GENERAL NOTES:

1. THE FOLLOWING SPECIFICATIONS ARE AN OUTLINE OF MINIMUM MATERIAL REQUIREMENTS AND THEIR APPLICATION. MANUFACTURER SPECIFICATION AND LOCAL CODE REQUIREMENTS, WHEN IN EXCESS OF MINIMUM SPECIFICATION, SHALL CONTROL. IT IS THE CONTRACTOR'S RESPONSIBILITY TO REVIEW AND SUBMIT ALL SHOP DRAWINGS AND REPORT ALL DOCUMENT DISCREPANCIES TO THE STRUCTURAL ENGINEER PRIOR TO FABRICATION OR ERECTION.

2. AT CONSTRUCTION ISSUE, THESE DRAWINGS REPRESENT STRUCTURAL COMPONENTS IN THEIR FINAL AND FINISHED STATE. CONSTRUCTION PROCEDURES, METHODS, SAFETY PRECAUTIONS OR MECHANICAL REQUIREMENTS USED TO ERECT THEM ARE THE SOLE RESPONSIBILITY OF THE GENERAL CONTRACTOR OR SUBCONTRACTOR DOING THE WORK.

CONCRETE NOTES AND SPECIFICATIONS:

1. ALL CONCRETE WORK SHALL CONFORM TO THE "A.C.I. BUILDING CODE", ACI 318 AND ACI 301, LATEST EDITION.

2. DETAILING, FABRICATION AND PLACING OF REINFORCING STEEL SHALL BE IN ACCORDANCE WITH ACI 315-90, "ACI DETAILING MANUAL - 1990".

3. UNLESS OTHERWISE NOTED, ALL REINFORCING BARS #4 AND LARGER SHALL CONFORM TO ASTM A-615 GRADE 60 (60,000 PSI YIELD) AND ALL #2 AND #3 BARS SHALL CONFORM TO GRADE 40 (40,000 PSI YIELD). REINFORCING SHALL BE FREE FROM OIL, DIRT AND OTHER MATERIALS THAT WOULD REDUCE THE BOND WITH THE CONCRETE.

4. WELDED WIRE REINFORCING (WWR) SHALL CONFORM TO ASTM A-185. WELDED WIRE REINFORCING SHALL BE CHAIRED TO MAINTAIN THE REINFORCING AT ONE-THIRD THE DEPTH BELOW THE TOP SURFACE DURING CONCRETE PLACEMENT. SUPPORTS SHALL BE AT 2'-0" O.C.

5. UNLESS OTHERWISE NOTED, CONCRETE PROTECTION FOR REINFORCING SHALL BE AS SPECIFIED IN THE "A.C.I. BUILDING CODE", (ACI 318 LATEST EDITION).

6. CONCRETE STRENGTH AND PROTECTION FOR REINFORCEMENT OF POURED-IN-PLACE MEMBERS: SEE SECTION 7.7 ACI 318 LATEST EDITION.

A. STRUCTURAL ELEMENT	MINIMUM COVER (INCHES)	CONCRETE STRENGTH (PSI) AT 28 DAYS	W/C RATIO
FOOTINGS	3" ALL SURFACES	3000	0.52
GRADE BEAMS	3" BOTTOM, 2" SIDES, 1 1/2" TOP	3000	0.52
SLAB ON GRADE	1 1/2" TOP & BOTTOM	3000	0.52
RETAINING WALLS	1" TOP &	3000	0.52
& ELEVATOR PITS	2" EXTERIOR, 3/4" INTERIOR		

PORTLAND CEMENT SHALL CONFORM TO ASTM C150, TYPE I. MINIMUM OF 5 SACKS OF CEMENT PER CUBIC YARD.

7. FLYASH MAY BE USED TO REPLACE A PORTION OF THE PORTLAND CEMENT. THE RATIO OF FLYASH TO THE TOTAL OF THE FLYASH AND CEMENT IN A MIX SHALL NOT EXCEED 20%. FLYASH SHALL CONFORM TO ASTM C618, TYPE C OR F.

