COORDINATION:

- STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH AND COORDINATED WITH ARCHITECTURAL DRAWINGS AND OTHER CONTRACT DOCUMENTS.
- 2. THE PROJECT ARCHITECT SHALL BE RESPONSIBLE FOR REVIEWING/COORDINATING ALL DIMENSIONS, ELEVATIONS AND DETAILS SHOWN ON THE STRUCTURAL DRAWINGS WITH THE ARCHITECTURAL DRAWINGS.
- 3. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL OF THE CONTRACT DOCUMENTS AND LATEST ADDENDA AND FOR SUBMITTING SUCH DOCUMENTS TO SUBCONTRACTORS AND MATERIAL SUPPLIERS PRIOR TO THE SUBMITTAL OF SHOP DRAWINGS, FABRICATION OF ANY STRUCTURAL MEMBERS, AND ERECTION IN THE FIELD. THE GENERAL CONTRACTOR SHALL COMPARE THE STRUCTURAL DRAWINGS AND OTHER CONTRACT DRAWINGS AND REPORT ANY DISCREPANCY BETWEEN AND WITHIN EACH SET OF DRAWINGS WITH THE PROJECT ARCHITECT AND THE STRUCTURAL ENGINEER PRIOR TO THE FABRICATION AND INSTALLATION OF ANY STRUCTURAL
- 4. DRAWINGS SHOW GENERAL AND TYPICAL SECTIONS/DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY SHOWN, SIMILAR SECTIONS/DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO THE APPROVAL OF THE ENGINEER.
- 5. THE STRUCTURAL MEMBERS OF THIS PROJECT HAVE BEEN DESIGNED BY THE STRUCTURAL ENGINEER TO RESIST THE REQUIRED CODE GRAVITY AND LATERAL FORCES THAT COULD OCCUR IN THE FINAL COMPLETED STRUCTURE ONLY. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL REQUIRED BRACING/SHORING DURING CONSTRUCTION TO MAINTAIN THE STABILITY AND SAFETY OF ALL STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PROCESS UNTIL THE STRUCTURE IS TIED TOGETHER AND COMPLETED.
- 6. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR THE METHODS. TECHNIQUES AND SEQUENCES OF PROCEDURES TO PERFORM THE WORK. THE SUPERVISION OF THE WORK IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- 7. LOADS APPLIED TO THE STRUCTURE DURING CONSTRUCTION SHALL NOT EXCEED THE SAFE LOAD-CARRYING CAPACITY OF THE STRUCTURAL MEMBERS. THE LIVE LOADS USED FOR THE DESIGN OF THE STRUCTURE ARE INDICATED IN THE GENERAL NOTES. DO NOT APPLY ANY CONSTRUCTION LOADS UNTIL STRUCTURAL FRAMING IS PROPERLY INSTALLED AND ALL TEMPORARY BRACING IS IN PLACE.
- ALL ASTM AND OTHER REFERENCES ARE PER THE LATEST EDITIONS UNLESS NOTED OTHERWISE.
- 9. EQUIPMENT PADS SHALL BE PROVIDED BY THE MECHANICAL, ELECTRICAL, OR PLUMBING CONTRACTORS REQUIRING THE PAD.
- 10. COORDINATE THE EXACT SIZE AND LOCATION OF ALL SLEEVES AND OPENINGS THROUGH CONCRETE WALLS, CONCRETE SLABS OR MASONRY WALLS WITH ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS.
- 11. SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. CONTRACTOR SHALL REVIEW, APPROVE AND SIGN EACH SHEET PRIOR TO SUBMISSION. THE STRUCTURAL ENGINEER'S REVIEW SHALL BE FOR CONFORMANCE WITH THE DESIGN CONCEPT AND GENERAL COMPLIANCE WITH THE CONTRACT DOCUMENTS. THE ENGINEER'S REVIEW DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW, CHECK AND COORDINATE THE SHOP DRAWINGS PRIOR TO SUBMISSION. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF THE SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, DIMENSIONS, ETC. CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS. SUBMIT PDF FILES FOR REVIEW.
- 12. CONTRACTOR SHALL VISIT THE SITE PRIOR TO BID TO ASCERTAIN CONDITIONS WHICH MAY ADVERSELY AFFECT THE WORK OR COST THEREOF.
- 13. WHERE CONFLICTS OCCUR BETWEEN GENERAL NOTES, STRUCTURAL DRAWINGS AND SPECIFICATIONS THE MOST STRINGENT REQUIREMENT SHALL APPLY.
- 14. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLYING WITH ALL SAFETY PRECAUTIONS AND REGULATIONS DURING THE WORK. THE ENGINEER WILL NOT ADVISE NOR ISSUE DIRECTION AS TO SAFETY PRECAUTIONS AND PROGRAMS.

- FOUNDATION DESIGN OF FOOTINGS BASED ON SOIL REPORT BY ARDAMAN & ASSOCIATES, INC., SHREVEPORT, LOUISIANA, FILE NO. 12.94.038, DATED 3-12-12. ALLOWABLE SOIL BEARING PRESSURE 1,500 PSF FOR CONTINUOUS WALL FOOTINGS AND 2,000 PSF FOR SPREAD COLUMN
- 2. ALL FOOTINGS AND SLAB ON GRADE SHALL BEAR ON UNDISTURBED RESIDUAL SOIL OR STRUCTURAL COMPACTED FILL AS PER SOIL REPORT RECOMMENDATIONS. ALL FOUNDATION EXCAVATIONS SHALL BE EVALUATED BY THE GEOTECHNICAL ENGINEER/TESTING AGENCY PRIOR TO PLACING CONCRETE FOR FOUNDATIONS.
- 3. COMPACT ALL MATERIALS SUPPORTING SLAB ON GRADE OR FOOTINGS AS PER GEOTECHNICAL ENGINEERS RECOMENDATIONS. SOILS TESTING LABORATORY SHALL CONDUCT COMPACTION TESTS ON ALL STRUCTURAL FILL MATERIAL.
- 4. NO UNBALANCED BACKFILLING SHALL BE DONE AGAINST BASEMENT WALLS UNLESS WALLS ARE BRACED BY TEMPORARY BRACING OR BY PERMANENT CONSTRUCTION.
- 5. FOUNDATION WALLS WITH BACKFILL ON EACH SIDE SHALL BE BACKFILLED EVENLY ON EACH SIDE. THESE WALLS HAVE NOT BEEN DESIGNED FOR UNBALANCED SOIL LOADS. ALL BASEMENT WALLS (FOUNDATION WALLS WITH DOWELS INTO SLAB ON GRADE) SHALL BE SHORED UNTIL SLAB ON GRADE REACHES 75 PERCENT OF THE 28 DAY COMPRESSIVE STRENGTH. WATERPROOF BACKSIDE OF ALL FOUNDATION WALLS UNLESS NOTED OTHERWISE.
- 6. COORDINATE FOUNDATION WORK WITH EXISTING UTILITIES. FOUNDATIONS SHALL BE LOWERED WHERE REQUIRED TO AVOID UTILITIES. NOTIFY PROJECT ARCHITECT AND STRUCTURAL ENGINEER TO PROVIDE REINFORCED CONCRETE PIER FOR COLUMN FOOTINGS
- 7. UNLESS NOTED OTHERWISE COLUMN CENTERLINES SHALL BE CENTERLINES OF COLUMN FOOTINGS.
- 8. HEAVY GRADING EQUIPMENT SHALL NOT BE ALLOWED WITHIN THE HEIGHT OF THE WALL (HORIZONTALLY) OF BASEMENT OR CANTILEVER RETAINING WALLS.

SLAB ON GRADE:

- CONTROL JOINTS FOR SLAB ON GRADE SHALL BE LOCATED AS SHOWN ON PLAN, WITH A MAXIMUM JOINT SPACING OF 2 1/2 TIMES THE SLAB THICKNESS IN FEET. JOINTS SHALL BE FORMED USING SAW CUTS 1/8" WIDE (MAXIMUM) BY T/4 (1 1/4" MIN.) DEEP. SAW CUT AS SOON AS PRACTICAL AND WITHIN 12 HOURS AFTER PLACING CONCRETE. JOINTS SHALL BE FILLED WITH SEMI-RIGID EPOXY JOINT FILLER (CONSPEC POLUREA JOINTFILL (OR EQUIVALENT).
- 2. SIDEWALKS AND OTHER EXTERIOR SLABS ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS. SEE ARCHITECTURAL, SITE AND CIVIL DRAWINGS FOR LOCATIONS, DIMENSIONS AND ELEVATIONS.
- SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATION OF DEPRESSED SLAB AREAS AND DRAINS. FLOOR SLABS SHALL SLOPE TO ALL FLOOR DRAINS. GENERAL CONTRACTOR SHALL COORDINATE WITH PLUMBING DRAWINGS - TYPICAL.
- 4. USE EVAPORATION RETARDER ONE OR MORE TIMES AFTER THE STRIKEOFF WHEN HIGH TEMPERATURES, LOW HUMIDITY AND/OR WIND WILL CAUSE CRUSTING AND PLASTIC CRACKING.
- 5. EXPOSED FLOOR SLAB AREAS ARE TO RECEIVE 2 COATS OF 25% MIN. SOLIDS ACRYLIC HARDENER AND SEAL (SPEC CHEM'S CURE AND SEAL WB 25 OR SPEC CHEM'S CURE AND SEAL 25UV, OR EQUIVALENT). APPLICATION IS TO CONFORM TO MANUFACTURER'S SPECIFICATIONS. FIRST COAT IS FOR CURING, SECOND COAT IS FOR SEALING AND DUST PROOFING AFTER BUILDING CONSTRUCTION COMPLETION. FIRST COAT ONLY REQUIRED WHERE SLAB IS RECEIVING FLOOR COVERING.
- SEE GEOTECHNICAL REPORT/GEOTECHNICAL ENGINEER FOR VAPOR RETARDER AND UNDERSLAB DRAINAGE FILL REQUIREMENTS. VAPOR RETARDER SHALL BE 10 MIL AND MEET ASTM E 1745 CLASS A INSTALLED PER ASTM E 1643 WITH LAPPED JOINTS NOT LESS THAN 6 INCHES.

CONCRETE:

WALL

1. CONCRETE SHALL BE PROPORTIONED TO MEET THE REQUIREMENTS OF THE FOLLOWING:

MENT	28-DAY	SLUMP	UNI.
	STRENGTH	RANGE	WEIG
	(PSI)	(IN.)	(PCF
UMN FOOTINGS	3000	`3 - 5	150
L FOOTINGS	3000	3-5	150
R ON CRADE	3000	3-A	150

- 2. PORTLAND CEMENT SHALL BE ASTM C 150, TYPE I. FLY ASH SHALL CONFORM TO ASTM C 618, CLASS F AND SHALL NOT EXCEED 25% OF CEMENT CONTENT BY WEIGHT. SLAG SHALL CONFORM TO ASTM C 989.
- NORMAL WEIGHT AGGREGATE SHALL CONFORM TO ASTM C 33. CONCRETE AGGREGATE GRADATION SHALL BE IN ACCORDANCE WITH ASTM C33 SPECIFICATION. "SPECIFICATION FOR CONCRETE AGGREGATE". FINE AGGREGATE SHALL CONSIST OF NATURAL SAND OR A COMBINATION THEREOF, WITH A FINENESS MODULUS BETWEEN 2.3 AND 3.1. LARGER COURSE AGGREGATE MIXES UP TO #67 ARE ACCEPTABLE FOR FLOOR SLAB CONCRETE TO MINIMIZE SHRINKAGE CRACKING.
- 4. FLY ASH AND/OR SLAG SHALL NOT BE PERMITTED IN CONCRETE PLACED SUBJECT TO COLD WEATHER PLACEMENT PROCEDURES.
- 5. CONCRETE EXCEEDING THE SPECIFIED SLUMP RANGES SHALL BE RETURNED. DO NOT ADD WATER TO THE CONCRETE MIX AT THE JOB SITE WITHOUT THE WRITTEN PERMISSION FROM THE STRUCTURAL ENGINEER.
- 6. ALL REINFORCING STEEL SHALL BE ASTM A615 GRADE 60 UNLESS NOTED OTHERWISE. ALL WELDED WIRE FABRIC (W.W.F.) SHALL BE ASTM A82 AND A185 COLD DRAWN STEEL WIRE. W.W.F. SHALL BE DELIVERED TO THE JOB SITE IN FLAT SHEETS (NO ROLLS). PLACE SHEETS ON BOLSTERS AT 36" MAXIMUM TO LOCATE IN UPPER THIRD OF SLAB. LAP CONTINUOUS REINFORCING BARS 36 BAR DIAMETERS UNLESS NOTED OTHERWISE. PROVIDE CORNER BARS IN ALL WALLS AND FOOTINGS. BAR SUPPORTS, DESIGN, DETAILING, FABRICATION, AND PLACING OF REINFORCING BARS SHALL BE IN ACCORDANCE WITH THE ACI CODE AND DETAILING MANUAL AND CRSI'S "MANUAL OF STANDARD PRACTICE".
- 7. MINIMUM CONCRETE PROTECTION FOR REINFORCEMENT SHALL BE:

CONCRETE EXPOSED TO EARTH OR WEATHER: No. 6 THROUGH No. 18 BARS2'	
No. 5 AND SMALLER	
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	·
SLABS, WALLS AND JOISTS:	
No. 14 AND No. 18 BARS	1/2"
No. 11 AND SMALLER	4"
BEAMS AND COLUMNS:	4 (0 !!
PRIMARY REINFORCEMENT, TIES, STIRRUPS AND SPIRALS1	1/2"

8. ANCHOR RODS FOR COLUMNS SHALL BE POSITIONED WITH A TEMPLATE PRIOR TO PLACING CONCRETE IN PIER OR FOOTING. NUTS SHALL BE TIGHTENED ON EACH SIDE OF THE TEMPLATE TO HOLD THE ANCHOR BOLTS IN PLACE.

