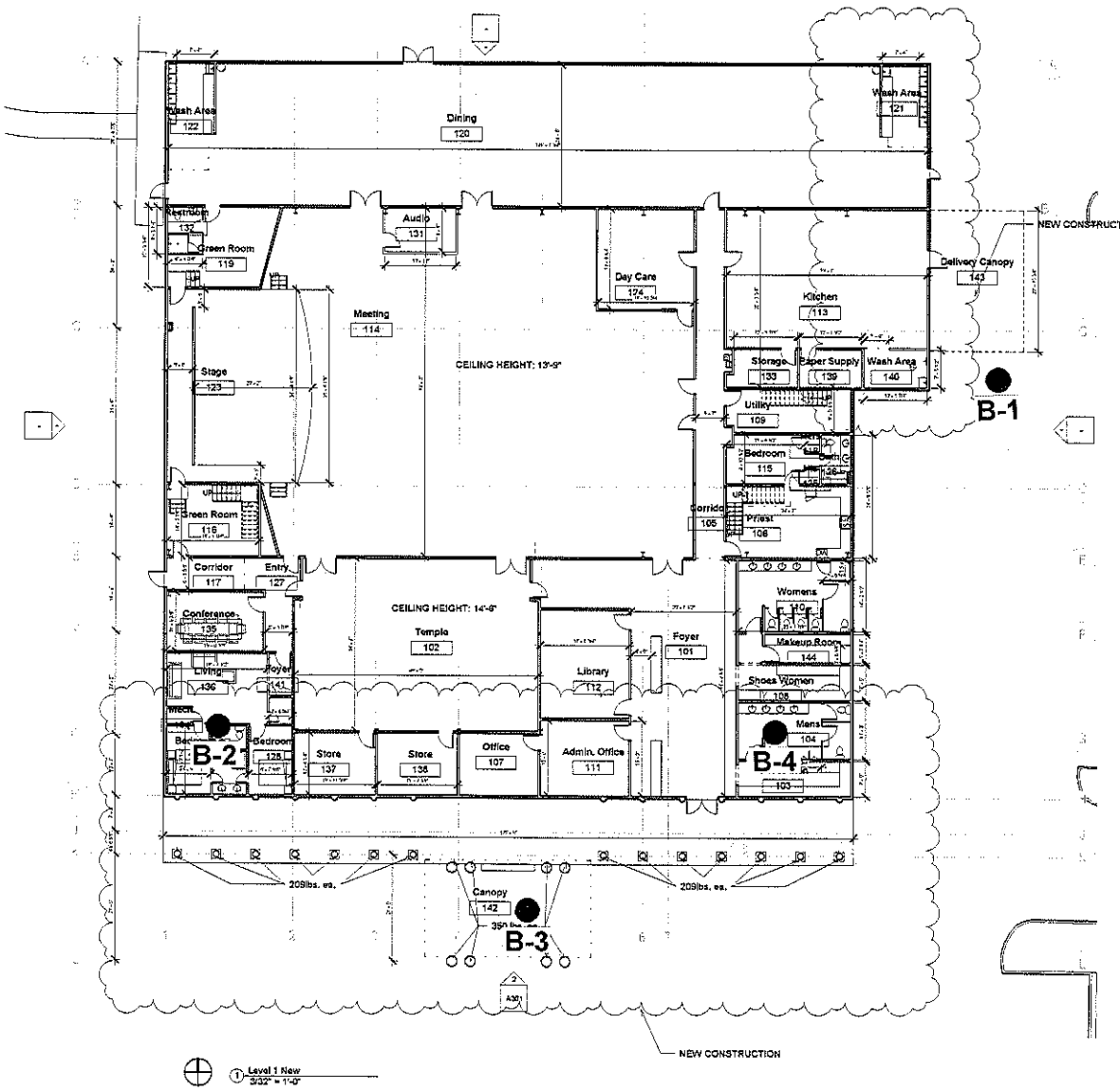
MAIN ROOF:
1. THREE DOMES: 6,600 X 3 = 19,800 LBS.
2. FOUR SMALL DOMES: 480 X 4 = 1,920 LBS.
3. PARAPET DECORATION: 135 FT. X 12 = 1,620 LBS.

PORCH ROOF:
1. SIX DOMES: 600 x 6 = 3,600 LBS.
2. PARAPET DECORATION: 135 X 12 = 1,620 LBS.

PORCH:
1. FOURTEEN COLUMNS: 209 X 14 = 2,926 LBS.
2. FOURTEEN FAKE WINDOWS: 167 X 14 = 2,818 LBS.

ENTRANCE CANOPY ROOF:
1. MAIN DOME: 1,230 LBS.
2. PARAPTET DECORATION: 92 X 14 = 1,288 LBS.

ENTRANCE CANOPY:
1. EIGHT COLUMNS: 350 X 8 = 2,800 LBS.
2. FOUR COLUMN DECORATION: 99 X 4 = 396 LBS.



1 Level 1 New
3/32" = 1'-0"

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REVISIONS		
No.	Date	Description
1	Date 1	Revision 1

KEY PLAN

BAPS, Inc.
Renovation of the Swami Narayan Temple

2389 Greenway Drive
Jackson, MS 39204

Drawing Title
First Floorplan New

Phase
Schematic Design

Project No.	15-072
Prepared by	Author
Checked by	Checker
Date	May 11, 2016

Sheet No.
A103

Plotted for
Owner Use ONLY

5/12/2016 8:35:01 AM

Renovation of the Swami Narayan Temple