8. NO WATER SHALL BE ADDED TO THE CONCRETE AT THE JOBSITE. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO COORDINATE WITH THE CONCRETE SUPPLIER TO ENSURE A PUMPABLE AND WORKABLE MIX WITHOUT THE ADDITION OF WATER AT THE JOBSITE. THE USE OF PLASTICIZERS, RETARDANTS AND OTHER ADDITIVES SHALL BE AT THE OPTION OF THE CONTRACTOR SUBJECT TO THE APPROVAL OF THE STRUCTURAL ENGINEER. FOLLOW THE RECOMMENDATIONS OF THE MANUFACTURER FOR THE PROPER USE OF ADDITIVES. THE USE OF CALCIUM CHLORIDE OR OTHER CHLORIDE BEARING SALTS SHALL NOT BE PERMITTED.

9. CONCRETE SLUMP TESTS SHALL BE MADE BEFORE AND AFTER THE ADDITION OF ADMIXTURES AND MAY BE TAKEN AT THE BACK OF THE TRUCK. CONCRETE FOR THE PREPARATION OF TEST CYLINDERS SHALL BE TAKEN FROM THE HOSE END FOR CONCRETE PLACED BY PUMP.

10. ALL REINFORCING STEEL MARKED "CONTINUOUS" SHALL BE LAPPED PER THE "REINFORCING SPLICE SCHEDULE" AT SPLICES AND AROUND CORNERS OR INTERSECTIONS WITH A STANDARD 90 DEGREE BEND ON CORNER BARS. LAP TOP BARS AT CENTER OF SPAN: LAP BOTTOM BARS AT SUPPORTS. LAP WELDED WIRE MESH ONE FULL MESH AT SIDE AND END LAPS.

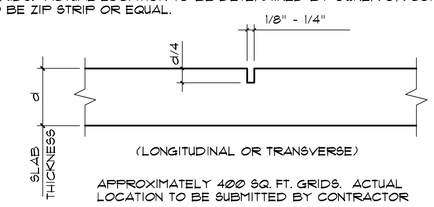
REINFORCING SPLICE SCHEDULE

REINF. SIZE	CONCRETE STRENGTH (PSI) 3000	4000	5000	6000	7000
#3	22"	19"	17"	16"	14"
#4	29"	25"	23"	21"	19"
#5	36"	31"	28"	26"	24"
#6	43"	37"	34"	31"	28"
#7	63"	54"	49"	45"	41"
#8	72"	62"	56"	51"	47"
#9	81"	70"	63"	57"	53"
#10	91"	79"	71"	64"	60"
#11	101"	87"	78"	71"	66"

11. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR ANY OTHER ADDITIONAL SLEEVES, ANCHORS, VENT OPENINGS, ETC., NOT SHOWN ON STRUCTURAL PLANS THAT MIGHT BE REQUIRED.

12. PLACE CONCRETE IN A MANNER SO AS TO PREVENT SEGREGATION OF THE MIX. DELAY FLOATING AND TROWELING OPERATIONS UNTIL CONCRETE HAS LOST SURFACE WATER SHEEN OR ALL FREE WATER. DO NOT SPRINKLE FREE CEMENT ON THE SLAB SURFACE. FINISHING OF SLAB SURFACES SHALL COMPLY WITH THE RECOMMENDATIONS OF ACI 302.1 AND 304.

