COMFORT SUITES

SECTION 00100 – INSTRUCTIONS TO BIDDERS

PART 1 - GENERAL

RELATED DOCUMENTS:

Drawings and general provisions of Contract, including General and Supplementary Conditions and Division-1 Specification sections, apply to work of this section.

INSTRUCTIONS TO BIDDERS:

AIA documents A701, 1997 Edition, is hereby incorporated in and made a part of this Project Manual.

END OF SECTION 00100

SECTION 00220 - SOIL INVESTIGATION DATA

PART 1 - GENERAL

Incorporated Information: The following information incorporated in the Project Manual is information supplied by the Owner for reference, and is not part of the Contract.

Borings: Soil borings have been made at the site to assist in the design process. Soil data and/or logs which are attached and in the Documents are for the information and convenience of Bidders.

Data Use Limitations: Because the sub-surface conditions indicated by the borings are a sampling in relation to the entire construction area and for other reasons, the Owner, the Architect, and the firm reporting the sub-surface conditions based on the borings, do not implicitly or expressly warrant the conditions adjacent to the borings or below the depths of the borings, or surrounding them, or that the strata logged from the borings are necessarily typical of the entire site.

Bidder/Sub-Bidder Resonsibility: Persons using soil information described in this section shall accept full responsibility for its use in preparing Bids/Subbids and in obtaining additional soil information that may be required.

Extra Payment: No consideration for extra payment will be given for conditions that occur which could or could not have been anticipated from the soil information. If conditions occur resulting in additional work which could have or could not have been anticipated or reasonably inferred from the soil information, the Conditions of the Contract shall apply.

Soil Investigation Data: Reproduced on the following pages are soil boring logs and report entitled:

Subsurface Investigation
For
Comfort Suites
Berryman Road
Warren County
Vicksburg, Mississippi

dated: November, 2006

Prepared by: GEOTECHNICAL ASSOCIATES NETWORK, LLC
110 Beechtree Road
Vicksburg, Mississippi 39183-7464

END OF SECTION

LADNER

JACKSON - HATTIESBURG - GULFPORT
(601) 362-5421 (601) 544-5782 (228) 604-2527

SUBSURFACE INVESTIGATION

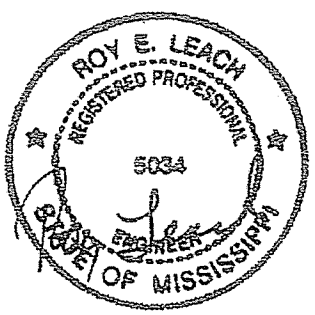
FOR

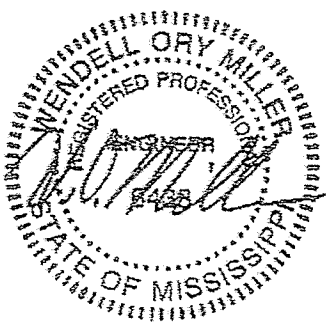
COMFORT SUITES
BERRYMAN ROAD
WARREN COUNTY
VICKSBURG, MISSISSIPPI

NOVEMBER 2006

BY

GEOTECHNICAL ASSOCIATES NETWORK, LLC
110 BEECHTREE ROAD
VICKSBURG, MISSISSIPPI 39183-
7464





SUBSURFACE INVESTIGATION FOR
COMFORT SUITES
BERRYMAN ROAD
WARREN COUNTY
VICKSBURG, MISSISSIPPI

PURPOSE

The purposes of this subsurface investigation are:

a. To determine the general characteristics of the subsurface soils within the area of the proposed construction;

b. To determine by field and laboratory testing, the physical characteristics of the foundation soils and the soil samples collected; and

c. To make recommendations for foundation construction at this particular location.

FIELD INVESTIGATION

Nine subsurface borings were made at the site of the proposed construction. The borings were advanced with a truck mounted, powered, continuous flight auger. Auger cuttings of the soil medium were collected at changes in strata, and at intervals not exceeding five feet in depth. All samples taken were stored in sealed containers for later classification and testing. In addition, standard penetration resistance values (see ASTM D-1586-84) were determined and recorded on the boring logs for the various materials encountered. The Standard Penetration Test (SPT) gives an indication of the consistency and the in-place shear strength of cohesive soils and the relative density of cohesionless soils by recording the number of blows required, by a 140-pound hammer falling 30 inches, to drive a 2-inch O.D. splitspoon sampler one foot. No water levels were observed or recorded during drilling and sampling operations at the site.

LABORATORY INVESTIGATION

Laboratory testing of selected soil samples included visual classification, Atterberg limits on cohesive soils with determination of the plasticity index (PI), grain size analyses, and in situ moisture contents. Atterberg limits (ASTM D-4318-93) were run on the clayey soils in an effort to estimate the susceptibility of these soils to shrink and swell with changes in moisture content. Liquid and plastic limits were run on samples selected from some of the various materials encountered. The liquid limit (LL) is the moisture content at which a soil changes from a plastic state to a viscous liquid state. The plastic limit is the moisture content at which a soil changes from a solid state to a plastic state. The plasticity index (PI) is the numerical difference between the liquid limit and the plastic limit and is indicative of the relative activity or sensitivity of a cohesive soil.

Grain size analyses (ASTM D-422-63) were conducted on representative samples of the various soils encountered to determine the particle size distribution of materials comprising the strata. Results of these tests were utilized in classifying the soils by the Unified Soil Classification System and in estimating the California Bearing Ratio (CBR) of the prospective subgrade soils. Classifications for each of the soil samples are shown on the boring logs and test results attached to this report.

To aid in the general interpretation of the soil conditions at the site, in situ moisture contents were determined for samples selected from the various soils encountered. This determination was made possible by placing extracted samples in sealed containers immediately upon removal from each interval. The results of these and other tests are recorded on the attached boring logs.

SUBSURFACE CONDITIONS

The site of the proposed construction is in Section 22, Township 16 North, Range 4 East, on Berryman Road in Vicksburg, Warren County, Mississippi. This site lies in the Gulf Coastal Plain of North America in the Loess Hills physiographic province of Mississippi. Structurally, the site is in the Mississippi

Embayment. Stratigraphically, the site is on Pleistocene loess deposits. These deposits are aeolian in nature and locally, if undisturbed, have fairly uniform index properties. From an engineering point of view, the undisturbed loess in Vicksburg has excellent strength properties because of its cementitious properties. The strength properties decrease rapidly if the soil is disturbed or becomes saturated.