- CONCRETE DESIGN AND REINFORCEMENT SHALL BE IN ACCORDANCE WITH THE "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" (CODE REFERENCED ACI 318) AND WITH "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT" (ACI 315-92). CONCRETE PLACED DURING HOT WEATHER AND COLD WEATHER SHALL MEET THE RECOMMENDATIONS OF ACI/PCA/TCA. CONCRETE SHALL BE SAMPLED AND TESTED BY AN INDEPENDENT TESTING AGENCY IN ACCORDANCE WITH ACI
- 10. CONCRETE MIXES SHALL BE DESIGNED IN ACCORDANCE WITH ACI 301. WATER SHALL NOT BE ADDED TO THE CONCRETE MIX AT THE JOB SITE WITHOUT THE PRIOR WRITTEN PERMISSION OF THE STRUCTURAL ENGINEER.
- 11. UNLESS OTHERWISE SHOWN ON ARCHITECTURAL DRAWINGS, PROVIDE 3/4" CHAMFER AT ALL COLUMN, WALL SLAB AND BEAM EDGES THAT ARE EXPOSED TO VIEW IN THE FINAL STRUCTURE.
- 12. PROVIDE VERTICAL CONTROL OR CONTRACTION JOINTS AT 25' MAXIMUM IN ALL CONCRETE BASEMENT WALLS, RETAINING WALLS, OR SCREENWALLS. PROVIDE VERTICAL EXPANSION JOINTS AT 100' MAXIMUM IN THE LINEAR PLANE OF THE WALL. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS IN AESTHETIC WALLS.
- 13. FIBER REINFORCEMENT POLYPROPYLENE FIBRILLATED FIBERS USE AT 1.5 POUNDS PER CUBIC YARD WITH A MINIMUM AVERAGE RESIDUAL STRENGTH OF 45 PSI IN ACCORDANCE WITH ASTM 1399 TESTING - FIBERMESH 300 OR EQUIVALENT.
- 14. SLAB ON GRADE SHALL HAVE AN OVERALL FLOOR FLATNESS (FF) OF 25 WITH A MINIMUM LOCAL VALUE OF 17 AND AN OVERALL FLOOR LEVELNESS (FL) OF 20 WITH A MINIMUM LOCAL VALUE OF 15.

MASONRY:

- 1. MASONRY CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE "SPECIFICATION FOR MASONRY STRUCTURES (CODE REFERENCED ACI 530.1)" AND NCMA SPECIFICATION TEK NOTES AND BIA TECHNICAL NOTES ON BRICK CONSTRUCTION. CONTINUOUS INSPECTION SHALL BE PERFORMED BY AN INDEPENDENT TESTING AGENCY FOR GENERAL COMPLIANCE WITH THE CONTRACT DOCUMENTS.
- 2. ALL HOLLOW CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C-90. LIGHTWEIGHT. MINIMUM NET COMPRESSIVE STRENGTH (f'm) SHALL BE 1,500 PSI. FILL BLOCK CELLS WHERE REBAR OCCURS SOLID WITH GROUT. SUBMIT VERIFICATION OF ALL MATERIALS TO ARCHITECT
- 3. ALL BRICK UNIT MASONRY SHALL CONFORM TO ASTM C-216, GRADE SW, TYPE FBS. MINIMUM NET COMPRESSIVE STRENGTH (f'm) SHALL BE 3,000 PSI. VERIFICATION OF ALL MATERIALS TO ARCHITECT FOR APPROVAL.
- 4. MORTAR SHALL BE PORTLAND CEMENT-LIME MIX (PORTLAND CEMENT SHALL COMPLY WITH ASTM C 150, TYPE I OR III, AND HYDRATED LIME COMPLYING WITH ASTM C 270) OR MORTAR CEMENT ASTM C 1329 - TYPE S, THE USE OF MASONRY-CEMENT IS STRICTLY FORBIDDEN. AGGREGATE FOR MORTAR SHALL COMPLY WITH ASTM C 144.
- 5. GROUT FOR UNIT MASONRY SHALL COMPLY WITH ASTM C 476 (SLUMP 8 TO 11 INCHES). AGGREGATE FOR GROUT SHALL COMPLY WITH ASTM C404. COMPRESSIVE STRENGTH SHALL BE GREATER THAN OR EQUAL TO 2.000 PSI OR f'm. WHICHEVER IS GREATER. TESTING SHALL BE DONE IN AN ABSORBENT MOLD IN ACCORDANCE WITH ASTM C 1019.
- 6. MASONRY JOINT REINFORCEMENT SHALL COMPLY WITH ASTM A-951 AND SHALL BE HOT DIPPED GALVANIZED, CARBON STEEL. BRICK TIES SHALL CONFORM TO SEISMIC DESIGN CATEGORY REQUIREMENTS (SUBMIT FOR APPROVAL). BRICK TIES USED IN SEISMIC DESIGN CATEGORY A, B, C SHALL BE SPACED AT 16" VERTICAL AND 24" HORIZONTAL. PROVIDE IN LENGTHS NOT LESS THAN 10 FEET IN LENGTH WITH PREFABRICATED CORNER AND TEE UNITS. FOR MULTIWYTHE MASONRY PROVIDE ADJUSTABLE 2-PIECE UNITS. PROVIDE CONTINUITY AT CORNERS AND WALL INTERSECTIONS BY USING PREFABRICATED "L" AND "T" SECTIONS. LAP REINFORCEMENT A MINIMUM OF 6". SPACE REINFORCEMENT NOT MORE THAN 16" O.C. PROVIDE REINFORCEMENT NOT MORE THAN 8" ABOVE OR BELOW WALL OPENINGS AND EXTENDING 24" BEYOND OPENINGS. CUT REINFORCEMENT AT CONTROL AND EXPANSION JOINTS UNLESS NOTED OTHERWISE.
- 7. ALL BOND BEAM REINFORCING AT FLOOR AND ROOF DIAPHRAGMS SHALL BE CONTINUOUS THROUGH MASONRY CONTROL JOINTS - UNLESS NOTED OTHERWISE.
- 8. PROVIDE VERTICAL CONTROL JOINTS AT 1.5 TIMES WALL HEIGHT OR 25' MAXIMUM (WHICHEVER IS LEAST). SEE ARCHITECTURAL DRAWINGS.
- 9. THE MASONRY CONTRACTOR SHALL PROVIDE ALL REQUIRED TEMPORARY WALL BRACING DURING CONSTRUCTION.
- 10. THE MINIMUM QUALITY ASSURANCE PROGRAM FOR NON-ESSENTIAL FACILITIES SHALL COMPLY WITH TABLE 1.14.2 OF ACI 530.

STRUCTURAL STEEL:

1. ALL W-SHAPE STRUCTURAL STEEL SHALL BE ASTM A992, ALL OTHER STRUCTURAL SHAPES SHALL BE ASTM A-36, SQUARE OR RECTANGULAR HSS SHAPES SHALL CONFORM TO ASTM A-500, GRADE B, ROUND HSS SHAPES SHALL CONFORM TO ASTM A-500, GRADE B, STRUCTURAL STEEL PIPE COLUMNS SHALL CONFORM TO ASTM A-501 OR ASTM A-53, TYPE E OR S, GRADE B. DESIGN, DETAILING, FABRICATION AND ERECTION SHALL BE IN ACCORDANCE WITH THE AISC CODE AND DETAILING MANUAL. NO STRUCTURAL MEMBERS SHALL BE SPLICED EXCEPT AS SHOWN ON APPROVED SHOP DRAWINGS.

2. FABRICATOR IS SOLELY RESPONSIBLE FOR THE DESIGN OF THE CONNECTIONS SHOWN ON THE STRUCTURAL DRAWINGS. REVIEW OF STRUCTURAL STEEL CONNECTIONS BY WGPM, INC. IS FOR GENERAL DESIGN INTENT ONLY. FOR THE PURPOSE OF CONNECTION DESIGN, THE FABRICATOR SHALL RETAIN A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT. THE ENGINEER SHALL SEAL, SIGN AND SUBMIT DESIGN CALCULATIONS FOR ALL NON-STANDARD AND LATERAL RESISTING CONNECTION DESIGNS. A NOTE SHOULD ACCOMPANY THE SEAL STATING THAT THE SEAL IS FOR "CONNECTION DESIGN ONLY" AND DOES NOT INCLUDE RESPONSIBILITY FOR MEMBER OR BUILDING DESIGN, DIMENSIONS, FITUP, ERECTION AND ETC. GENERALLY CONNECTIONS SHOWN ON THE STRUCTURAL DRAWINGS ARE SCHEMATIC AND ARE INTENDED TO SHOW THE RELATIONSHIP OF THE MEMBERS. CONNECTIONS SHALL BE DESIGNED FOR REACTIONS SHOWN ON CONTRACT STRUCTURAL DRAWINGS, IF REACTIONS ARE NOT SHOWN ON CONTRACT STRUCTURAL DRAWINGS, DESIGN FOR ONE HALF (1/2) THE ALLOWABLE LOAD ON THE MEMBER, USING THE AISC "ALLOWABLE UNIFORM LOAD TABLES" WITH GIVEN BEAM SPAN, OR A MINIMUM OF 10 KIPS, WHICHEVER IS GREATEST. MEMBER FORCES AND REACTIONS HAVE BEEN REDUCED IN CONFORMANCE TO CODE PROVISIONS RELATED TO COMBINATIONS OF LOADINGS THAT INCLUDE WIND AND SEISMIC FORCES. NO FURTHER REDUCTIONS IN FORCES OR INCREASED IN ALLOWABLE STRESSES IS PERMITTED. CONNECTIONS MAY BE BOLTED OR WELDED UNLESS NOTED OTHERWISE.

3. FABRICATOR SHALL BE CATEGORY I CERTIFIED (CONVENTION STEEL STRUCTURES), OR A COMPANY SPECIALIZING IN PROJECTS OF THIS NATURE WITH A MINIMUM OF 5 YEARS OF EXPERIENCE.