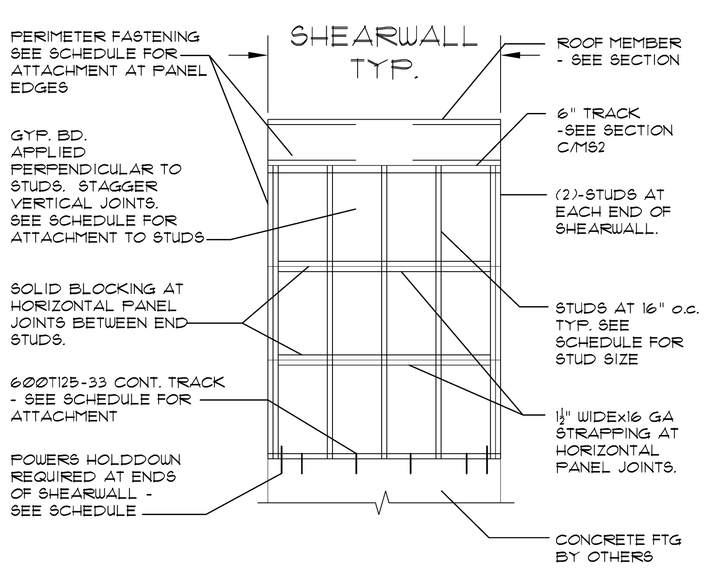
13. UNLESS NOTED OTHERWISE ON PLAN, CONTROL JOINTS TO BE LOCATED AT APPROXIMATELY 400 SQ. FT. GRIDS. ACTUAL LOCATION TO BE DETERMINED BY OWNER OR CONTRACTOR. CONTROL JOINT TO BE ZIP STRIP OR EQUAL.



TYP. CONTROL JOINT DETAIL (SLAB ON GRADE)

SHEAR WALL SCHEDULE

SHEARWALL TYPE	SHEATHING	ATTACHMENT	SILL BOLT
SHEARWALL S-1	1/2" DENS-GLASS GOLD (EXTERIOR SIDE) 5/8" GYPSUM BOARD (INTERIOR SIDE)	ATTACH ALL EDGES TO TOP PLATES, SILL PLATES, STUDS, AND BLOCKING, AT PANEL EDGES w/ #6 TYPE W x 1 5/8" LG. DRYWALL SCREWS AT 4" O.C. ATTACH TO STUDS AT 8" O.C. IN FIELD OF PANELS ATTACH ALL EDGES TO TOP PLATES, SILL PLATES, AND STUDS AT PANEL EDGES W/#6 TYPE Wx1 5/8" LG. DRYWALL SCREWS @ 7" O.C. ATTACH TO STUDS IN FIELD OF PANELS @ 7" O.C.	3/4" DIA. POWERS EXP. ANCHOR W/ 3" EMBED. EA. END 3/4" DIA. BOLTS @ 32" O.C. POWERS EXP. BOLTS OR EQUAL W/ 3" EMBED.



FOUNDATION NOTES:

1. FOOTINGS SHALL BEAR ON SOIL SUITABLE FOR SUPPORTING 3,000 P.S.F. NET ALLOWABLE BEARING AT PAD FOOTINGS AND 2,500 PSF AT WALL FOOTINGS.
   IF QUESTIONABLE SOIL IS ENCOUNTERED, THE STRUCTURAL ENGINEER SHALL BE NOTIFIED IMMEDIATELY. RE: REPORT PREPARED BY BURNS, COOLEY DENNIS INC. DATED SEPT 7, 2006. ALL RECOMMENDATIONS THEREIN THAT RELATE TO THE WORK SHOWN ON THESE DRAWINGS MUST BE FOLLOWED.

2. EXCAVATION FOR FOOTINGS SHALL BE NEAT.

3. FOOTINGS SHALL BE POURED IMMEDIATELY AFTER EXCAVATION.

4. ALL REINFORCING STEEL MARKED "CONTINUOUS" SHALL BE LAPPED PER THE "REINFORCING SPLICE SCHEDULE" AT SPLICES AND AROUND CORNERS OR INTERSECTIONS WITH A STANDARD 90 DEGREE BEND ON CORNER BARS. LAP WELDED WIRE MESH ONE FULL MESH AT SIDE AND END LAPS.

5. LAP TOP BARS AT CENTER OF SPAN: LAP BOTTOM BARS AT SUPPORTS.

6. THERE SHALL BE NO PLUMBING LINES RUNNING PARALLEL TO, WITHIN OR UNDER ANY FOUNDATION BEAM.

7. PLACE A 10 MIL VAPOR RETARDER OF POLYETHYLENE UNDER ALL CONCRETE SLABS.

REINFORCED MASONRY

1. ALL REINFORCED CONCRETE MASONRY MATERIALS AND CONSTRUCTION SHALL CONFORM TO THE FOLLOWING:

ACI 530.1-05/ASCE 6-05/TMS 602-05 "SPECIFICATIONS FOR MASONRY STRUCTURES"
ACI 530-05/ASCE 5-05/TMS 402-05 BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES.