Nine borings were placed at the site. Three borings were drilled to depths of 20 feet (Boring Nos. B-1, B-2, and B-4) and one boring (Boring No. B-3 was drilled to a depth of 10 feet, where an obstruction was encountered) in the building footprint. One boring (Boring No. B-5) was drilled to a depth of 40 feet under the pool area. Four boring borings (Boring Nos. B-6 through B-9) were drilled to depths of 5 feet in the parking lot areas. The soils consisted of layers of silty clay (CL-ML) and lean clay (CL). Note that obstructions were encountered at a depth of 10 feet in both Boring Nos. B-3 and B-5. Colors of the soils at the site were brown, brown and tan, tan, tan and gray, and gray and tan. These materials had consistencies which ranged from soft to stiff, as inferred from Standard Penetration Test (SPT) blow counts.

The silty clay (CL-ML) was encountered in every boring except two of the shallow borings (Boring Nos. B-7 and B-8) in the parking areas. The consistencies of these silty clays ranged from soft to stiff, with SPT blow counts varying from 3 to 16 blows (averaging approximately 9 blows). The field moisture contents within these strata ranged from 9.4 percent to 28.2 percent, and averaged 19.0 percent. These are low plasticity silty clays with liquid limits ranging from 29 percent to 33 percent (averaging approximately 31 percent) and plasticity indices ranging from 4 percent to 7 percent (averaging approximately 6 percent). The shrink/swell potential is low in these materials and only small changes in volume should be noted with changes in moisture content. The percentages of samples of these materials passing through the #200 grain-size sieve ranged from 83.5 percent (in Boring No. B-3) to 99.8 percent, and averaged 98.0 percent. Sand and gravel were noted in Boring No. B-3 between 5 and 10 feet deep. Note that this was the boring that was terminated at 10 feet deep because of an obstruction.

Lean clays (CL) were encountered in six out of the nine borings (Boring Nos. B-1 and B-4 through B-8. Consistencies of these

lean clays ranged from medium to stiff, as inferred from the Standard Penetration Test (SPT) blow counts. Blow counts within these lean clays varied from 8 blows to 12 blows, averaging approximately 10 blows. The field moisture contents varied from 14.5 percent to 24.5 percent, and averaged 19.2 percent. These are, also, low plasticity materials with liquid limits ranging from 32 percent to 36 percent (averaging approximately 34 percent) and plasticity indices ranging from 8 percent to 13 percent (averaging approximately 10 percent). The shrink/swell potential is, also, low and only small changes in volume should be noted with changes in moisture content. The percentages of samples of these CL materials passing through the #200 grain-size sieve ranged from 96.6 percent to 99.8 percent, and averaged 98.9 percent.

No static water levels were noted on the boring logs after completion of drilling and sampling operations. However, we note that groundwater levels and soil moisture contents in this area do fluctuate during the year with variations in rainfall and other environmental factors. Therefore, the groundwater levels and soil moisture contents will vary throughout the year and will probably be different if tested at a different time.

RECOMMENDATIONS FOR SITE PREPARATION AND FOUNDATIONS

We understand that this project will consist of a Comfort Suites motel located on Berryman Road, Vicksburg, Mississippi. The following conclusions and recommendations are based on our understanding of the proposed construction, information gathered during the exploration, accepted geotechnical engineering principles and practices, and our experience with similar sites and subsurface conditions. This report has been prepared for the exclusive use of Roy Patel, CHA, Brandon, Mississippi in the planning and design of the motel. We request that we be informed of any significant changes to the proposed construction so we might review our recommendations in light of the new information. We are available to review the final foundation and grading plans, as well as applicable portions of the project specifications, prior to construction.

Final plans and specifications were not available at the time of this geotechnical report, but it is our assumption that the placement of the building will be on a lot that is essentially

flat. Based upon our interpretation of the soil conditions at the site, proper placement of the foundation soil, and the assumption that no large or unusual loads are anticipated, it is our opinion that the proposed construction could be supported by a combination of foundation units, such as grade beams and spread footings. The foundation could consist of a monolithically cast, reinforced concrete, slab on-grade with turned-down, continuous grade beams and interior stiffeners to produce a beam diaphragm system. Column loads could be supported by isolated spread footings or thickened sections.

SITE PREPARATION

The near-surface material occurring in the borings at the site consisted of layers of silty clay (CL-ML) and lean clay (CL). Obstructions that caused auger refusal were encountered at a depth of 10 feet in both Boring Nos. B-3 and B-5 and soft material was encountered at 10 feet in Boring No. B-2.

The boring data have indicated both soft soil and some type of obstruction across the building footprint at approximately 10 feet deep and it is our opinion that the foundation soil should be excavated a minimum of 10 feet deep to remove the soft soil within an area beneath and extending a minimum of 10 feet beyond the perimeter of the structure, if possible. The obstruction may have to be removed. Properly compacted select fill should be placed to bring the foundation up to construction grade.

Following excavation we recommend that the subgrade in all fill areas be evaluated by a geotechnical engineer or his representative prior to fill placement. The engineer may recommend proof-rolling the areas as a means of evaluating the suitability of the subgrade for fill. Proof-rolling consists of systematically patrolling the area preferably in perpendicular directions, utilizing a heavily loaded dump truck (minimum 20 tons) or other suitable vehicle approved by the engineer. Any areas which pump or rut excessively and which cannot be densified by continued rolling should be undercut to suitable material and properly backfilled. If the construction footprint is too small for proof-rolling, numerous density tests or hand held cone penetration tests should be conducted to determine soft areas.

Select structural fill material should be placed in maximum loose lifts of 8 inches and should be compacted to 98 percent standard Proctor density (ASTM D-698-91) within 2 percentage points of optimum moisture content. Sufficient field density tests should be conducted to insure compaction requirements are met during construction. As a rule of thumb, we recommend that two density tests per lift be performed for each 2000 square feet of surface area. In addition, monitoring of fill construction and compaction will result in minimizing future settlement of the fill and the structure. We, therefore, believe that it is important that a qualified geotechnical engineer or certified technician monitor earthwork operations and that this work not be controlled by the earthwork contractor.