- 4. ALL SHOP AND FIELD WELDING SHALL BE BY A CERTIFIED WELDER AND SHALL CONFORM TO AWS STANDARDS (LATEST EDITION). FIELD FILLET WELDS GREATER THAN 1/4" THICKNESS SHALL BE INSPECTED BY AN INDEPENDENT TESTING AGENCY.
- 5. WHERE CAMBER IS NOT PRESENT ERECT MILL CAMBER UP.
- 6. SEE ARCHITECTURAL DRAWINGS FOR MISCELLANEOUS STEEL NOT SHOWN ON STRUCTURAL
- 7. GALVANIZE OR PAINT ALL EXTERIOR EXPOSED STRUCTURAL STEEL, SEE ARCHITECTURAL DRAWINGS

TIMBER/WOOD/PLYWOOD/OSB:

1. A STRUCTURAL ENGINEER LICENSED TO PRACTICE IN THE STATE OF THE PROJECT SHALL DESIGN WOOD TRUSSES. DESIGN FOR ALL CODE REQUIRED LIVE, SNOW AND WIND LOADS. DESIGNS SHALL BE SEALED AND SIGNED BY HIM/HER AND SHALL BE SUBMITTED TO THE ARCHITECT FOR APPROVAL. FABRICATION SHALL BE BY A MEMBER OF THE TRUSS PLATE INSTITUTE, INC. SUBMIT SHOP DRAWINGS SHOWING LAYOUT OF TRUSSES AND STRUCTURAL FRAMING INCLUDING ARRANGEMENT, DIMENSIONS, MATERIALS, GRADES, STRESS VALUES, CONNECTORS, ANCHORAGE, AND RELATION TO ADJACENT WORK TO ARCHITECT FOR APPROVAL TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING MINIMUM LOADS:

FLOOR TRUSSES: TOP CHORD DEAD LOAD BOTTOM CHORD DEAD LOAD TOP CHORD LIVE LOAD	22.0 PSF 13.0 PSF 40.0 PSF
ROOF TRUSSES: TOP CHORD DEAD LOAD BOTTOM CHORD DEAD LOAD TOP CHORD LIVE LOAD BOTTOM CHORD LIVE LOAD (WHERE CODE REQUIRED)	8.0 PSF 12.0 PSF 20.0 PSF 10.0 PSF

GENERAL CONTRACTOR SHALL PROVIDE TRUSS SUPPLIER WITH SPRINKLER LAYOUT PLAN WITH HANGER LOCATIONS AND WEIGHTS. GENERAL CONTRACTOR SHALL PROVIDE TRUSS SUPPLIER WITH ALL OTHER HVAC/ELECTRICAL HANGING LOADS.

- 2. ALL TEMPORARY TRUSS BRACING REQUIRED FOR ERECTION, AS PER THE GUIDELINES SET FORTH BY THE TRUSS PLATE INSTITUTE PUBLICATION "HIB-91", SHALL BE PERMANENTLY ATTACHED AND REMAIN IN PLACE TO SERVE AS PERMANENT TRUSS BRACING UNLESS NOTED
- 3. TRUSS MEMBERS AND COMPONENTS SHALL NOT BE CUT, NOTCHED, DRILLED OR ALTERED IN ANY OTHER MANNER WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER RESPONSIBLE FOR DESIGNING THE TRUSSES.
- 4. LOAD BEARING PARTITIONS, JACKS, BEAMS AND COLUMN SUPPORTS MUST BE SOLID BLOCKED THROUGH FLOOR. TRUSSES AND PLYWOOD CANNOT SUPPORT CONCENTRATED POINT LOADS. I-JOIST MATERIAL SHOULD NOT BE USED AS BLOCKING UNDER CONCENTRATED POINT LOADS. ALL POINT LOADS MUST BE CARRIED TO FOUNDATIONS WITH ADEQUATE BLOCKING AND/OR BEAMS.
- TRUSS LAYOUTS AND CONFIGURATIONS SHOWN ARE SCHEMATIC ONLY AND MAY BE ALTERED AS REQUIRED. COORDINATE TRUSS CONFIGURATIONS WITH ALL ARCHITECTURAL REQUIREMENTS AND OTHER TRADES.
- 6. WOOD TRUSSES SHALL BE ERECTED IN ACCORDANCE WITH THE MANUFACTURERS RECOMMENDATIONS. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY AND PERMANENT BRACING AS REQUIRED FOR THE SAFE ERECTION AND PERFORMANCE OF THE TRUSSES. PLYWOOD/OSB ROOF SHEATHING SHALL RUN CONTINUOUS UNDER ALL VALLEY/OVERBUILD TRUSSES - TYPICAL
- 7. WOOD TRUSSES USED IN THE POOL BUILDING MUST BE INSPECTED BIANNUALLY. A MOISTURE BARRIER AT THE BOTTOM CHORD OF THE TRUSSES MUST BE INSTALLED. TRUSS PLATES SHALL HAVE TWO COATS OF EPOXY ASPHALT PAINT MINIMUM. AN ACTIVE ATTIC EXHAUST FAN MUST BE INSTALLED.
- 8. PROVIDE MINIMUM OF TWO (2) STUDS UNDER 2-PLY TRUSSES, THREE (3) STUDS UNDER 3-PLY TRUSSES AND FOUR (4) STUDS UNDER 4-PLY TRUSSES UNLESS NOTED OTHERWISE.
- 9. MICRO=LAM (LVL) TIMBER SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE DESIGN STRESSES: BENDING STRESS, Fb = 2,600 PSI, HORIZONTAL SHEAR STRESS, Fv = 285 PSI, AND MODULUS OF ELASTICITY, E = 1,900,000 PSI - CONNECT MULTIPLE MEMBERS TOGETHER AS PER MANUFACTURERS RECOMMENDATIONS.
- 10. ALL WOOD CONNECTORS, ANCHORS, FASTENERS, TIES, STRAPS, BASES, CAPS, ETC. SHALL BE SIMPSON "STRONG-TIE" (OR EQUIVALENT). CONNECTORS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURERS REQUIREMENTS. ALL CONNECTORS IN CONTACT WITH PRESSURE TREATED LUMBER SHALL MEET THE REQUIREMENTS OF ASTM A653 (CLASS G185) OR ASTM A153.
- 11. ALL FRAMED LUMBER SHALL BE SOUTHERN PINE NO. 2 (SURFACED AT 19% MOISTURE CONTENT) OR BETTER - UNLESS NOTED OTHERWISE. ALL STUD TOP AND BOTTOM PLATES SHALL BE SOUTHERN PINE STUD GRADE OR BETTER.
- 12. ALL LOAD BEARING TIMBER WALL STUDS SEE WALL STUD SCHEDULE SHEET SO02.
- 13. ALL TIMBER/WOOD/PLYWOOD/OSB IN CONTACT WITH CONCRETE OR MASONRY OR EXPOSED TO THE EXTERIOR SHALL BE PRESSURE TREATED.
- 14. ALL WOOD CONNECTIONS SHALL NOT BE LESS THAN THOSE SPECIFIED IN TABLE 2304.9.1 OF THE CURRENT NORTH CAROLINA BUILDING CODE/IBC UNLESS NOTED OTHERWISE. LEAD HOLES FOR LAG SCREWS SHALL BE IN ACCORDANCE WITH NDS REQUIREMENTS.
- 15. GENERAL CONTRACTOR SHALL COORDINATE LOCATION OF TRUSSES WITH OTHER TRADES -SHIFT TRUSSES A MAXIMUM OF 3 1/2" AS REQUIRED.
- 16. ALL NON TONGUE AND GROOVE PLYWOOD/OSB PANELS SHALL HAVE 1/8" GAP AT ALL PANEL EDGES. PROVIDE SIMPSON PSCL (OR EQUIVALENT) PLYWOOD CLIPS @ 24" AT PANEL EDGES OF ALL ROOF PLYWOOD/OSB SHEATHING. WHERE SHEATHING IS APPLIED TO BOTH SIDES OF A SHEAR WALL PROVIDE DOUBLE STUDS OR STAGGER SHEATHING JOINTS.
- 17. WALL SHEATHING SHALL LAP AND CONNECT TO FOUNDATION SILL PLATE AND LAP PAST WALL PLATES TO CONNECT TO UPPER STORY FLOOR PLATE - PROVIDE EDGE PATTERN NAILING. PROVIDE 2x BLOCKING AT ALL EDGES.

18. ALL TIMBER/WOOD POSTS GREATER THAN 5" IN SIZE SHALL BE SOUTHERN PINE, NO. 2 DENSE SR OR BETTER. TYPICAL UNLESS NOTED OTHERWISE.

FLAT ROOFS:

1. FLAT ROOFS SHALL HAVE CONTROLLED DRAINAGE PROVISIONS AND SHALL BE EQUIPPED WITH A SECONDARY DRAINAGE SYSTEM AT A HIGHER ELEVATION WHICH PREVENTS PONDING ON THE ROOF ABOVE THAT ELEVATION. THE SECONDARY DRAINAGE SYSTEM SHALL BE POSITIONED SO THAT A 3" MAXIMUM DEPTH OF WATER WILL POND ON THE ROOF DURING THE DESIGN RAINSTORM. THE DESIGN OF THE ROOF DRAINAGE, SECONDARY DRAINAGE AND/OR OVERFLOW SCUPPERS IS BEYOND THE SCOPE OF THE STRUCTURAL ENGINEER'S SCOPE OF SERVICES.

FOLDING PARTITION:

1. FOLDING PARTITION IS ASSUMED TO BE 10'-0" HIGH WITH A WEIGHT OF 12 PSF. MAXIMUM SUPPORT LIVE LOAD DEFLECTION SHALL BE LIMITED TO 1 1/2". GENERAL CONTRACTOR SHALL VERIFY WITH PARTITION SUPPLIER.

POST-INSTALLED ANCHORS:

POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONTRACT DOCUMENTS. CONTRACTOR SHALL OBTAIN APPROVAL FROM STRUCTURAL ENGINEER OF RECORD PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE GIVEN TO AVOID CONFLICTS WITH EXISTING STEEL REINFORCING. HOLES SHALL BE DRILLED AND CLEANED AS PER MANUFACTURERS RECOMMENDATIONS. ANCHORS SHALL BE INSTALLED AS PER MANUFACTURERS RECOMMENDATIONS. CONTRACTOR INSTALLING ADHESIVE ANCHORS SHALL BE TRAINED BY THE MANUFACTURERS REPRESENTATIVE. THIS TRAINING SHALL INCLUDE HOLE DRILLING, CLEANING AND INSTALLATION METHODS FOR CONSTRUCTION CONDITIONS ON THIS PROJECT.

*CONCRETE ANCHORS SEISMIC DESIGN CATEGORY A, B, C, D, E, AND F: 1) EXPANSION ANCHORS - "STRONG-BOLT 2" OR "STRONG-BOLT" BY SIMPSON STRONG-TIE, "KWIK BOLT TZ" BY HILTI OR EQUIVALENT, "POWER STUD+ SD1" BY POWERS FASTENERS - UNLESS NOTED OTHERWISE.

2) CONCRETE ADHESIVE ANCHORS - "SET-XP EPOXY-TIE" BY SIMPSON STRONG-TIE, "HIT-RE 500-SD" OR "HIT-HY 150 MAX-SD" BY HILTI OR EQUIVALENT - UNLESS NOTED OTHERWISE. 3) SCREW ANCHORS - "TITEN HD" BY SIMPSON STRONG-TI, "KWIK HUS-EZ" BY HILTI OR 4) SLEEVE ANCHORS - "HSL-3" BY HILTI OR EQUIVALENT

*MASONRY ANCHORS:

1) EXPANSION ANCHORS - "WEDGE-ALL" BY SIMPSON STRONG-TIE, "KWIK BOLT 3" BY HILTI OR EQUIVALENT - UNLESS NOTED OTHERWISE. 2) ADHESIVE ANCHORS (GROUT FILLED) - "SET EPOXY-TIE" BY SIMPSON STRONG-TIE, "HIT-HY 150 MAX" BY HILTI OR EQUIVALENT - UNLESS NOTED OTHERWISE. 3) ADHESIVE ANCHORS (HOLLOW CMU OR BRICK) - "SET EPOXY-TIE" BY SIMPSON STRONG-TIE, "HIT-HY 70" BY HILTI OR EQUIVALENT - UNLESS NOTED OTHERWISE. 4) SCREW ANCHORS - "TITEN HD" BY SIMPSON STRONG-TIE, "HUS-H" BY HILTI OR EQUIVALENT 5) SLEEVE ANCHORS - "SLEEVE-ALL" BY SIMPSON STRONG-TIE, "HLC" BY HILTI OR EQUIVALENT.

STAIR DESIGN:

STAIRS, LANDINGS AND HANDRAILS SHALL BE DESIGNED BY A STRUCTURAL ENGINEER LICENSED TO PRACTICE IN THE STATE OF OF THE PROJECT. DESIGNS SHALL BE SEALED AND SIGNED BY HIM/HER AND SHALL BE SUBMITTED TO THE ARCHITECT FOR APPROVAL. SUBMIT SHOP DRAWINGS SHOWING LAYOUT OF STAIRS AND STRUCTURAL FRAMING, INCLUDING ARRANGEMENT, DIMENSIONS, CONNECTIONS AND RELATION TO ADJACENT WORK TO ARCHITECT FOR APPROVAL.