2. CONCRETE BLOCK SHALL CONFORM TO ASTM C-90. THE REQUIRED STRENGTH ON THE NET CROSS SECTIONAL AREA OF THE CONCRETE BLOCK SHALL BE 1,900 PSI.

3. MORTAR SHALL BE TYPE M OR S, CONFORMING TO ASTM C270.

4. GROUT SHALL CONFORM TO ASTM C476. GROUT MAY BE PLACED BY THE "HIGH LIFT" METHOD, CONFORMING TO THE GROUTING PATTERNS REQUIRED BY THE CONTRACT DRAWINGS.

5. THE REQUIRED MINIMUM 28-DAY COMPRESSIVE STRENGTH OF THE COMBINATION OF CONCRETE BLOCK, GROUT AND MORTAR ON THE NET AREA OF THE WALL (F'm) SHALL BE A MINIMUM OF1,500 PSI.

6. THE ACTUAL 28-DAY COMPRESSIVE STRENGTH OF THE CONCRETE MASONRY ASSEMBLY (F'm) SHALL BE DETERMINED AS DESCRIBED IN SECTION 1.6 OF ACI 530.1-05/ASCE 6-05/TMS 602.05.

7. ALL CONCRETE BLOCK MASONRY UNITS SHALL BE LAID IN RUNNING BOND, UNLESS NOTED OTHERWISE.

8. MASONRY BLOCK CELLS CONTAINING VERTICAL REINFORCING SHALL BE GROUTED SOLID. FILLING CELLS WITH MORTAR IS UNACCEPTABLE.

9. THE BASE OF EACH CELL, IN WHICH A BAR IS PLACED, MUST HAVE A CLEANOUT HOLE.
10. REINFORCING STEEL SHALL CONFORM TO ASTM A615 GRADE 60. VERTICAL REINFORCING BARS SHALL

BE LAPPED A MINIMUM OF 40 BAR DIAMETERS.

11. PROVIDE CONTINUOUS REINFORCED BOND-BEAMS IN ALL REINFORCED MASONRY WALLS AT TOPS OF WALLS, IMMEDIATELY BELOW STEEL BEARINGS, AND WHEREVER CALLED FOR IN CONTRACT DRAWINGS. BOND

BEAMS AT TOP OF WALL SHALL BE CONTINUOUS AT MASONRY CONTROL JOINTS. OTHER BOND BEAMS SHALL NOT BE CONTINUOUS AT MASONRY CONTROL JOINTS. BOND BEAM REINFORCING SHALL EXTEND INTO AND BE CONTINUOUS WITH ALL INTERSECTING BOND BEAMS.

12. REINFORCED MASONRY WALLS SHALL HAVE #9 GAUGE (TRUSS TYPE) HORIZONTAL REINFORCING AT SPACING AS NOTED ON THE CONTRACT DRAWINGS, BUT AT A MAXIMUM OF 16" O.C. VERTICALLY.

13. FILL CORES OF MASONRY UNDER ALL BEARING PLATES FOR A WIDTH EQUAL TO THREE TIMES THE BEARING PLATE LENGTH FOR THREE COURSES BELOW BEARING, OR AS SHOWN ON DRAWINGS.

14. IN NON-LOAD BEARING WALLS PROVIDE AND INSTALL ONE LINTEL FOR EACH 4" OF WALL THICKNESS ACCORDING TO THE FOLLOWING SCHEDULE:

OPENING	LINTEL
3'-0"	3 1/2X3 1/2X 5/16
4'-0"	L4X3 1/2X5/16
5'-0"	L4X3 1/2X5/16
6'-0"	L5X3 1/2X5/16
7'-0"	L6X3 1/2X5/16

LINTELS SHALL BEAR A MINIMUM OF 6" ON EACH SIDE OF OPENING. LONG LEG OF ANGLE SHALL BE VERTICAL.