The select, structural fill material should consist of a material having a liquid limit of less than 40 percent and a plasticity index between 8 percent and 20 percent. The excavated materials, that contain topsoil and any debris, should not serve as select fill and should be disposed of outside the foundation area. On site excavated lean clay (CL) and silty clay (CL-ML) could be used as select fill with proper moisture content and compaction control.

FOUNDATION STRENGTHS

The foundation system should bear at a minimum depth 24 inches below the finished grade elevation. Minimum depths needed to offset wind forces should be verified by your structural engineer. All foundation members should be reinforced both top and bottom, sufficient to resist differential movement, and the completed foundation system should provide for uniform distribution of applied loads to the bearing soils. The maximum soil pressure under the foundation members should not exceed 2.2 kips per square foot for continuous foundation units or 2.8 kips per square foot for individual spread footings. Foundations sized in accordance with recognized criteria for the above stated allowable soil bearing pressure should provide a factor of safety of 2.0 - 3.0 against ultimate failure of the soil medium with total estimated settlements of 1 inch, more or less.

Note that the soils at this site contain lean clay (CL) and silty clay (CL-ML) that can lose strength with increases in moisture content. It is important to properly control the moisture

content of these soils during construction. The final site grading plan should provide for quick runoff of surface waters away from the building foundation in all directions. In addition, any foundation soils in exposed excavations that become wet or soft should be removed and replaced prior to footing installation. The landscape plans should insure that large water consuming trees and shrubs are not located within 50 feet of the perimeter of the foundation members. Where any large trees or stumps are removed or where any plumbing or electrical trenches are cut under the foundation, select fill material should be used and compacted.

All foundation recommendations made in this report are contingent upon proper execution of the earthwork requirements noted herein. We believe that it is very important that a qualified geotechnical engineer, familiar with working with such soils, be present during foundation construction.

RECOMMENDATIONS FOR PARKING AREAS AND ACCESS DRIVES

The near-surface materials at the site, which will be the in situ material for the subgrade for the parking areas and access drives, were primarily composed of silty clay (CL-ML) and lean clay (CL). The consistency of these soils were not determined for these borings but consistencies as noted in the foundation borings ranged from soft to stiff.

The foundation soil should be excavated a minimum of 2 feet, independent of the amount of select fill to be used, to remove any topsoil, roots, and organics. The soils at this site are variable in both strength and composition. Every effort should be made to insure that the exposed soils do not "dry out" during construction. Any soft or wet areas encountered during construction which cannot be stabilized should be undercut and filled with compacted select material.

We recommend proof-rolling the area as a means of evaluating the suitability of the subgrade for fill or pavement support. Proof-rolling is defined above. We recommend that, after proof-rolling, the subgrade soils for any cut sections should be compacted to 98 percent standard Proctor density (ASTM D-698-91) within 2 percentage points of optimum moisture content to a depth of 8 inches.

Compacted fill should then be placed to bring the subgrade up to elevation where required. Prior to placing the select fill in any area, we recommend that the subgrade be evaluated by a geotechnical engineer or his representative to determine the suitability of the subgrade.

Select fill material should consist of a soil having a liquid limit of not more than 40 percent and a plasticity index between 8 percent and 22 percent. This soil should be placed in maximum loose lifts of 8 inches and also compacted to a minimum of 98 percent standard Proctor density. Compaction for the entire site could be attained using a rubber tired or sheeps foot roller. After preparation of the subgrade, the remaining pavement structure can then be placed according to the recommendations provided below.

Based on the type of soils encountered, we anticipate that a CBR value greater than 3 will be representative of the strength of the prepared subgrade soils and compacted fill placed at this site, assuming proper control of the soil moisture content. It is our assumption that the parking lot and access drive will be used by the customers, employees, and a minimum of two-axle trucks for any deliveries; this precludes street traffic. Based on that assumption and the soil properties, we have selected a Structural Number (SN) of 2.0 for the parking lot and 2.9 for the access drive design and the dumpster pad (based on AASHTO Guide for Design of Pavement Structures, Chapter 4, Low-Volume Road Design, 1986). One of the following pavement alternatives should be used, assuming proper compaction of the subgrade soils:

LIGHT PARKING

Alternative #1

a) Base Course - Six (6) inches of hot mixed Bituminous Base course (Type BB-1) conforming to Mississippi State Highway Department (MSHD) Specifications.

b) Surface Course - One and one half (1 ½) inches of hot mixed bituminous Surface Course (Type SC-1) conforming to MSHD Specifications.

Alternative # 2

a) Clay Gravel Base or Subbase - Eight (8) inches of clay gravel subbase course, Class 4, Group B conforming to MSHD Specifications.

b) Base Course - Four (4) inches of hot mix Bituminous Base, (Mix Number BB-1) conforming to MSHD Specifications.

c) Surface Course - One and one-half (1 ½) inches of hot mix bituminous Surface Course, (Mix Number SC-1) conforming to MSHD Specifications.

Alternative #3

Surface Course - Five (5) inches of Portland Cement Concrete.

ACCESS ROADS

Alternative #1

a) Base Course - Six and one-half (6 ½) inches of hot mix Bituminous Base, (Mix Number BB-1) conforming to Mississippi State Highway Department (MSHD) Specifications.

b) Surface Course - One and one-half (1 ½) inches of hot mix bituminous Surface Course, (Mix Number SC-1) conforming to MSHD Specifications.

Alternative # 2

Surface - Six (6) inches of Portland Cement Concrete.

DUMPSTER PAD

Alternative # 1

Surface - Six (6) inches of Portland Cement Concrete. The dumpster pad should be the total length of dumpster and the

garbage truck.

The concrete pavement recommendations are for non-reinforced Portland Cement concrete pavement placed on a six-inch-thick clay gravel base course placed on the compacted subgrade. The clay gravel base course should be compacted to a minimum of 98% standard Proctor density immediately prior to concrete placement. The concrete should have a minimum 28-day flexural strength of 650 psi and a compressive strength of 3000 psi. Joint spacing, joint configuration, mix design, mix placement, and curing should conform to the recommendations of the American Concrete Institute (ACI) and the Portland Cement Association (PCA).