LADDER AND HANDRAIL DESIGN:

1. LADDERS AND HANDRAILS SHALL BE DESIGNED BY A STRUCTURAL ENGINEER LICENSED TO PRACTICE IN THE STATE OF OF THE PROJECT. DESIGNS SHALL BE SEALED AND SIGNED BY HIM/HER AND SHALL BE SUBMITTED TO THE ARCHITECT FOR APPROVAL. SUBMIT SHOP DRAWINGS SHOWING LAYOUT OF LADDER AND HANDRAILS AND STRUCTURAL FRAMING, INCLUDING ARRANGEMENT. DIMENSIONS, CONNECTIONS AND RELATION TO ADJACENT WORK TO ARCHITECT FOR APPROVAL.

1. GENERAL CONTRACTOR SHALL COORDINATE/VERIFY ALL STRUCTURAL STEEL REQUIRED FOR GUIDE RAIL SUPPORT AND HOIST BEAM. PROVIDE W8x10 HOIST BEAM AND HSS 6x4x5/16 GUIDE RAIL UNLESS NOTED OTHERWISE. VERIFY/COORDINATE ALL STEEL LOCATIONS, DIMENSIONS AND ELEVATIONS - TYPICAL.

PRE-MANUFACTURED CANOPIES AND AWNINGS:

1. THE DESIGN, CONNECTION AND ALL ATTACHMENTS OF ALL CANOPIES AND AWNINGS SHALL BE THE RESPONSIBILITY OF THE CANOPY/AWNING SUPPLIER. THE GENERAL CONTRACTOR SHALL COORDINATE ALL ATTACHMENT REQUIREMENTS AND PROVIDE ADDITIONAL STUDS, BLOCKING ETC. AS REQUIRED.

SIMPSON ANCHOR TIEDOWN SYSTEMS (ATS):

. ATS IS AN ASSEMBLAGE OF STEEL COMPONENTS, WHICH INCLUDE RODS, PLATES, COUPLER NUTS, TAKE-UP DEVICES AND NUTS. STUDS, POSTS AND BLOCKING BY ENGINEER OF RECORD.

SIMPSON STRONG-TUE IS PROVIDING THE ANCHOR TIEDWON SYSTEM TO MEET THE DESIGN FORCES PROVIDED BY THE ENGINEER OF RECORD. THE EOR IS RESPONSIBLE FOR EVALUATING THE EFFECTS OF LUMBER SHRINKAGE AND ATS ELONGATION ON SHEARWALL DRIFT.

- GENERAL CONTRACTOR OR INSTALLER OF ATS SHALL CUT RODS TO LENGTH AS REQUIRED.
- 4. DO NOT WELD PRODUCTS UNLESS DRAWINGS SPECIFICALLY IDENTIFY A PRODUCT AS ACCEPTABLE FOR WELDING, OR UNLESS SPECIFIC APPROVAL FOR WELDING IS PROVIDED BY SIMPSON STRONG-TIE.
- 5. FULLY ENGAGE EACH ROD INTO THE SPECIFIED COUPLING NUT OR UNTIL EACH ROD CAN BE SEEN FULLY IN THE WITNESS HOLES.
- INSTALL NUTS AND ISOLATOR NUTS SNUG TIGHT, PLUS AN ADDITIONAL 1/2 TURN. 7. IN THE EVENT OF A DISCREPANCY BETWEEN THE STRUCTURAL DRAWINGS AND SIMPSON

FLOOD ZONE DATA(SEE CIVIL DRAWING COO3:

1. PROPERTY IS LOCATED IN FLOOD ZONE 'X' SHADED (AREAS OF 500 YEAR FLOOD WITH AVERAGE DEPTHS OF LESS THAN 1') ACCORDING TO PANEL 75, 22073C0075E, DATED MACH 15,

THE PROPERTY LOCATED ADJACENT TO THE SOUTH WEST END OF THE PROPERTY IS SHOWN TO BE ELEV. 67 MSL, ON PANEL 75, 22073C0075E, DATED MACH 15, 1994.

3. FINISH FLOOR ELEVATION OF THE BUILDING IS 71.50 MSL.

INSTALLATION DRAWINGS, THE STRUCTURAL DRAWINGS SHALL GOVERN.

SEE SOILS REPORT FOR FOOTING BEARING ELEVATION INFORMATION - SOILS ENGINEER SHALL FIELD VERIFY TOP OF FOOTING ELEVATIONS - TYPICAL

WGPM, Inc.

Wright • Gibson • Patton

STRUCTURAL ENGINEERING

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Charlotte, North Carolina 28277 704-542-7199 Fax: 704-542-7195 www.wgpminc.com

JOB NUMBER: 57-13



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WGPM, Inc. 11220 Elm Lane, Suite 201 Charlotte, NC 28277 Phone: (704) 542-7199 Fax: (704) 542-7195 Email: lwright@wgpminc.com

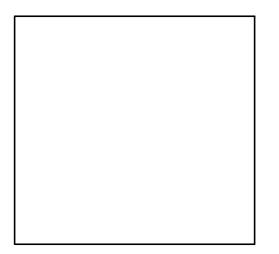
Ritter Engineering 1043 Arroyo Vista Lane Matthew, NC 28014 Phone: (704) 516-0385 Fax: (704) 644-1464 Email: shane@rittereng.com

MECHANICAL:
Shultz Engineering Group, P.C. 212 North McDowell St., Suite 204 Charlotte, NC 28204 Phone: (704) 334-7363 Fax:(704) 347-0093

Email: don@shultzeg.com

•		REVI	SIONS
	No.	Date	Description
	1	11.01.13	Pool Equip.,
			Ftgs & Rf Slope
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KEY PLAN

Southern Hospitality Services

Hampton Inn and Suites

5400 I-20 & Frontage Rd. Monroe, LA 71201

Drawing Title **General Notes**

Construction Documents Project No. MARCUS Ĉ. GIBSON

License No.32774

12-111 Prepared by AB/LW

Checked by HLW ^{le}September 16, 2013

NOTE! - PLYWOOD/OSB SHEAR WALLS MAIN WINDFORCE-RESITING SYSTEM SUBJECT TO SPECIAL INSPECTIONS 1705.4.1 INCLUDES PLYWOOD/OSB SHEATHING AND ATTACHMENT, BOTTOM & ANCHORS, TENSION TIES, HURRICANE TIES, STRAPS, BLOCKING PANELS, BOLTS, THREADED RODS, AT ALL EXTERIOR WALLS AND INTERIOR SHEAR WALLS (SW), PLYWOOD/OSB FLOOR AND ROOF SHEATHING AND ATTACHMENT.

NOTE! - PLYWOOD/OSB SHEAR WALLS ALL EXTERIOR WALLS AND INTERIOR SHEAR WALLS (SW) SHALL HAVE 7/16" EXTERIOR GRADE PLYWOOD/OSB SHEATHING. ATTACH SHEATHING TO SUPPORTING MEMBERS USING 8d NAILS. ALL EDGES TO BE BLOCKED WITH 2" NOMINAL FRAMING MEMBERS. NAILS SHALL BE PLACED NOT LESS THAN 3/8" FROM THE PANEL EDGE -SEE SCHEDUE (S002) FOR PANEL EDGE NAIL SPACING - NAIL @ 12" O.C. AT INTERMEDIATE FRAMING MEMBERS. LAYOUT PANELS STAGGERED AND PERPENDICULAR TO WALL STUDS.

NOTE!

ATTACH DOUBLE 2x6 WALL STUDS TOGETHER USING 2 ROWS OF 16d NAILS @ 24" O.C. - TYPICAL

NOTE!

PROVIDE SOLID WOOD BLOCKING FROM ALL STUDS AND MULTIPLE (GANG) STUDS TO BEAM/WALL, STUDS OR FOUNDATION BELOW -TYPICAL

NOTE!

ATTACH GANG STUDS (3 OR MORE STUDS) TOGETHER USING SIMPSON SDS1/4x6 SCREWS @ 12" O.C. STAGGERED EACH FACE - TYPICAL WHERE USING 3 OR MORE STUDS TOGETHER

NOTE!

ATTACH WOOD STUDS TO EACH SIDE OF STEEL COLUMNS USING 0.157"ø P.A.F. @ 12" O.C. STAGGERED - TYPICAL

3RD SPFS STUD 2x6 ● 16" O.C. INTERIOR SPFS NO. 2 2x6 ● 16" O.C. EXTERIOR OR SP STUD 2x6 ● 16" O.C. EXTERIOR SPFS STUD 2x6 ● 16" O.C. — INT. SHEAR WALLS SPFS STUD (2)2 - 2x6 — JAMBS SEE HEADER SCHEDUL 2ND SP STUD 2x6 ● 16" O.C. INTERIOR OR SPFS NO. 2 2x6 ● 16" O.C. EXTERIOR OR SP STUD 2x6 ● 16" O.C. EXTERIOR OR SP STUD 2x6 ● 16" O.C. EXTERIOR SPFS STUD 2 - 2x6 ● 16" O.C. — INT. SHEAR WALLS SPFS STUD 2 - 2x6 ● 16" O.C. — INT. SW - SEE S601 1ST SP NO. 2 2x6 ● 16" O.C. INTERIOR OR SPFS STUD 2 - 2x6 ● 16" O.C. INTERIOR OR SPFS STUD 2 - 2x6 ● 16" O.C. INTERIOR OR SPFS STUD 2 - 2x6 ● 16" O.C.	WALL STUD SCHEDULE.						
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		SPFS NO. 2	_	-	JAMBS	SEE HEADER SCHEDULE	

SP STUD INDICATES SOUTHERN PINE STUD GRADE SP NO. 2 INDICATES SOUTHERN PINE NO. 2 SPFS STUD INDICATES SPRUCE-PINE-FIR(SOUTH) STUD GRADE SPFS NO. 2 INDICATES SPRUCE-PINE-FIR(SOUTH) NO. 2

		SHEAR V	VALLS	SCHEDUL	E
MARK	FLOOR	PLYWOOD/OSB	NAIL	SPACING	TENSION TIE
SW1	1ST	7/16" ONE (1) SIDE OF WALL	8d	@ 4" O.C.	SIMPSON ATS EACH END OF WALL
	2ND	7/16" ONE (1) SIDE OF WALL	8d	© 6" O.C.	- SEE S601
	3RD	7/16" ONE (1) SIDE OF WALL	8d	© 6" O.C.	
	4TH	7/16" ONE (1) SIDE OF WALL	8d	© 6" O.C.	
SW2	1ST	7/16" ONE (1) SIDE OF WALL	8d	@ 4" O.C.	
– EXT.	2ND	7/16" ONE (1) SIDE OF WALL	8d	© 6" O.C.	
WALLS	3RD	7/16" ONE (1) SIDE OF WALL	8d	© 6" O.C.	
	4TH	7/16" ONE (1) SIDE OF WALL	8d	© 6" O.C.	

^{* 8}d @ 12" O.C. INTERMEDIATE

STRUCTURAL DESIGN CRITERIA:

DESIGN:

1. STRUCTURAL DESIGN CONFORMS TO THE REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE, 2009 EDITION AND ASCE 7-05.

2. BUILDING CATEGORY (T1604.5) II

3. FLOOR LIVE LOADS USED IN DESIGN (POUNDS PER SQUARE FOOT):

RESIDENCE AND CORRIDORS SERVING THEM 40 PSF PUBLIC ROOMS AND CORRIDORS SERVING THEM 100 PSF STAIRS 100 PSF

4. BUILDING CODE REQUIRED ROOF LIVE AND SNOW LOAD USED IN DESIGN (POUNDS PER SQUARE FOOT):

LIVE SNOW - Pg SNOW - Pf		20 PSF 5 PSF 9 PSF
SNOW EXPOSURE FACTOR, Ce SNOW LOAD IMPORTANCE FACTOR, Is SNOW THERMAL FACTOR, Ct	1.0 1.0 1.0	

5. DEAD LOADS USED IN DESIGN (POUNDS PER SQUARE FOOT)	:
FLOOR DEAD LOADS: FLOOR FINISH 3/4" GYPCRETE TOPPING 3/4" PLYWOOD HANGING MECHANICAL SPRINKLERS PARTITIONS TRUSSES	1 PSF 8 PSF 3 PSF 4 PSF 3 PSF 10 PSF 3 PSF
ROOF DEAD LOADS: ROOFING (SINGLE PLY MECHANICALLY FASTENED) INSULATION 3/4" PLYWOOD HANGING MECHANICAL SPRINKLERS TRUSSES	3 PSF 2 PSF 3 PSF 3 PSF 3 PSF 3 PSF 3 PSF
6. WIND LOAD DATA: BASIC WIND SPEED, V WIND IMPORTANCE FACTOR, Iw WIND EXPOSURE INTERNAL PRESSURE COEFFICIENT (2009 IBC 1604.1.4)	90 MPH 1.0 C ±0.18
COMPONENT AND CLADDING WIND PRESSURE	

<u>10sf</u> 22.2 PSF

27.3 PSF 21.3 PSF 17.0 PSF CALCULATED WIND BASE SHEARS (FOR MWFRS) Vx = 53.4K Vy = 218.2K

19.2 PSF

COMPLIANCE WITH ASCE 7-05 SECTION 11.7 ONLY? NO

7. SEISMIC LOAD DATA:

PILE SIZE, TYPE AND CAPACITY

NOTE!