14. PROVIDE 7 DAY CURING OF SLAB IMMEDIATELY AFTER FINISHING USING ONE OF THE FOLLOWING METHODS:
    A. CONTINUOUSLY WATERED BURLAP
    B. WATERPROOF MEMBRANES
    C. SPRAYED-ON LIQUID MEMBRANE

15. PROTECT THE CONCRETE SURFACE BETWEEN FINISHING OPERATIONS ON HOT, DRY DAYS OR ANY OTHER TIME THAT PLASTIC SHRINKAGE CRACKS COULD DEVELOP BY USING WET BURLAP, PLASTIC MEMBRANE OR FOGGING. PROTECT CONCRETE SLAB AT ALL TIMES FROM RAIN, HAIL OR OTHER INJURIOUS EFFECTS.

16. UNLESS SPECIFIED, CONCRETE MUST REACH THE FOLLOWING PERCENTAGES OF ITS 28-DAY COMPRESSIVE STRENGTH (F'c), BEFORE FORMS MAY BE REMOVED.

    WALLS, COLUMNS, & BEAM SIDES ——— 40%
    JOIST PANS & BEAM BOTTOMS (IF RESHORED) ——— 70%
    SHORING FOR FLOOR SYSTEMS (IF NOT RESHORED) ——— 85%

17. RESHORING WHEN REQUIRED, TO EXTEND AT LEAST TWO FLOORS BELOW FLOOR SUPPORTING FALSEWORK (OR GROUND FLOOR). LAYOUT AND PROCEDURE TO BE SUBMITTED TO STRUCTURAL ENGINEER FOR APPROVAL.

18. AN INDEPENDENT CERTIFIED TESTING LABORATORY SHALL VERIFY AND PROVIDE REPORTS CERTIFYING THE FOLLOWING:
    A. CONCRETE PLANT BATCH TICKETS FOR EACH TRUCK VERIFY THAT THE CONCRETE MATCHES THE APPROVED DESIGN MIX.
    B. CONCRETE SLUMP IS IN ACCORDANCE WITH APPROVED DESIGN MIX.
    C. CONCRETE PLACEMENT OPERATIONS ARE IN ACCORDANCE WITH ACI SPECIFICATIONS.
    D. CONTROL JOINTS ARE INSTALLED WITHIN THE ACI TIME ALLOWANCE.
    E. PROPER CURING METHODS ARE BEING UTILIZED.

19. NO CONCRETE SHALL BE PLACED OUTSIDE OF THESE SPECIFICATIONS WITHOUT THE OWNER'S PRIOR APPROVAL. ANY ITEMS NOT IN COMPLIANCE WITH THE OUTLINED SPECIFICATION SHALL BE REPORTED TO THE OWNER AND STRUCTURAL ENGINEER WITHIN 24 HOURS.

20. CONSTRUCTION VEHICLE LOADS SHALL NOT BE PERMITTED ON ELEVATED SLABS AT ANY TIME.

21. ALL RETAINING WALLS TO BE SHORED UNTIL UPPER SLAB IS IN PLACE AND HAS REACHED 70% OF ITS DESIGN STRENGTH OR THE RETAINING WALL HAS REACHED 100% OF ITS DESIGN STRENGTH. PROVIDE GRANULAR BACKFILL AND PERFORATED DRAIN PIPE CONNECTED TO SITE DRAINAGE, RE: CIVIL PLAN.

STRUCTURAL STEEL

1. STRUCTURAL STEEL WORK SHALL CONFORM TO THE FOLLOWING:

AISC - "SPECIFICATION FOR DESIGN, FABRICATION AND ERECTION OF STEEL FOR BUILDINGS".
AISC - "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
AWS D1.1 - "STRUCTURAL WELDING CODE - STEEL".
AISC - "STRUCTURAL STEEL DETAILING MANUAL".

2. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING:

HOT ROLLED WIDE-FLANGE AND WT SHAPES ASTM A992 (Fy=50 KSI)
ALL OTHER STRUCTURAL SHAPES AND PLATESTM A36 (Fy=36KSI)
HOLLOW STRUCTURAL SECTIONS (HSS) - ASTM A500 GRADE B (Fy=46 KSI)
HIGH STRENGTH BOLTS -ASTM A325N¾"ø
ANCHOR BOLTS - ASTM F1554 GR. 36.

3. PROVIDE 2 MIL. THICK RED OR GREY OXIDE PRIMER ON ALL STEEL SURFACES UNLESS NOTED OTHERWISE.

4. ANCHOR BOLTS SHALL BE PRESET WITH TEMPLATES AT REQUIRED LOCATIONS.

5. LEVELING PLATES AND BEARING PLATES SHALL BE SET IN FULL BED OF NON-SHRINK GROUT.

6. CONNECTIONS MAY BE EITHER BOLTED OR WELDED AT THE FABRICATOR'S OPTION. BOLTED CONNECTIONS SHALL BE AS FOLLOWS:

MINIMUM BOLT DIAMETER: 3/4"

SHEAR CONNECTIONS FOR MOMENT CONNECTED MEMBERS: FRICTION TYPE HIGH STRENGTH BOLTS IN SINGLE OR DOUBLE SHEAR.
SHEAR CONNECTIONS FOR NON-MOMENT CONNECTED MEMBERS: BEARING TYPE HIGH STRENGTH BOLTS IN SINGLE OR DOUBLE SHEAR.

7. ALL BEAM CONNECTIONS NOT DETAILED, SHALL SUPPORT 1/2 THE TOTAL UNIFORM LOAD CAPACITY FOR THE GIVEN BEAM AND SPAN OR THE INDICATED REACTION, WHICHEVER IS GREATER. CONNECTIONS SHALL GENERALLY FOLLOW THE TYPES SHOWN IN THE "AISC MANUAL OF STEEL CONSTRUCTION", TABLE II, III, OR X.

8. WELDS SHALL FULLY DEVELOP STRENGTH OF THE MATERIALS BEING WELDED, UNLESS NOTED OTHERWISE, EXCEPT THAT FILLET WELDS SHALL BE A MINIMUM 3/16".

9. WELDED CONNECTIONS SHALL BE MADE BY APPROVED CERTIFIED WELDERS USING FILLER METAL CONFORMING TO E70XX.

10. CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SUPPORTS TO HOLD STRUCTURAL STEEL FRAMING SECURELY IN POSITION. TEMPORARY BRACING SHALL REMAIN UNTIL THE PERMANENT LATERAL BRACING HAS BEEN INSTALLED AND THE CONCRETE FOR FLOOR SLABS HAS ATTAINED 75 ITS REQUIRED STRENGTH.

11. STRUCTURAL STEEL FRAMING SHALL BE TRUE AND PLUMB BEFORE FINAL BOLTING OR WELDING OF CONNECTIONS.

12. CONTRACTOR SHALL NOT MODIFY OR CUT ANY STRUCTURAL STEEL WITHOUT WRITTEN APPROVAL FROM THE ENGINEER.

13. CONTRACTOR SHALL FIELD TOUCH UP ALL ABRASIONS, BURNS, AND SIMILAR DEFECTS IN PAINT OF THE STRUCTURAL STEEL, JOISTS, AND STEEL DECK.

14. AT OWNERS OPTION, PROVIDE ACCESS FOR AN INDEPENDENT TESTING AGENCY, HIRED BY THE OWNER, TO MAKE THE FOLLOWING INSPECTIONS:

-BOLT INSTALLATION
-VISUAL INSPECTION OF FILLET WELDS.
-ULTRASONIC TESTING OF FULL-PENETRATION WELDS (TEST REPORTS FROM ALL SHOP-FABRICATED, FULL PENETRATION WELDS SHALL BE SUBMITTED TO ENGINEER PRIOR TO RELEASE FOR SHIPPING).