Applicable Mississippi State Highway Department specifications and structural number coefficients utilized in the pavement recommendations are provided as follows:

a) Bituminous Surface Course - Structural Coefficient = 0.44 - hot mixed bituminous Surface Course (Type SC-1) - Mississippi Standard Specifications for Road and Bridge Construction (1976 edition) or from the AASHTO Interim Guide for Design of Pavement Structures.

b) Bituminous Base Course - Structural Coefficient = 0.34 - hot mixed Bituminous Base course (Type BB-1) - Mississippi Standard Specifications for Road and Bridge Construction (1976 edition), Section 301, page 257 and Section 703, page 785 or from the AASHTO Guide.

c) Clay Gravel Base or Subbase - Structural Coefficient = 0.11 - Class 4, Group B - Mississippi Standard Specifications for Road and Bridge Construction (1976 edition), Section 304, page 275 and Section 703, page 773 or from the AASHTO Guide.

All pavement design recommendations made in this report are contingent upon proper execution of the subgrade requirements noted herein. We believe that it is very important that a qualified geotechnical engineer, familiar with working with such soils, be present after excavation and during proof-rolling, fill, and compaction. In addition, sufficient field density tests should be taken to insure that the compaction criteria are satisfied, and to reduce the possibility of settlement at this location. It is important that a good drainage system be established to quickly remove surface water, thus leaving no

standing water.

REPORT LIMITATIONS

The recommendations made in this report are based on the assumption that the borings are representative of the subsurface conditions throughout the site. Therefore, we cannot warrant that our boring logs represent subsurface conditions at other locations or times. If during construction, any unusual or significantly different conditions are encountered, we should be advised in order to review the changed conditions, and subsequently reconsider any of the above recommendations.

Further, we are available to review those portions of the plans and specifications relating to earthwork and foundations for this particular project and request that we be retained to do so in order to determine whether the plans and specifications are consistent with the recommendations contained within this report. In addition, we are available to observe foundation construction procedures, including interpretation of the use of on-site materials and compaction of the structural fill, quality control of concrete placement, and other field observations and quality control measures as required.

Other information, such as is outlined by the Association of Soil and Foundation Engineers (ASFE), regarding the use of this geotechnical report follows.

IMPORTANT INFORMATION ABOUT YOUR GEOTECHNICAL ENGINEERING REPORT

More construction problems are caused by site subsurface conditions than any other factor. As troublesome as subsurface problems can be, their frequency and extent have been lessened considerably in recent years, due in large measure to programs and publications of the (ASFE).

The following suggestions and observations are offered tohelp you reduce the geotechnical-related delays, cost-overruns and other costly headaches that can occur during a construction project.

A GEOTECHNICAL ENGINEERING REPORT IS BASED ON A UNIQUE SET OF PROJECTSPECIFIC FACTORS

A geotechnical engineering report is based on a subsurface exploration plan designed to incorporate a unique set of projectspecific factors. These typically include: the general nature of the structure involved, its size and configuration, the location of the structure on the site, and its orientation;

physical concomitants such as access roads, parking lots, and underground utilities, and the level of additional risk which the client assumed by virtue of limitations imposed upon the exploratory program. To help avoid costly problems, consult the geotechnical engineer to determine how any factors which change subsequent to the date of the report may affect its recommendations.

Unless your consulting geotechnical engineer indicates otherwise your geotechnical engineering report shouldnot be used:

When the nature of the proposed structure is changed, for example, if an office building will be erected instead of a parking garage, or if a refrigerated warehouse will be built instead of an unrefrigerated one.

When the size or configuration of the proposed structure is altered.

When the location or orientation of the proposed structure is modified.

When there is a change of ownership.

For application to an adjacent site.

Geotechnical engineers cannot accept responsibility for problemswhich may develop if they are not consulted after factors considered in their report's development have changed.

MOST GEOTECHNICAL "FINDINGS" ARE PROFESSIONAL ESTIMATES

Site exploration identifies actual subsurface conditions only at those points where samples are taken, when they are taken. Data derived through sampling and subsequent laboratory testing are extrapolated by geotechnical engineers who then render an opinion about overall subsurface conditions, their likely reaction to proposed construction activity, and appropriate foundation design. Even under optimal circumstances actual conditions may differ from those inferred to exist, because no geotechnical engineer, no matter how qualified, and no subsurface exploration program, no matter how comprehensive, can reveal what is hidden by earth, rock, and time. The actual conditions in areas not sampled may differ from predictions. Nothing can be done to prevent the unanticipated, but steps can be taken to help minimize their impact. For this reason, most experienced owners retain their geotechnical consultants through the construction stage, to identify variances, conduct additional tests which may be needed, and to recommend solutions to problems encountered on site.

SUBSURFACE CONDITIONS CAN CHANGE

Subsurface conditions may be modified be constantlychanging natural forces. Because a geotechnical engineering report is based on conditions which existed at the time of subsurface exploration, construction decisions should not be based on a geotechnical engineering report whose adequacy may have been affected by time. Speak with the geotechnical consultant to learn if additional tests are advisable before construction starts.

Construction operations at or adjacent to the site and natural eventssuch as floods, earthquakes, or ground water fluctuations may also affect subsurface conditions and, thus, the continuing adequacy of a geotechnical report. The geotechnical engineer should be kept apprised of any such events, and should

be consulted to determine if additional tests are necessary.