SEISMIC DESIGN CATEGORY B, C & D	
SEISMIC IMPORTANCE FACTOR, Ie	1.0
SOIL SITE CLASS	D
SPECTRAL RESPONSE ACCELERATION - SHORT PERIOD, SDS	0.197g
SPECTRAL RESPONSE ACCELERATION - 1.0 SECOND, SD1	0.131g
SEISMIC DESIGN CATEGORY	В
BASIC SEISMIC-FORCE RESISTING SYSTEM	
BEARING WALL SYSTEM/LIGHT FRAMED WALL W/ WOOD SHEAR W	ALLS
RESPONSE MODIFICATION COEFFICIENT, R	6 1/2
DEFLECTION AMPLIFICATION FACTOR, Cd	4
BUILDING HEIGHT LIMIT, FEET	H = NL

EQUIVALENT LATERAL-FORCE PROCEDURE SEISMIC BASE SHEAR Vx = 54.8K Vy = 65.4K

ARCHITECTURAL, MECHANICAL, COMPONENTS ANCHORED? SEE CHAPTER 13 OF ASCE 7-05

LATERAL DESIGN CONTROLLED BY: X WIND-Y X SEISMIC-X SOIL BEARING CAPACITIES: FIELD TEST (PROVIDED COPY OF TEST REPORT) 1500/2000 PSF PRESUMPTIVE BEARING CAPACITY

SEE SOILS REPORT FOR FOOTING BEARING ELEVATION

INFORMATION - SOILS ENGINEER SHALL FIELD VERIFY

Wright + Gibson + Patton STRUCTURAL ENGINEERING

11220 Elm Lane, Suite 201

Charlotte, North Carolina 28277 704-542-7199 Fax: 704-542-7195 www.wgpminc.com

JOB NUMBER: 57-13

TOP OF FOOTING ELEVATIONS - TYPICAL

MISHRA ARCHITECTURE PLLC

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EMÁIL:ashish@mishraarch.com WEB: www.mishraarch.com <u>CIVIL:</u>
James W Braswell and Associates 1005 North 4th Street

Phone: Fax: Email:braswellassociates@yahoo.com

STRUCTURAL: WGPM, Inc. 11220 Elm Lane, Suite 201 Charlotte, NC 28277 Phone: (704) 542-7199 Fax: (704) 542-7195

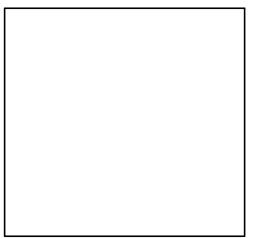
Monroe, LA 71201

Email: lwright@wgpminc.com ELECTRICAL:
Ritter Engineering 1043 Arroyo Vista Lane Matthew, NC 28014 Phone: (704) 516-0385 Fax: (704) 644-1464 Email: shane@rittereng.com

MECHANICAL:
Shultz Engineering Group, P.C. 212 North McDowell St., Suite 204 Charlotte, NC 28204 Phone: (704) 334-7363 Fax:(704) 347-0093 Email: don@shultzeg.com

	REVI	SIONS
No.	Date	Description
1	11.01.13	Pool Equip.,
		Ftgs & Rf Slope

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KEY PLAN

Southern Hospitality Services

Hampton Inn and Suites

5400 I-20 & Frontage Rd. Monroe, LA 71201

Drawing Title General Notes

Construction Documents

Prepared by

12-111 AB/LW Checked by HLW ^{ate}September 16, 2013



^{*} ALL TENSION TIES ARE SIMPSON OR EQUIVALENT

THIS STATEMENT OF SPECIAL INSPECTIONS IS SUBMITTED AS A CONDITION FOR PERMIT ISSUANCE IN ACCORDANCE WITH THE SPECIAL INSPECTIONS REQUIREMENTS OF THE BUILDING CODE. IT INCLUDES A SCHEDULE OF SPECIAL INSPECTION SERVICES APPLICABLE TO THIS

THE SPECIAL INSPECTOR SHALL KEEP RECORDS OF ALL INSPECTIONS AND SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, STRUCTURAL ENGINEER AND ARCHITECT OF RECORD. DISCOVERED DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF SUCH DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL, STRUCTURAL ENGINEER AND ARCHITECT OF RECORD. THE SPECIAL INSPECTION PROGRAM DOES NOT RELIEVE THE CONTRACTOR OF HIS OR HER RESPONSIBILITIES.

INTERIM REPORTS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL, OWNER, STRUCTURAL ENGINEER AND ARCHITECT OF RECORD.

A FINAL REPORT OF SPECIAL INSPECTIONS DOCUMENTING COMPLETION OF ALL REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED PRIOR TO ISSUANCE OF A CERTIFICATE OF USE AND OCCUPANCY.

JOB SITE SAFETY AND MEANS AND METHODS OF CONSTRUCTION ARE SOLELY THE RESPONSIBILITY OF THE CONTRACTOR.

STATEMENT OF SPECIAL INSPECTIONS (INTERNATIONAL BUILDING CODE, 2009 EDITION, CHAPTER 17):

	REQUIRED	VERIFICATIO	N AND INSPEC	TION OF SOILS	
	VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD	IBC REFERENCE
1.	VERIFY MATERIALS BELOW FOOTINGS AND SLAB ON GRADE ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY.	-	х	-	CHAPTER 18
2.	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	-	Х	-	CHAPTER 18
3.	PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS.	-	Х	-	CHAPTER 18
4.	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL.	х	1	-	CHAPTER 18
5.	PRIOR TO PLACEMENT OF CONTROLLED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	-	Х	-	CHAPTER 18

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD	IBC REFERENCE
. MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS:	CONTINUOUS	TENTODIO	NEI ENENCED STANDAND	THE NET ENERGE
A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	х	APPLICABLE ASTM MATERIAL SPECIFICATIONS: AISC 360, SECTION A3.3	-
B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	-	х	-	-
. INSPECTION OF HIGH-STRENGTH BOLTING:				
A. BEARING-TYPE CONNECTIONS.	-	Х	A100 700 CEOTION NO E	4704 7 7
B. SLIP-CRITICAL CONNECTIONS.	х	Х	AISC 360, SECTION M2.5	1704.3.3
. MATERIAL VERIFICATION OF STRUCTURAL STEEL:				
A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	-	ASTM A 6 OR ASTM A 568	1708.4
B. MANUFACTURER'S CERTIFIED MILL TEST REPORTS.	-	-	ASTM A 6 OR ASTM A 568	
. MATERIAL VERIFICATION OF WELD FILLER MATERIALS:				
A. IDENTIFICATION MARKINGS TO CONFORM TO ASW STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	-	AISC 360, SECTION A3.5	-
B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	-	-	-	-
. INSPECTION OF WELDING: A. STRUCTURAL STEEL:	-	-		
 COMPLETE AND PARTIAL PENETRATION GROOVE WELDS. 	х	-		
2. MULTIPASS FILET WELDS.	х	-	AWS D1.1	1704.3.1
3. SINGLE-PASS FILET WELDS > ⅓6"	х	-		
4. SINGLE-PASS FILET WELDS < 1/16"	-	Х		
5. FLOOR AND DECK WELDS	-	Х	AWS D1.3	-
B. REINFORCING STEEL	-	-		
1. VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAT ASTM A 706	-	х		
2. REINFORCING STEEL-RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL REINFORCED CONCRETE SHEAR WALLS AND SHEAR REINFORCEMENT.	Х	-	AWS D1.4 ACI 318: 3.5.2	-
3. SHEAR REINFORCEMENT.	х	-		
4. OTHER REINFORCING STEEL.	-	Х		
INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENT:	-	Х		4704 7 0
 A. DETAILS SUCH AS BRACING AND STIFFENING. B. MEMBER LOCATIONS. C. APPLICATION OF JOINT DETAILS AT EACH CONNECTION. 	- - -	-	-	1704.3.2

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD	IBC REFERENCE	
INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, AND PLACEMENT.	-	х	ACI 318: 3.5, 7.1-7.7	1913.4	
. INSPECTION OF REINFORCING STEEL WELDING IN ACCORDANCE WITH TABLE 1704.3 ITEM 5B.	-	-	AWS D1.4 ACI 318: 3.5.2	-	MISI
. INSPECT BOLTS TO BE INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED.	x	-	-	1911.5	ARCHITECTU 6800 S Creek Rd, Cha
. VERIFYING USE OF REQUIRED DESIGN MIX.	-	х	ACI 318: CH. 4, 5.2-5.4	1904.2.2, 1913.2, 1913.3	Ph:(704) 625-6554 Fa EMAIL:ashish@mi WEB: www.mish
AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	Х	-	ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8	1913.10	CIVIL: James W Braswell and A 1005 North 4th Street Monroe, LA 71201 Phone:
. INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	х	-	ACI 318: 5.9, 5.10	1913.6, 1913.7 1913.8	Fax: Email:braswellassociates
. INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	-	х	ACI 318: 5.11-5.13	1913.9	STRUCTURAL: WGPM, Inc. 11220 Elm Lane, Suite 20
. INSPECTION OF PRESTRESSED CONCRETE: A. APPLICATION OF PRESTRESSING FORCES. B. GROUTING OF BONDED PRESTRESSING TENDONS IN THE SEISMIC-FORCE-SYSTEM.	X X	-	ACI 318: 18.20 ACI 318: 18.18.4	-	Charlotte, NC 28277 Phone: (704) 542-7199 Fax: (704) 542-7195 Email: lwright@wgpminc.
. ERECTION OF PRECAST CONCRETE MEMBERS.	-	х	ACI 318: CH. 16	-	ELECTRICAL: Ritter Engineering 1043 Arroyo Vista Lane
O. VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS	-	х	ACI 318: 6.2	-	Matthew, NC 28014 Phone: (704) 516-0385 Fax: (704) 644-1464 Email: shane@rittereng.c
FROM BEAMS AND STRUCTURAL SLABS. .INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	-	х	ACI 318: 6.1.1	-	MECHANICAL: Shultz Engineering Grou 212 North McDowell St., Charlotte, NC 28204 Phone: (704) 334-7363 Fax:(704) 347-0093

	FREQUENCY O	F INSPECTION	REI	FERENCE FOR CRITER	I A]
INSPECTION TASK		PERIODICALLY DURING TASK LISTED	IBC SECTION	ACI 530/ASCE 5/TMS 402 ^d	ACI 530/ASCE 6/TMS 602 ^d	
I. AS MASONRY CONSTRUCTION BEGINS, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:] -
A. PROPORTIONS OF SITE-PREPARED MORTAR.		Х			ART. 2.6A] [
B. CONSTRUCTION OF MORTAR JOINTS.	-	X	-	-	ART. 3.3B]
C. LOCATION OF REINFORCEMENT AND CONNECTORS, PRESTRESSING TENDONS AND ANCHORAGES.		Х			ART. 3.4,3.6A	
D. PRESTRESSING TECHNIQUE.		Х			ART. 3.6B	
E. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES.		x			ART. 2.4B, 2.4H	
2. THE INSPECTION PROGRAM SHALL VERIFY:						In
A. SIZE AND LOCATION OF STRUCTURAL ELEMENTS.	-	Х		-	ART. 3.3G	in fo
B. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION.	1	Х	-	SEC. 1.2.2(e), 2.1.4, 3.1.6	-	ar A re w au
C. SPECIFIED SIZE, GRADE AND TYPE OF REINFORCEMENT.	-	х	-	SEC. 1.13	ART. 2.4, 3.4	24 M
D. WELDING OF REINFORCING BARS.	х	-	-	SEC. 2.1.10.7.2, 3.3.3.4(B)	-	
E. PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURE ABOVE 90°F).	-	х	SEC. 2104.3, 2104.4	-	ART. 1.8C, 1.8D	
F. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE.	-	х	-	-	ART. 3.3B	
3. PRIOR TO GROUTING, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:						
A. GROUT SPACE IS CLEAN.		Х		-	ART. 3.2D	
B. PLACEMENT OF REINFORCEMENT AND CONNECTORS.	_	X	_	SEC. 1.13	ART. 3.4	_
C. PROPORTION OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDDED TENDONS.		Х		-	ART. 2.6B	KE'
D. CONSTRUCTION OF MORTAR JOINTS.		Х		-	ART 3.3B	1
4. GROUT PLACEMENT SHALL BE VERIFIED TO ENSURE COMPLIANCE WITH CODE AND CONSTRUCTION DOCUMENT PROVISIONS.	х	-	-	-	ART. 3.5	
5. PREPARATION OF ANY REQUIRED GROUT SPECIMENS, MORTAR SPECIMENS AND/OR PRISMS SHALL BE OBSERVED.	х	-	SEC. 2105.2.2, 2105.3	-	ART. 1.4	
6. COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS AND THE APPROVED SUBMITTALS SHALL BE VERIFIED.	-	х	-	-	ART. 1.5] -