STAIR & HANDRAIL NOTES:

ALL STAIRS AND HANDRAILS SHALL BE DESIGNED BY A REGISTERED STRUCTURAL ENGINEER BASED ON THE FOLLOWING DESIGN CRITERIA:

1. STAIR STRINGERS, TREADS AND RISERS SHALL BE DESIGNED TO SUPPORT 100 PSF LIVE LOAD.

2. INDIVIDUAL STAIR TREADS SHALL BE DESIGNED TO SUPPORT A 300 POUND CONCENTRATED LOAD PLACED IN A POSITION THAT WOULD CAUSE MAXIMUM STRESS.

3. THE TOP RAIL OF HANDRAILS SHALL BE DESIGNED TO WITHSTAND A LOAD OF 50 PLF APPLIED HORIZONTALLY AT RIGHT ANGLES, OR A 200 POUND CONCENTRATED LOAD APPLIED IN ANY DIRECTION. INTERMEDIATE RAILS, PANEL FILLERS AND THEIR CONNECTIONS SHALL BE DESIGNED TO WITHSTAND A LOAD OF 50 PSF APPLIED HORIZONTALLY AT RIGHT ANGLES OVER THE ENTIRE TRIBUTARY AREA, INCLUDING OPENINGS AND SPACES BETWEEN RAILS.

4. ENDS OF HANDRAILS MUST PROVIDE A MINIMUM 12" OF ADDITIONAL LENGTH, PARALLEL TO GROUND/FLOOR EXTENDING BEYOND RAMP/STAIR TOP AND BOTTOM. ENDS OF HANDRAILS MUST BE ROUNDED AND RETURN SMOOTHLY TO FLOOR, WALL OR POST.

COLD FORMED STRUCTURAL STEEL MEMBERS INCLUDING LIGHT GAUGE STEEL

1. ALL COLD FORMED STRUCTURAL MATERIAL AND WORK SHALL CONFORM TO THE LATEST EDITIONS OF THE FOLLOWING:

AISI - "SPECIFICATIONS FOR THE DESIGN OF LIGHT-GAUGE COLD-FORMED STEEL STRUCTURAL MEMBERS"
AISC - "SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS"

2. COLD FORMED STEEL SHALL CONFORM TO THE FOLLOWING:

LIGHT GAUGE STUDS, JOISTS, TRACKS AND ACCESSORIES: ASTM A 653/653 M, G60 GALVANIZED.

ASTM A653 COLD FORMED GALVANIZED LIGHT GAUGE FRAMING MEMBERS

1. STUDS AND JOISTS SHALL BE CHANNEL SHAPED WITH LIPPED FLANGES, PUNCHED WEB, SIZE, GAUGE AND GRADE AS SHOWN ON THE DRAWINGS, MINIMUM.

2. TRACKS SHALL BE CHANNEL SHAPED SOLID WEB, DEPTH COMPATIBLE WITH STUDS GAUGE AND GRADE AS SHOWN ON THE DRAWINGS, MINIMUM.

3. FRAMING ACCESSORIES SHALL HAVE A MINIMUM YIELD STRENGTH OF 65 KSI.

4. SCREWS SHALL BE CORROSION RESISTANT, SELF DRILLING PAN OR HEX WASHER HEAD.

5. POWDER ACTUATED FASTENERS: AISI 1062 OR 1065 STEEL, MINIMUM CORE HARDNESS 50 TO 54 HRC AND ZINC PLATED IN ACCORDANCE WITH ASTM B 633. DIAMETER AND LENGTH AS SHOWN ON THE DRAWINGS.

6. WIND DESIGN LOADING FOR SECONDARY FRAMING PER ASCE 7-02 FOR COMPONENTS AND CLADDING. WIND DESIGN SPEED

7. MATERIAL SUPPLIER IS RESPONSIBLE FOR MEMBER DESIGN. CALCULATIONS MUST BE INCLUDED WITH SHOP DRAWINGS FOR APPROVAL.

RICHARD F. STELDT, ARCHITECT
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