GEOTECHNICAL SERVICES ARE PERFORMED FOR SPECIFIC PURPOSES AND PERSONS

Geotechnical engineers' reports are prepared to meet the specific needs of specific individuals. A report prepared for a consulting civil engineer may not be adequate for a construction contractor, or even some other consulting civil engineer. Unless indicated otherwise, this report was prepared expressly for the client involved and expressly for purposes indicated be the client. Use by any other persons for any purpose, or by the client for a different purpose, may result in problems. No individual other than the client should apply this report for its intended purpose without first conferring with the geotechnical engineer. No person should apply this report for any purpose other than that originally contemplated without first conferring with the geotechnical engineer.

ladner testing laboratories, inc

2832 Utica Avenue/Post Office Box 10778/Jackson, Mississippi 39289-0778 / (601) 362-5421
2123 Glendale Avenue/ Hattiesburg, Mississippi 39402/ (601) 544-5782
P.O. Box 2363/ Gulfport, Mississippi 39505/ (228) 604-2527

PROJECT: COMFORT SUITES VICKSBURG MS	**CLIENT:** ROY PATEL, CHA 84 GRANDVIEW CIRCLE BRANDON MS 39047	**DATE** 11/1/2006 **LAB NO.** 761-06-A **BORE NO.** B-1 **TECHNICIAN** MILYN

SAMPLES: | AUGER(ASTM D-1452) | TUBE(ASTM D-1582) | X | PENETRATION TEST(ASTM D-1596)

DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0		BROWN LEAN CLAY (0 - 3.5')		23.0	32.0	8.0	98.8	CL	
	X		MEDIUM						8
	X	BROWN SILTY CLAY (3.5' - 10')	STIFF	19.6	29.0	5.0	98.2	CL-ML	10
5									
			MEDIUM						8
10	X	BROWN SILTY CLAY (10' - 15')		19.7	30.0	4.0	99.0	CL-ML	
			STIFF						11
15	X	BROWN & TAN SILTY CLAY (15' - 20')		16.6	32.0	7.0	98.8	CL-ML	
	X		STIFF						12
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.

WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 20 FT.

ladner testing laboratories, inc

2832 Utica Avenue/Post Office Box 10778/Jackson, Mississippi 39289-0778 / (601) 362-5421
2123 Glendale Avenue/ Hattiesburg, Mississippi 39402/ (601) 544-5782
P.O. Box 2363/ Gulfport, Mississippi 39505/ (228) 604-2527

PROJECT: COMFORT SUITES VICKSBURG MS	**CLIENT:** ROY PATEL, CHA 84 GRANDVIEW CIRCLE BRANDON MS 39047	**DATE** 11/1/2006 **LAB NO.** 761-06-A **BORE NO.** B-2 **TECHNICIAN** MILYN

SAMPLES: | AUGER(ASTM D-1452) | TUBE(ASTM D-1582) | X | PENETRATION TEST(ASTM D-1596)

DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0		BROWN SILTY CLAY (0 - 2.5')		18.0	31.0	7.0	99.0	CL-ML	
	X		STIFF						16
		BROWN SILTY CLAY (2.5' - 8.5')		17.1	32.0	7.0	99.0	CL-ML	
	X		STIFF						9
5									
	X	BROWN SILTY CLAY (8.5' - 13.5')	SOFT	15.5	33.0	7.0	99.0	CL-ML	3
10									
	X	TAN SILTY CLAY (13.5' - 15')	STIFF	9.4	29.0	4.0	98.4	CL-ML	9
15									
	X		STIFF						10
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.

WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 20 FT.

ladner testing laboratories, inc

2832 Utica Avenue/Post Office Box 10778/Jackson, Mississippi 39289-0778 / (601) 362-5421
2123 Glendale Avenue/ Hattiesburg, Mississippi 39402/ (601) 544-5782
P.O. Box 2363/ Gulfport, Mississippi 39505/ (228) 604-2527

PROJECT: COMFORT SUITES VICKSBURG MS	**CLIENT:** ROY PATEL, CHA 84 GRANDVIEW CIRCLE BRANDON MS 39047	**DATE** 11/1/2006 **LAB NO.** 761-06-A **BORE NO.** B-3 **TECHNICIAN** MILYN

SAMPLES: | AUGER(ASTM D-1452) | TUBE(ASTM D-1582) | X | PENETRATION TEST(ASTM D-1596)

DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0		BROWN SILTY CLAY (0 - 2.5')		24.4	30.0	6.0	99.0	CL-ML	
	X		MEDIUM						7
		BROWN SILTY CLAY (2.5' - 5')		21.3	29.0	5.0	98.8	CL-ML	
	X		STIFF						12
5		BROWN SILTY CLAY W/SAND, GRAVEL (5' - 10')		21.0	33.0	7.0	83.5	CL-ML	
	X	(HIT OBSTRUCTION & TERMINATED SOIL BORING)	STIFF						14
10									
15									
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.

WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 10 FT.

ladner testing laboratories, inc

2832 Utica Avenue/Post Office Box 10778/Jackson, Mississippi 39289-0778 / (601) 362-5421
2123 Glendale Avenue/ Hattiesburg, Mississippi 39402/ (601) 544-5782
P.O. Box 2363/ Gulfport, Mississippi 39505/ (228) 604-2527

PROJECT: COMFORT SUITES VICKSBURG MS	**CLIENT:** ROY PATEL, CHA 84 GRANDVIEW CIRCLE BRANDON MS 39047	**DATE** 11/1/2006 **LAB NO.** 761-06-A **BORE NO.** B-4 **TECHNICIAN** MILYN

SAMPLES: | AUGER(ASTM D-1452) | TUBE(ASTM D-1582) | X | PENETRATION TEST(ASTM D-1596)

DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0		BROWN SILTY CLAY (0 - 5')		21.9	30.0	6.0	99.0	CL-ML	
	X		MEDIUM						6
	X		MEDIUM						7
5	X	BROWN LEAN CLAY (5' - 13.5')		19.4	36.0	13.0	96.6	CL	
	X		STIFF						9
10									
	X	BROWN & TAN LEAN CLAY (13.5' - 20')	STIFF	14.5	36.0	10.0	99.8	CL	10
15									
	X		STIFF						12
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.

WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 20 FT.

ladner testing laboratories, inc

2832 Utica Avenue/Post Office Box 10778/Jackson, Mississippi 39289-0778 / (601) 362-5421
2123 Glendale Avenue/ Hattiesburg, Mississippi 39402/ (601) 544-5782
P.O. Box 2363/ Gulfport, Mississippi 39505/ (228) 604-2527

PROJECT: COMFORT SUITES VICKSBURG MS	**CLIENT:** ROY PATEL, CHA 84 GRANDVIEW CIRCLE BRANDON MS 39047	**DATE** 11/1/2006 **LAB NO.** 761-06-A **BORE NO.** B-5 **TECHNICIAN** MILYN

SAMPLES: AUGER(ASTM D-1452) | TUBE(ASTM D-1582) | X PENETRATION TEST(ASTM D-1596)

DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0		BROWN SILTY CLAY (0 - 5')		16.4	31.0	7.0	99.8	CL-ML	
	X		MEDIUM						6
	X		MEDIUM						7
5		GRAY & TAN LEAN CLAY (5' - 8.5)		18.4	34.0	10.0	97.8	CL	
	X	BROWN & TAN LEAN CLAY (8.5' - 15')	MEDIUM	14.9	35.0	11.0	99.6	CL	8
10		(HIT DEBRIS @ 10'/OFF-SET HOLE TO COMPLETE)							
	X		STIFF						11
15		BROWN & TAN LEAN CLAY (15' - 20')		11.1	33.0	9.0	99.2	CL	
	X		STIFF						12
20		TAN SILTY CLAY (20' - 28.5')		12.7	32.0	6.0	99.0	CL-ML	
25									
	X	TAN & GRAY SILTY CLAY (28.5' - 33.5')	STIFF	19.2	32.0	7.0	99.2	CL-ML	10
30									

WATER DEPTH 0 **FT.** **AFTER** 0 **HRS.** **BORING ELEVATION** 0 **FT.**

WATER DEPTH 0 **FT.** **AFTER** 0 **HRS.** **BORING TERMINATED AT** 40 **FT.**

ladner testing laboratories, inc

2832 Utica Avenue/Post Office Box 10778/Jackson, Mississippi 39289-0778 / (601) 362-5421
2123 Glendale Avenue/ Hattiesburg, Mississippi 39402/ (601) 544-5782
P.O. Box 2363/ Gulfport, Mississippi 39505/ (228) 604-2527

PROJECT: COMFORT SUITES VICKSBURG MS	**CLIENT:** ROY PATEL, CHA 84 GRANDVIEW CIRCLE BRANDON MS 39047	**DATE** 11/1/2006 **LAB NO.** 761-06-A **BORE NO.** B-5 **TECHNICIAN** MILYN

SAMPLES: | AUGER(ASTM D-1452) | TUBE(ASTM D-1582) | X | PENETRATION TEST(ASTM D-1596)

DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
30									
	X	TAN SILTY CLAY (33.5' - 40')	MEDIUM	28.2	31.0	7.0	98.4	CL-ML	6
35									
	X		SOFT						3
40									
45									
50									
55									
60									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.

WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 40 FT.

ladner testing laboratories, inc

2832 Utica Avenue/Post Office Box 10778/Jackson, Mississippi 39289-0778 / (601) 362-5421
2123 Glendale Avenue/ Hattiesburg, Mississippi 39402/ (601) 544-5782
P.O. Box 2363/ Gulfport, Mississippi 39505/ (228) 604-2527

PROJECT:	CLIENT:	DATE	11/1/2006
COMFORT SUITES	ROY PATEL, CHA	LAB NO.	761-06-A
	84 GRANDVIEW CIRCLE	BORE NO.	B-6
VICKSBURG MS		TECHNICIAN	MILYN
	BRANDON MS 39047		

SAMPLES: | AUGER(ASTM D-1452) | TUBE(ASTM D-1582) | X | PENETRATION TEST(ASTM D-1596)

DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0		BROWN LEAN CLAY (0 - 1')		24.5	33.0	9.0	99.6	CL	
		BROWN SILTY CLAY (1' - 5')		16.7	30.0	6.0	98.0	CL-ML	
5									
10									
15									
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.

WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 5 FT.

ladner testing laboratories, inc

2832 Utica Avenue/Post Office Box 10778/Jackson, Mississippi 39289-0778 / (601) 362-5421
2123 Glendale Avenue/ Hattiesburg, Mississippi 39402/ (601) 544-5782
P.O. Box 2363/ Gulfport, Mississippi 39505/ (228) 604-2527

PROJECT: COMFORT SUITES VICKSBURG MS	**CLIENT:** ROY PATEL, CHA 84 GRANDVIEW CIRCLE BRANDON MS 39047	**DATE** 11/1/2006 **LAB NO.** 761-06-A **BORE NO.** B-7 **TECHNICIAN** MILYN

SAMPLES: | AUGER(ASTM D-1452) | TUBE(ASTM D-1582) | X | PENETRATION TEST(ASTM D-1596)

DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0		BROWN LEAN CLAY (0 - 1')		20.6	32.0	9.0	99.2	CL	
		BROWN LEAN CLAY (1' - 5')		17.2	32.0	8.0	98.8	CL	
5									
10									
15									
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.

WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 5 FT.

ladner testing laboratories, inc

2832 Utica Avenue/Post Office Box 10778/Jackson, Mississippi 39289-0778 / (601) 362-5421
2123 Glendale Avenue/ Hattiesburg, Mississippi 39402/ (601) 544-5782
P.O. Box 2363/ Gulfport, Mississippi 39505/ (228) 604-2527

PROJECT: COMFORT SUITES VICKSBURG MS	**CLIENT:** ROY PATEL, CHA 84 GRANDVIEW CIRCLE BRANDON MS 39047	**DATE** 11/1/2006 **LAB NO.** 761-06-A **BORE NO.** B-8 **TECHNICIAN** MILYN

SAMPLES: AUGER(ASTM D-1452) ▒ TUBE(ASTM D-1582) X PENETRATION TEST(ASTM D-1596)

DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0		BROWN LEAN CLAY (0 - 1')		23.2	33.0	10.0	99.4	CL	
		BROWN LEAN CLAY (1' - 5')		16.8	35.0	11.0	99.7	CL	
5									
10									
15									
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.

WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 5 FT.

ladner testing laboratories, inc

2832 Utica Avenue/Post Office Box 10778/Jackson, Mississippi 39289-0778 / (601) 362-5421
2123 Glendale Avenue/ Hattiesburg, Mississippi 39402/ (601) 544-5782
P.O. Box 2363/ Gulfport, Mississippi 39505/ (228) 604-2527

PROJECT: COMFORT SUITES VICKSBURG MS	**CLIENT:** ROY PATEL, CHA 84 GRANDVIEW CIRCLE BRANDON MS 39047	**DATE** 11/1/2006 **LAB NO.** 761-06-A **BORE NO.** B-9 **TECHNICIAN** MILYN

SAMPLES: | AUGER(ASTM D-1452) | TUBE(ASTM D-1582) | X | PENETRATION TEST(ASTM D-1596)

DEPTH	SAMPLE	VISUAL DESCRIPTION - REMARKS	CONSISTENCY	FIELD MOIST %	LL%	PI %	PASS #200 %	UNIFIED CLASS	STD. PEN
0		BROWN SILTY CLAY (0 - 3')		22.5	32.0	6.0	99.4	CL-ML	
		BROWN SILTY CLAY (3' - 5')		22.4	31.0	6.0	98.6	CL-ML	
5									
10									
15									
20									
25									
30									

WATER DEPTH 0 FT. AFTER 0 HRS. BORING ELEVATION 0 FT.

WATER DEPTH 0 FT. AFTER 0 HRS. BORING TERMINATED AT 5 FT.

SECTION 00330 - BID FORM

BID FOR SITE IMPROVEMENTS AND NEW CONSTRUCTION

TO: Roy Patel
84 Grandview Circle
Brandon, MS 39047

PROJECT: Comfort Suites Hotel
Berryman Road
Vicksburg, Mississippi

SUBMITTED: (Bidder's Name):_____

(Address):_____

(City, State, Zip):_____

(Telephone #):_____ (Facsimile #):_____

(Submittal Date):_____

The undersigned, in submitting this bid, understands and agrees that the Instructions to Bidders control, and, without limiting the foregoing, that this Bid Proposal is based upon the following:

1. That they have visited the site of the Work and have familiarized themselves with conditions as they affect the cost of the work.

2. That they have received and examined the Contract Documents, including all addenda issued by the Architect and have acknowledged same herein.

3. Proposes to furnish and install items as defined below and as required by the Contract Documents and All Addenda.

BASE BID : The four story Comfort Suites Hotel

Interior and exterior finishes, systems and equipment for the Comfort Suites Hotel as indicated on the Contract Documents.

BID AMOUNT:_____

_____Dollars ($_____)

AGREEMENT:

The Bidder agrees this Bid shall be good and may not be withdrawn for a period of thirty (30) days after the scheduled time and date for receiving bids. A bid bond is included with this bid form and shall be accepted by the Owner in the event of a withdrawal or forfeiture by the Bidder.

The Bidder understands the Owner reserves the right to accept or reject any or all bids and to waive informalities in bids received and monitor discrepancies in bidding procedures. If the Owner or Architect, in reviewing this Bid Form, discovers that the information provided has been falsified or exaggerated, or if the Contractor does not meet the pre-established criteria, same Contractor shall forfeit his/her right to the contract.

The Bidder also agrees, if awarded the Contract, to complete his work in accordance with the Owner's schedule and Milestone dates listed in Section 1040, Project Coordination.

The Contractor/Bidder agrees to forfeit the following penalties, for each area and phase of work, to the Owner in the event that the same contractor, through no fault of the Owner, fails to meet his/her obligations to complete the work on schedule, as follows:

Substantial Completion: $_____/day until a certificate of substantial completion is certified.

Project Close-Out:$_____/day until "punch list" completed and signed.

The final corrections to the contract shall be made through a "Change Order". The cost of the Performance/Material Payment Bond is included in the contract sum.

BID GUARANTEE:

The undersigned agrees that the Owner shall have the right to retain this bid for a period of thirty (30) days from the date and time of receiving bids and guarantee through a bid bond in the amount of five percent (5%) for the same thirty (30) day period.

BID ACCEPTANCE:

If written notice of the acceptance of this bid is mailed, telegraphed or delivered to the undersigned within the time noted herein, after the date of the opening of bids or any time thereafter before this bid is withdrawn, the undersigned agrees that he will execute a construction contract (AIA document A101, 1997 edition) in accordance with the Bid as accepted and will furnish contract security in the form of Performance and Material and Labor Payment Bonds with such surety or sureties as the Owner may approve. The cost of the Performance and Material and Labor Payment Bonds is included in the base bid contract sum.

It is understood and agreed that the Owner reserves the right to award the contract to his best interests, to reject any or all bids, to waive any informalities in bidding, and to hold all bids for the bid guarantee period.

ADDENDUM/ADDENDA :

The Bidder acknowledges that he has received the following Addenda and that each Addenda has been included in his Bid:

Addenda No._____Dated:_____
Addenda No._____Dated:_____
Addenda No._____Dated:_____
Addenda No._____Dated:_____
Addenda No._____Dated:_____

The undersigned Bidder represents that the above information and assurances are accurate to the best of his knowledge.

Signed and sealed this_____day of _____, 2005.

_____
Business Name

_____
Business Name

_____
Telephone #

BY: _____
Printed Name of Signer

_____
Signature

_____
Title

Corporate Seal

Attest if a Corporation

Title

END OF SECTION 00300

REQUEST FOR SUBSTITUTION

To: Mr. Roy Patel
84 Grandview Circle
Vicksburg, Mississippi 39047
(601) 829-9898

From: _____
name of company

_____
address

_____
city, state, zip code phone

Please fully answer all of the information requested below. Failure to answer any of the items below may cause rejection of request for substitution. Please use one form for each product being requested. Only the first product listed will be considered on forms with more than one product listed.

Specification Drawing Number
Section Number Detail Number

Specified Product Proposed Substitute

Product or part number
Color or finish
Pattern or profile
Weight or service rating

Please answer the following questions. Attach an explanation sheet on your company's letterhead when required.

Does the proposed substitution affect dimensions indicated on Drawings? No Yes (explain how).

Does the proposed substitution require changes in the drawings and/or design or installation changes? No Yes
(if yes, is the cost of these changes included in the proposed amount? No Yes)

Does the proposed substitution affect other trades?
No Yes (explain who and how).

If the proposed product does affect the work of other trades, has the cost impact on their work been included in the price of the proposed substitution? No Yes

Does the proposed product's guarantee differ from that of the specified product's? No Yes (explain how).

Why is this proposal for substitution being submitted? (list reasons below)

Attach a listing of 3 projects using the proposed substitution that have been completed within the past 5 years in the geographic and climatic region of this project. One of the applications must have been in service for at least 3 years.