ARCHITECTURE PLLC

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ELECTRICAL:
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Shultz Engineering Group, P.C. 212 North McDowell St., Suite 204 Charlotte, NC 28204 Phone: (704) 334-7363 Fax:(704) 347-0093

		SIONS	
	No.	Date	Description
	1	11.01.13	Pool Equip.,
			Ftgs & Rf Slope
6A			

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Southern Hospitality Services

Hampton Inn and Suites

5400 I-20 & Frontage Rd. Monroe, LA 71201

Drawing Title Special Inspections

Construction Documents MARCUS C. GIBSON License No.32774

WGPM, Inc.
Wright · Gibson · Patton
STRUCTURAL ENGINEERING

11220 Elm Lane, Suite 201 Charlotte, North Carolina 28277 704-542-7199 Fax: 704-542-7195 www.wgpminc.com

JOB NUMBER: 57-13

12-111 AB/LW Checked by HLW

^{Date}September 16, 2013

SEISMIC RESISTANCE

Verification/Inspection	Agent No.	Inspecti	ons	Referenced	IBC	Notes	Design	
,,,	/ MQIA	Cont.	Periodic	Standard	Reference		J	
1. Structural Wood: a.Periodic special inspection is required for nailing, bolting, anchoring and framing components within the seismic—force—resisting system, including wood shear walls, wood diaphragms, drag struts, braces, shear panels and hold downs		-	X		1707.3		W	

WALL PANELS AND VENEERS

Verification/Inspection	Agent No.	Inspections		Referenced	IBC	Notes	Design
vormodiony moposition	MQIA	Cont.	Periodic	Standard	Reference	110100	· g. ·
 Architectural wall panels a. Interior b. Exterior 	1 1	<u>-</u>	X 50% X 50%		1704.10		Α
2. Masonry veneer (see 2.1-2.6)			Х		1704.5		Α
3. Exterior insulations and finish systems (EIFS) See Note #1 below			Х		1704.12		Α

NOTE #1: Special Inspections for EIFS are not required when installed over a water resistive barrier, with a means of draining moisture to the exterior; and not required where installed over concrete or masonry walls.

*NOTE!

PROVIDE #5 @ 24" O.C. VERTICAL

AT ALL CMU STAIR AND ELEVATOR

BOND BEAM AT LANDING ELEVATIONS

WALLS - PROVIDE 8" DEEP CONT.

- GROUT ALL CELLS SOLID

<u>6</u> \$302

WF55

(21)

TRANSVERSE REINF.

COMMENTS

REINFORCING

4 #4 x3'-0" EACH WAY

7 #4 x4'-0" EACH WAY

5 #5 x4'-6" EACH WAY

5 #5 x4'-6" E.W.T. & B.

6 #5 x5'-0" E.W.T. & B.

7 #5 x5'-6" EACH WAY 8 #5 x6'-0" EACH WAY

8 #5 x6'-0" E.W.T. & B.

8 #6 x7'-0" EACH WAY

H

FOOTING SCHEDULE (2000 PSF)

THICK.

1'-0" 1'-0" 1'-0"

1'-6"

1'-8"

1'-0" 1'-0"

SOILS LABORATORY SHALL VERIFY SOIL BEARING PRESSURE AT BOTTOM OF EACH FOOTING PRIOR TO

WIDTH | THICKNESS | LONGITUDINAL REINF. |

PLACING CONCRETE. STRUCTURAL FILL SUPPORTING SLAB OR FOOTINGS SHALL BE PLACED IN MAXIMUM 8" LIFTS, WITH EACH 8" LIFT TESTED BY LABORATORY FOR COMPACTION LISTED IN GENERAL NOTES.

SIZE

3'-6"x3'-6"

4'-6"x4'-6"

5'-0"x5'-0"

5'-0"x5'-0"

5'-6"x5'-6"

6'-0"x6'-0"

6'-6"x6'-6"

6'-6"x6'-6"

7'-6"x7'-6"

E.W.T. & B. INDICATES EACH WAY TOP AND BOTTOM

MARK

F35 F45 F50 F50* F55* F60 F65

F65*

F75

6 S301

SEE 9/301 FOR THICKENED SLAB UNDER WASHER - COORDINATE W/ ARCH. AND PLUMBING DWGS.

6 \$301

4" CONCRETE SLAB (3000 PSI) WITH 6x6-W1.4xW1.4

W.W.F. OR FIBERMESH 300 (SEE GEN. NOTES) — ON

VAPOR RETARDER ON 4" DRAINAGE FILL - LOCATE

W.W.F. IN UPPER THIRD OF SLAB - PROVIDE SLAB

CONSTRUCTION/CONTROL JOINT @ 10'-0" MAX. (SEE

1/S301) - F.F.E. 71.50' (0'-0")

NOTE!

GENERAL CONTRACTOR SHALL COORDINATE/VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS

WF55

NOTE! — INDICATES SIMPSON ANCHOR TIEDOWN SYSTEM — SEE DETAILS

7/S302 AND ON SHEET S602

(-2'-0")

(22 REQ'D)

F75__

- POOL/SPA DESIGNED

By Pool Supplier

6 S301

11 S301

WF55

NOTE!

SEE 8/S301 AND ARCH/PLUMBING DWGS. FOR SUMP PUMP PIT

WIDEN CONT. WALL FOOTING AND-

PROVIDE ADDITIONAL #4 @ 12" O.C. TOP AND BOTTOM — TYP.

(-2'-0")

24" FTG. STEP-

(15)

(-4'-0")

NOTE!

ALL EXTERIOR WALLS ARE SHEAR WALLS SW2. SW1 INDICATES INTERIOR SHEAR WALL. SEE SHEET S002 FOR SHEAR WALL SCHEDULE.

NOTE!

SEE SOILS REPORT FOR FOOTING BEARING ELEVATION INFORMATION - SOILS ENGINEER SHALL FIELD VERIFY TOP OF FOOTING ELEVATIONS - TYPICAL

12 S301

W12x22 W/ HSS 6x4x516 ELEVATOR

GUIDE RAIL - SEE 13/S301 FOR

BEAM BEARING DETAIL - TYP.

(-2'-0")



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Phone: (704) 334-7363 Fax:(704) 347-0093 Email: don@shultzeg.com

REVISIONS No. Date Description 1 11.01.13 Pool Equip., Ftgs & Rf Slope

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KEY PLAN

Southern Hospitality

Hampton Inn and Suites

NOTE!
CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS

(12) (S301)

5400 I-20 & Frontage Rd. Monroe, LA 71201

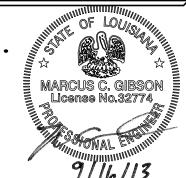
Foundation and SHEET S302 - PROVIDE W1.7 (9 GA.) TRUSS TYPE

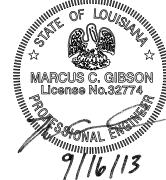
Services

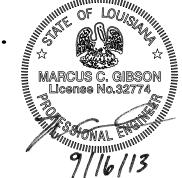
HORIZONTAL JOINT REINFORCING @ 16" MAX. ALL Construction Documents

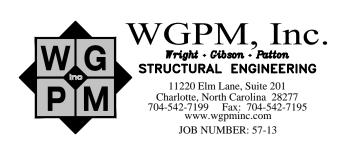
MARCUS C. GIBSON License No.32774

Prepared by Checked by









OTHERWISE

#5 @ 24" VERTICAL ALL 8" CMU WALLS - SEE

MASONRY WALLS - TYPICAL UNLESS NOTED

12-111 AB/LW HLW ate September 16, 2013

#3x2'-6" @ 24" O.C. TOP SEE 11/S301 3 #4 TOP WF30 | 3'-0" 1'-3" #4x2'-6" @ 24" O.C. BOTTOM 3 #5 BOTTOM SEE 12/S301 5 #4 TOP #4x3'-6" @ 24" O.C. TOP SEE 11/S301 WF40 | 4'-0" 5 #5 BOTTOM #4x3'-6" @ 16" O.C. BOTTOM SEE 12/S301 6 #4 TOP #4x5'-0" @ 24" O.C. TOP SEE 6/S301 WF55 5'-6" 6 #6 BOTTOM #5x5'-0" @ 16" O.C. BOTTOM SEE 4 & 6/S302

WALL FOOTING SCHEDULE 4,000 PSF SOIL BEARING

(19)

1 FOUNDATION AND FLOOR SLAB PLAN

ALL ELEVATIONS REFERENCED () FROM FINISH FLOOR ELEVATION 71.50' (0-0). INTERIOR TOP OF FOOTING - T.O.F. (-1'-4") TYPICAL UNLESS NOTED OTHERWISE. EXTERIOR TOP OF FOOTING - T.O.F. (-1'-4") TYPICAL UNLESS NOTED OTHERWISE. CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS.

SW1 INDICATES SHEAR WALL 1 - SEE SHEAR WALL SCHEDULE ON SHEET S002 TYPICAL.

9. TOP OF PIER - T.O.P. (-1'-0") TYPICAL UNLESS NOTED OTHERWISE - SEE 1/S303.

SEE SHEET S001 AND S002 FOR GENERAL NOTES, WALL STUDS AND WALL SHEATHING NOTES. SEE SHEET S302 FOR COLUMN BASE PLATE AND ANCHOR ROD SCHEDULE. ALL FOOTING STEPS 24" TYPICAL UNLESS NOTED OTHERWISE - SEE 2/S301.

*NOTE!

SEE 9/301 FOR THICKENED SLAB UNDER WASHER - COORDINATE W/ ARCH. AND PLUMBING DWGS.

NOTE!

PROVIDE #5 @ 24" O.C. VERTICAL AT ALL
CMU STAIR AND ELEVATOR WALLS — PROVIDE 8" DEEP CONT. BOND BEAM AT LANDING ELEVATIONS — GROUT ALL CELLS

4" CONCRETE \$LAB (3000 PSI) WITH 6x6-W1.4xW1.4

W.W.F. OR FIBERMESH 300 (SEE GEN. NOTES) - ON

VAPOR RETARDER ON 4" DRAINAGE FILL - LOCATE

W.W.F. IN UPPER THIRD OF SLAB — PROVIDE SLAB

1/S301) - F.F.E. 71.50' (0'-0")

CONSTRUCTION/CONTROL JOINT @ 10'-0" MAX. (SEE

5 S301

WF40

GENERAL CONTRACTOR SHALL COORDINATE/VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS

NOTE!

SEE 8/S301 AND ARCH/PLUMBING DWGS. FOR SUMP PUMP PIT

NOTE!

ALL EXTERIOR WALLS ARE SHEAR WALLS SW2. SW1 INDICATES INTERIOR SHEAR WALL. SEE SHEET S002 FOR SHEAR WALL SCHEDULE.

NOTE!

S302

SEE SOILS REPORT FOR FOOTING BEARING ELEVATION INFORMATION - SOILS ENGINEER SHALL FIELD VERIFY TOP OF FOOTING ELEVATIONS - TYPICAL

-D.7



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CIVIL: James W Braswell and Associates 1005 North 4th Street Monroe, LA 71201 Phone:

Email:braswellassociates@yahoo.com

STRUCTURAL: WGPM, Inc.

Fax:

11220 Elm Lane, Suite 201 Charlotte, NC 28277 Phone: (704) 542-7199 Fax: (704) 542-7195 Email: lwright@wgpminc.com

ELECTRICAL: Ritter Engineering 1043 Arroyo Vista Lane

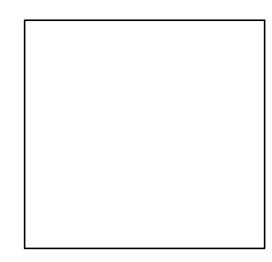
Matthew, NC 28014 Phone: (704) 516-0385 Fax: (704) 644-1464 Email: shane@rittereng.com

MECHANICAL:
Shultz Engineering Group, P.C. 212 North McDowell St., Suite 204 Charlotte, NC 28204 Phone: (704) 334-7363 Fax:(704) 347-0093

Email: don@shultzeg.com

REVISIONS						
No.	Date	Description				
1	11.01.13	Pool Equip.,				
		Ftgs & Rf Slope				
2	03.11.14	Canopy				

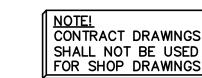
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KEY PLAN

Southern Hospitality Services

Hampton Inn and Suites



#5 @ 24" VERTICAL ALL 8" CMU WALLS - SEE

OTHERWISE

Wright + Gibson + Patton

STRUCTURAL ENGINEERING

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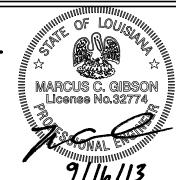
JOB NUMBER: 57-13

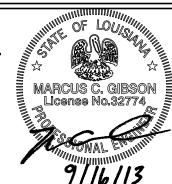
NOTE! 5400 I-20 & Frontage Rd. SHALL NOT BE USED | Monroe, LA 71201

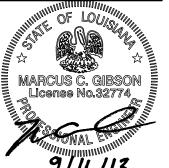
| Foundation and SHEET S302 - PROVIDE W1.7 (9 GA.) TRUSS TYPE Floor Slab Plan

HORIZONTAL JOINT REINFORCING @ 16" MAX. ALL MASONRY WALLS — TYPICAL UNLESS NOTED Construction Documents









12-111 Prepared by AB/LW Checked by HLW ate September 16, 2013

MARK | WIDTH | THICKNESS | LONGITUDINAL REINF. TRANSVERSE REINF. COMMENTS #3x2'-6" @ 24" O.C. TOP 3 #4 TOP SEE 11/S301 WF30 3'-0" 1'-3" #4x2'-6" @ 24" O.C. BOTTOM 3 #5 BOTTOM SEE 12/S301 5 #4 TOP #4x3'-6" @ 24" O.C. TOP SEE 11/S301 WF40 4'-0" 1'-4" 5 #5 BOTTOM #4x3'-6" @ 16" O.C. BOTTOM SEE 12/S301 6 #4 TOP #4x5'-0" @ 24" O.C. TOP SEE 6/S301 WF55 | 5'-6" 6 #6 BOTTOM #5x5'-0" @ 16" O.C. BOTTOM SEE 4 & 6/S302

WALL FOOTING SCHEDULE 4,000 PSF SOIL BEARING

FOOTING SCHEDULE (2000 PSF)

		•	
MARK	SIZE	THICK.	REINFORCING
F35 F45 F50 F50* F55* F60 F65 F65*	3'-6"x3'-6" 4'-6"x4'-6" 5'-0"x5'-0" 5'-0"x5'-0" 5'-6"x5'-6" 6'-0"x6'-0" 6'-6"x6'-6" 6'-6"x6'-6" 7'-6"x7'-6"	1'-0" 1'-0" 1'-6" 1'-0" 1'-0" 1'-0" 1'-6" 1'-2"	4 #4 x3'-0" EACH WAY 7 #4 x4'-0" EACH WAY 5 #5 x4'-6" EACH WAY 5 #5 x4'-6" E.W.T. & B. 6 #5 x5'-0" E.W.T. & B. 7 #5 x5'-6" EACH WAY 8 #5 x6'-0" EACH WAY 8 #5 x6'-0" E.W.T. & B. 8 #6 x7'-0" EACH WAY

SOILS LABORATORY SHALL VERIFY SOIL BEARING PRESSURE AT BOTTOM OF EACH FOOTING PRIOR TO PLACING CONCRETE. STRUCTURAL FILL SUPPORTING SLAB OR FOOTINGS SHALL BE PLACED IN MAXIMUM 8" LIFTS, WITH EACH 8" LIFT TESTED BY LABORATORY FOR COMPACTION LISTED IN GENERAL NOTES. E.W.T. & B. INDICATES EACH WAY TOP AND BOTTOM

NORTH		FO	UN	IDATIO	<u>N</u>	AND	FL(OOR_	SLAE	3 PI	_AN	
	S102	1/8"	' = 1	' -0"								
21		NOTE	ES:									
		1	ΔΙΙ	FLEVATIO	NS R	REFERENCE	D () FROM	FINISH	FLOOR	FLEV	ΔΤ

ALL ELEVATIONS REFERENCED () FROM FINISH FLOOR ELEVATION 71.50' (0-0). INTERIOR TOP OF FOOTING - T.O.F. (-1'-4") TYPICAL UNLESS NOTED OTHERWISE. EXTERIOR TOP OF FOOTING - T.O.F. (-1'-4") TYPICAL UNLESS NOTED OTHERWISE.
CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS.
SEE SHEET S001 AND S002 FOR GENERAL NOTES, WALL STUDS AND WALL SHEATHING NOTES.

SEE SHEET S302 FOR COLUMN BASE PLATE AND ANCHOR ROD SCHEDULE. ALL FOOTING STEPS 24" TYPICAL UNLESS NOTED OTHERWISE - SEE 2/S301. 8. SW1 INDICATES SHEAR WALL 1 - SEE SHEAR WALL SCHEDULE ON SHEET SOO2 TYPICAL.

H1 3 - 2x6 + PLYWOOD FILLER H2 3 - 2x8 + PLYWOOD FILLER H3 3 - 2x12 + PLYWOOD FILLER

S1 2 - 2x6 SILL

J1 2 - 2x6 JACK STUDS + 1 - 2x6 BYPASS STUDS J2 2 - 2x6 JACK STUDS + 2 - 2x6 BYPASS STUDS J3 2 - 2x6 JACK STUDS + 3 - 2x6 BYPASS STUDS J4 3 - 2x6 JACK STUDS + 1 - 2x6 BYPASS STUDS

1. ALL HEADERS ARE H1, ALL SILLS ARE S1 AND ALL JAMB STUDS ARE J1 UNLESS NOTED OTHERWISE.

2. PROVIDE DOUBLE JACK (CRIPPLE) STUDS UNDER EACH END OF ALL HEADERS AND DOUBLE BYPASS (KING) STUDS EACH SIDE OF JACK STUDS (U.N.O.) - SEE 8/S302.

PROVIDE #5 @ 24" O.C. VERTICAL -

AT ALL CMU STAIR AND ELEVATOR

WALLS - PROVIDE 8" DEEP CONT.

- GROUT ALL CELLS SOLID

BOND BEAM AT LANDING ELEVATIONS

NOTE!

OPEN

23/32" TONGUE AND GROOVE PLYWOOD/OSB SUBFLOOR WITH

SPAN RATING OF 48/24 OR BETTER, ATTACH TO SUPPORTING

MEMBERS USING 10GA. x 2" LONG SCREWS @ 6" O.C. EDGES

AND @ 12" O.C. INTERMEDIATE - GLUE TO ALL SUPPORTING

MEMBERS. LAYOUT STAGGERED AND PERPENDICULAR TO

FIN. FL. (+12-0) - T.O.PLYWOOD + 2ND FL.

(22)

SUPPORTING MEMBERS. SEE ARCH. FOR GYPCRETE TOPPING.

(21)

ALL EXTERIOR WALLS ARE SHEAR WALLS SW2. SW1 INDICATES INTERIOR SHEAR WALL. SEE SHEET S002 FOR SHEAR WALL SCHEDULE.

H1-J1

S402

H2-J2

H1-J1 👭 H1-J1

Щ

W21x44

51/4x11% LVL - J4

51/4x11% LVL - J4

51/4x111/8 LVL - J4

W21x44

514x1176 LVL - J4 H3-J1 514x1176 LVL - J4 514x16 LVL - 3

NOTE! SEE 11/S403 FOR FOLDING PARTITION HANGER DETAIL - TYPICAL

SEE SHEET SO02 FOR WALL STUDS AND PLYWOOD/OSB WALL SHEATHING - TYPICAL

ROOF SHEATHING: 5/8" (19/32") EXTERIOR RATED

| PLYWOOD/OSB ROOF DECK - LAYOUT STAGGERED

AND PERPENDICULAR TO ROOF TRUSSES — PROVIDE

w" gap around each sheet use simpson pscl

SHEATHING CLIPS BETWEEN TRUSSES (24" MAX AND

OVER SUPPORTS) - ATTACH TO ROOF TRUSSES W/

H2 - TYP. U.N.O.

5 \$403

W30x99

10d x 2 1/2" LONG NAILS AT 6" O.C. AROUND

SUPPORTED EDGES AND 12" O.C. INTERMEDIATE

(8 (S403)

S402

51/4x117/8 LVL - J4

9 \$403

S404

- 2x6 **@** 24"

W16x31

S402

H1-J2 H1-J2

ROOF SHEATHING: 5/8" (19/32") EXTERIOR RATED

PLYWOOD/OSB ROOF DECK - LAYOUT STAGGERED

| w" GAP AROUND EACH SHEET USE SIMPSON PSCL

AND PERPENDICULAR TO ROOF TRUSSES - PROVIDE

SHEATHING CLIPS BETWEEN TRUSSES (24" MAX AND

OVER SUPPORTS) - ATTACH TO ROOF TRUSSES W/

10d x 2 1/2" LONG NAILS AT 6" O.C. AROUND SUPPORTED EDGES AND 12" O.C. INTERMEDIATE

J2 - TYP. U.N.O.

MECHANICAL DUCTWORK RUNS THROUGH TRUSSES - COORDINATE DIAGONALS W/ MECH. CONTR.

NOTE:

4 S401

H1-J1 +51/4x117/8 LVL - J4

- INDICATES SIMPSON ANCHOR TIEDOWN SYSTEM - SEE DETAILS

on sheet s601 (22 req'd)

(15)

W12x22 W/ HSS 6x4x546

BEARING DETAIL - TYP.

51/4x16 LVL - J4

ELEVATOR GUIDE RAIL -SEE 6/S403 FOR BEAM

MECHANICAL CONTR. SHALL VERIFY SIZE, WEIGHT AND LOCATION OF ALL MECHANICAL UNITS AND OPENINGS PRIOR TO WOOD JOIST FABRICATION. MECH. CONTRACTOR SHALL VERIFY MAGNITUDE AND LOCATION OF POINT LOADS PRIOR TO TRUSS FABRICATION. ALL INFORMATION REGARDING MECHANICAL UNITS ARE BASED ON PROPOSED UNITS. PROVIDE DBL. 2x6 FRAME UNDER MECHANICAL UNIT CURBS BELOW ROOF DECK AND 2x6 FRAME AROUND ROOF OPENINGS - SEE 6/S501 - TYPICAL

open '

W18x40

2 S403

51/4x16 LVL - J4

24" DEEP PREFABRICATED WOOD FLOOR TRUSSES @

TRUSSES AS REQUIRED — GENERAL CONTRACTOR TO COORDINATE ALL SHAFT OPENING LOCATIONS AND

24" O.C. MAX. - PROVIDE HEADER AND GIRDER

DIMENSIONS - TYP.

(12)

13

(14)

\$402

NOTE:

ALL TEMPORARY TRUSS BRACING REQUIRED FOR ERECTION, AS PER THE GUIDELINES SET FORTH BY THE TRUSS PLATE INSTITUTE PUBLICATION "HIB-91" SHALL BE PERMANENTLY ATTACHED AND REMAIN IN PLACE TO SERVE AS PERMANENT TRUSS BRACING UNLESS NOTED OTHERWISE. PROVIDE TRUSS BOTTOM CHORD BRACING AS REQUIRED.