Attach product data/brochures for the substitute product:

The undersigned represents that he has examined the Construction Documents and is familiar with the specified products, that he understands the indicated application of those products and the design intent of the Architect, and that the proposed substitution complies with the Construction Documents and will perform at least equally to the specified product within the limitations stated above. Additionally, the undersigned accepts responsibility for coordinating application and installation of the proposed substitution and waives all claims for additional costs resulting from the incorporation of the proposed substitution into the Project or its subsequent failure to perform according to specified requirements.

The undersigned represents that he has examined the specific provisions contained in Section 00800, the Supplementary General Conditions as negotiated between Owner and Contractor and agrees to comply with all of the provisions contained therein, or elsewhere in the Contract Documents.

Submitted by:_____
typed signature

Date: _____

The decision of the Owner regarding the acceptance or rejection of the proposed substitution will be based, at least in part, on the information supplied above and in the attached explanations and product data. The Owner, prior to the acceptance of the bid must receive requests for substitution.

SECTION 00500 –CONTRACT FORMS, BONDS AND CERTIFICATES

PART 1 - GENERAL

RELATED DOCUMENTS:

Applicable provisions of the General conditions and of Division 1, General Requirements govern all work in this Section.

WORK INCLUDED:

The following forms will be used during the construction of this Project. It shall be the Contractor's responsibility to obtain copies of these forms for his use.

CONTRACT FORMS:

A. Standard form of Agreement Between Owner and Contractor, AIA document A101, 1997 Edition.

FORMS AND CERTIFICATES:

A. File with Owner Prior to starting Work:
   1. Certificate of Insurance
   2. Performance Bond and Payment Bond, AIA Document A312, 1984 Edition.

B. Construction Change Order, AIA Document G701, 2000 Edition.

C. Application and Certificate for Payment, AIA Document G702 and G703, 1992 Edition.

D. Architect will prepare at Substantial Completion: Certificate of Substantial completion, AIA Document G704, 2000 Edition.

E. Closeout documents to be filed with Owner in accordance with Section 01700.

   1. Contractor's Affidavit of Payment of Debt's and Claims, AIA Document G706, 1994 edition.
   2. Contractor's Affidavit of Release, AIA Document G706A, 1994 Edition.
   3. Consent of Surety Company to final Payment, AIA Document G707, 1994 Edition.

END OF SECTION 00500

SECTION 00700 – AIA GENERAL CONDITIONS OF THE CONTRACT FOR CONSTRUCTION

PART 1 - GENERAL

A. The contractor and all persons whom he may employ or contract to do Work on this Project, shall be bound by these General conditions as if repeated in each Section of this Project Manual.

B. The General Conditions of the Contract for Construction, Fourteenth Edition, AIA Document A201, 1987 Edition, are hereby made a part of this Project Manual.

C. The failure on the part of the Contractor to familiarize himself or examine these Documents will in no way relieve him of the responsibilities and conditions set out herein.

END OF SECTION 00700

Section 00800

SUPPLEMENTAL CONDITIONS:

The Supplemental Conditions shall be those agreed to between the Owner and the Contractor at a future date.

END OF SECTION 00800

COMFORT SUITES

SECTION 00850 – SCHEDULE OF DRAWINGS

GENERAL

G001	Cover Sheet
G002	Project Information Sheet

CIVIL ENGINEERING

C1.1	General Construction Notes & Details
C2.1	Site Plan
C3.1	Utility Layout
C3.2	Grading & Drainage Plan
C3.3	Erosion Control Layout
C4.1	Water Distribution System & Sanitary Sewer System Details
C4.2	Storm Drain Details

ARCHITECTURAL

A101	Architectural Site Plan
A102	Architectural Site Details
A103	Ground Floor Plan
A104	Second Floor Plan
A105	3rd Floor Plan
A106	4^{th} Floor Plan
A107	Rated Partition Plan
A201	Ground Floor Ceiling Plan
A202	Second Floor Ceiling Plan
A203	3^{rd}-4^{th} Floor Ceiling Plan
A204	Roof Plan
A301	Large Scale Guestroom Plans
A302	Large Scale Lobby Plan
A401	Exterior Elevations
A402	Exterior Elevations
A403	Building Sections
A501	Wall Sections
A502	Wall Sections
A503	Wall Sections
A504	Exterior Details
A505	Stair Details
A601	Interior Elevations
A601.1	Interior Elevations
A602	Interior Elevations
A603	Interior Details
A604	Schedules
A605	Schedules
A606	Registration Desk Details
A607	Registration Desk Details

STRUCTURAL ENGINEERING

S-1	Foundation Plan
S-2	2^{nd} Floor Framing Plan
S-3	3^{rd}-4^{th} Floor framing Plan
S-4	Roof Framing Plan
S-5	Details/ Sections
S-6	Details/ Sections

MECHANICAL ENGINEERING

M101	Mechanical First Floor Plan & Schedules
M102	Mechanical 2nd Floor Plan, Schedules & Details
M103	Mechanical 3rd Floor Plan
M104	Mechanical 4th Floor Plan
M105	Mechanical Roof Plan & Details

P101	Plumbing First Floor Plan-DWV
P102	Plumbing First Floor Plan-Domestic Water
P103	Plumbing Second Floor Plan
P104	Plumbing Third & Fourth Floor Plan
P105	Plumbing Risers Diagrams-DWV
P106	Plumbing Risers Diagrams-DWV
P107	Plumbing Risers Diagrams-Domestic Water
P108	2nd & 3rd Floor Water
P109	Plumbing Details

ELECTRICAL ENGINEERING

E101	Electrical Site Plan
E201	Ground Floor Power Plan
E202	Ground Floor Lighting Plan
E203	2nd Floor Electrical Plan
E204	3rd Floor Electrical Plan
E205	4th Floor Electrical Plan
E206	Electrical Roof Plan
E301	Typical Guest Rooms Electrical Plans
E401	Electrical Panelboards One-Line Diagram
E402	Electrical Panelboards Schedules
E403	Electrical Panelboards Schedules
E404	Electrical Panelboards Schedules

END OF SECTION 00850