TRUSSES SHALL BE DESIGNED FOR ALL

5 S402

H1-J4

H1-J4

S403

51/4x11% LVL - J4 51/4x11% LVL - J4

(9)

HORIZONTAL JOINT REINFORCING @ 16" MAX. ALL

MASONRY WALLS - TYPICAL UNLESS NOTED

4 S401

27K W16x26 27K

AND PROVIDE ADDITIONAL TRUSSES IF REQ'D.



6800 S Creek Rd, Charlotte, NC 28277 Ph:(704) 625-6554 Fax:(704) 919-5822 EMAIL:ashish@mishraarch.com WEB: www.mishraarch.com

ADDITIONAL LOADS SHOWN ON FRAMING PLANS CIVIL James W Braswell and Associates 1005 North 4th Street Monroe, LA 71201 Phone:

Email:braswellassociates@yahoo.com

Fax:

STRUCTURAL: WGPM, Inc. 11220 Elm Lane, Suite 201 Charlotte, NC 28277 Phone: (704) 542-7199

Fax: (704) 542-7195 Email: lwright@wgpminc.com **ELECTRICAL**: Ritter Engineering 1043 Arroyo Vista Lane

Matthew, NC 28014 Phone: (704) 516-0385 Fax: (704) 644-1464 Email: shane@rittereng.com

MECHANICAL:
Shultz Engineering Group, P.C. 212 North McDowell St., Suite 204 Charlotte, NC 28204 Phone: (704) 334-7363 Fax:(704) 347-0093 Email: don@shultzeg.com

REVISIONS No. Date Description 1 11.01.13 Pool Equip., Ftgs & Rf Slope

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MISHRA ARCHITECTURE PLLC

KEY PLAN

Southern Hospitality

Hampton Inn and

NOTE! CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS

5400 I-20 & Frontage Rd. Monroe, LA 71201

Services

Suites

Second Floor

#5 @ 24" VERTICAL ALL 8" CMU WALLS - SEE SHEET S302 - PROVIDE W1.7 (9 GA.) TRUSS TYPE

Construction Documents

MARCUS C. GIBSON License No.32774 _

Prepared by Checked by

12-111 AB/LW HLW ate September 16, 2013



WGPM, Inc. Wright + Gibson + Patton STRUCTURAL ENGINEERING 11220 Elm Lane, Suite 201 Charlotte, North Carolina 28277 704-542-7199 Fax: 704-542-7195 www.wgpminc.com JOB NUMBER: 57-13

OTHERWISE

PROVIDE LOOSE L6x4x5/16 (LLV) BRICK SHELF OVER MASONRY OPENINGS - PROVIDE 8" MINIMUM BEARING OVER SOLID MASONRY EACH END - TYPICAL U.N.O.

SECOND FLOOR FRAMING PLAN

ALL ELEVATIONS REFERENCED FROM FINISH FLOOR ELEVATION 71.50' (0-0). TRUSS BEARING - T.B. (+9'-11 1/4") TYPICAL UNLESS NOTED OTHERWISE.

TOP OF STEEL - T.O.S. (+11'-9 3/4") TYPICAL UNLESS NOTED OTHERWISE CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS. SEE SHEET S001 AND S002 FOR GENERAL NOTES, WALL STUDS AND WALL SHEATHING NOTES. 6. LVL INDICATES MICRO=LAM LVL BY I-LEVEL OR EQUIVALENT.

BB INDICATES 8" DEEP BOND BEAM - SEE 5/S302. 8. SW1 INDICATES SHEAR WALL 1 - SEE SHEAR WALL SCHEDULE ON SHEET SO02 TYPICAL.

NOTE!

FRAME ALL BEAMS OVER STEEL COLUMNS AS SHOWN AND PROVIDE 5/16" WEB STIFF P. - SEE 4/S404 - TYP. U.N.O.

NOTE!

BB INDICATES 8" DEEP CONTINUOUS BOND BEAM W/ 2 #4 CONTINUOUS GROUT SOLID - 16BB INDICATES 16" DEEP CONTINUOUS BOND BEAM W/ 2 #4 CONTNUOUS GROUT SOLID - SEE SECTIONS

NOTE!

H1 3 - 2x6 + PLYWOOD FILLER H2 3 - 2x8 + PLYWOOD FILLER H3 3 - 2x12 + PLYWOOD FILLER

S1 2 - 2x6 SILL

J1 2 - 2x6 JACK STUDS + 1 - 2x6 BYPASS STUDS J2 2 - 2x6 JACK STUDS + 2 - 2x6 BYPASS STUDS J3 2 - 2x6 JACK STUDS + 3 - 2x6 BYPASS STUDS J4 3 - 2x6 JACK STUDS + 1 - 2x6 BYPASS STUDS

NOTES:

1. ALL HEADERS ARE H1, ALL SILLS ARE S1 AND ALL JAMB STUDS ARE J1 UNLESS NOTED OTHERWISE.

2. PROVIDE DOUBLE JACK (CRIPPLE) STUDS UNDER EACH END OF ALL HEADERS AND DOUBLE BYPASS (KING) STUDS EACH SIDE OF JACK STUDS (U.N.O.) - SEE 8/S302.

NOTE!

ALL EXTERIOR WALLS ARE SHEAR WALLS SW2. SW1 INDICATES INTERIOR SHEAR WALL. SEE SHEET S002 FOR SHEAR WALL SCHEDULE.

NOTE!

SEE SHEET S002 FOR WALL STUDS AND PLYWOOD/OSB WALL SHEATHING - TYPICAL

H2-J2

W18x65

\$403 W16x36

J3 51/4x91/4 LVL J3

\$403

open

NOTE!

- 24" DEEP PREFABRICATED WOOD FLOOR TRUSSES

@ 24" O.C. MAX. - PROVIDE HEADER AND GIRDER

TRUSSES AS REQUIRED - GENERAL CONTRACTOR

TO COORDINATE ALL SHAFT OPENING LOCATIONS

AND DIMENSIONS - TYP.

W18x65

J3 5<mark>1/4x91/4 LVL</mark>

4

SEE 11/S403 FOR FOLDING PARTITION HANGER DETAIL - TYPICAL

PROVIDE #5 🛭 24" O.C. VERTICAL

AT ALL CMU STAIR AND ELEVATOR

WALLS - PROVIDE 8" DEEP CONT. BOND BEAM AT LANDING ELEVATIONS

TYP. U.N.O.

\$403

3.1(3)

NOTE:

23/32" TONGUE AND GROOVE PLYWOOD/OSB SUBFLOOR WITH

SPAN RATING OF 48/24 OR BETTER. ATTACH TO SUPPORTING

MEMBERS USING 10GA. x 2" LONG SCREWS @ 6" O.C. EDGES

AND @ 12" O.C. INTERMEDIATE — GLUE TO ALL SUPPORTING MEMBERS. LAYOUT STAGGERED AND PERPENDICULAR TO

SUPPORTING MEMBERS. SEE ARCH. FOR GYPCRETE TOPPING.

W12x30 (+15'-41/2")

-HSS 9x5x546 x CONT. AT TWO (2) STORY SPACE -

SEE 1/S403 - MITER CORNERS AND WELD ALL

AROUND - TYP. (4 REQ'D)

FIN. FL. (+12'-0") - T.O.PLYWOOD - 2ND FL.

MECHANICAL DUCTWORK RUNS THROUGH TRUSSES - COORDINATE DIAGONALS W/ MECH. CONTR.

NOTE:

NOTE:

-(0.7)

20" DEEP PREFABRICATED WOOD ROOF TRUSSES @ 24" 0.C. MAX. (TCDL = 8PSF, BCDL = 7PSF AND TCLL =

ROOF SHEATHING: 5/8" (19/32") EXTERIOR RATED PLYWOOD/OSB ROOF DECK - LAYOUT STAGGERED AND PERPENDICULAR TO ROOF TRUSSES — PROVIDE | w" gap around each sheet use simpson pscl SHEATHING CLIPS BETWEEN TRUSSES (24" MAX AND

OVER SUPPORTS) - ATTACH TO ROOF TRUSSES W

10d x 2 1/2" LONG NAILS AT 6" O.C. AROUND SUPPORTED EDGES AND 12" O.C. INTERMEDIATE

ALL TEMPORARY TRUSS BRACING REQUIRED FOR ERECTION, AS PER THE GUIDELINES SET FORTH BY THE TRUSS PLATE INSTITUTE PUBLICATION "HIB-91" SHALL BE PERMANENTLY ATTACHED AND REMAIN IN PLACE TO SERVE AS PERMANENT TRUSS BRACING UNLESS NOTED OTHERWISE. PROVIDE TRUSS BOTTOM CHORD BRACING AS REQUIRED.

TRUSSES SHALL BE DESIGNED FOR ALL

AND PROVIDE ADDITIONAL TRUSSES IF REQ'D.



6800 S Creek Rd, Charlotte, NC 28277 Ph:(704) 625-6554 Fax:(704) 919-5822 EMAIL:ashish@mishraarch.com

WEB: www.mishraarch.com ADDITIONAL LOADS SHOWN ON FRAMING PLANS | CIVIL: James W Braswell and Associates

1005 North 4th Street Monroe, LA 71201 Phone: Fax: Email:braswellassociates@yahoo.com

STRUCTURAL: WGPM, Inc.

11220 Elm Lane, Suite 201 Charlotte, NC 28277 Phone: (704) 542-7199 Fax: (704) 542-7195 Email: lwright@wgpminc.com

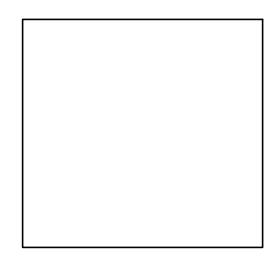
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Email: shane@rittereng.com MECHANICAL:
Shultz Engineering Group, P.C. 212 North McDowell St., Suite 204 Charlotte, NC 28204 Phone: (704) 334-7363 Fax:(704) 347-0093

Email: don@shultzeg.com

REVISIONS						
No.	Date	Description				
1	11.01.13	Pool Equip.,				
		Ftgs & Rf Slope				
2	03.11.14	Canopy				

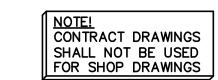
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KEY PLAN

Southern Hospitality Services

Hampton Inn and Suites



5400 I-20 & Frontage Rd. Monroe, LA 71201

Second Floor #5 @ 24" VERTICAL ALL 8" CMU WALLS — SEE Framing Plan

HORIZONTAL JOINT REINFORCING @ 16" MAX. ALL

Construction Documents

Prepared by Checked by

12-111 AB/LW HLW ate September 16, 2013

WGPM, Inc. Wright + Gibson + Patton STRUCTURAL ENGINEERING MARCUS C. GIBSON License No.32774 11220 Elm Lane, Suite 201 Charlotte, North Carolina 28277 704-542-7199 Fax: 704-542-7195 www.wgpminc.com JOB NUMBER: 57-13

OTHERWISE

MASONRY WALLS - TYPICAL UNLESS NOTED

PROVIDE LOOSE L6x4x5/16 (LLV) BRICK SHELF OVER MASONRY OPENINGS - PROVIDE 8" MINIMUM BEARING OVER SOLID MASONRY

SECOND FLOOR FRAMING PLAN

ALL ELEVATIONS REFERENCED FROM FINISH FLOOR ELEVATION 71.50' (0-0). TRUSS BEARING - T.B. (+9'-11 1/4") TYPICAL UNLESS NOTED OTHERWISE. TOP OF STEEL - T.O.S. (+11'-9 3/4'') TYPICAL UNLESS NOTED OTHERWISE

CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS. SEE SHEET S001 AND S002 FOR GENERAL NOTES, WALL STUDS AND WALL SHEATHING NOTES. 6. LVL INDICATES MICRO=LAM LVL BY I-LEVEL OR EQUIVALENT.

BB INDICATES 8" DEEP BOND BEAM - SEE 5/S302. 8. SW1 INDICATES SHEAR WALL 1 - SEE SHEAR WALL SCHEDULE ON SHEET S002 TYPICAL.

NOTE!

NOTE!

BB INDICATES 8" DEEP CONTINUOUS BOND BEAM W/ 2 #4 CONTINUOUS GROUT SOLID - 16BB INDICATES 16" DEEP CONTINUOUS BOND BEAM W/ 2 #4 CONTNUOUS GROUT SOLID - SEE SECTIONS

NOTE!

EACH END - TYPICAL U.N.O.

FRAME ALL BEAMS OVER STEEL COLUMNS

- SEE 4/S404 - TYP. U.N.O.

AS SHOWN AND PROVIDE 5/16" WEB STIFF P.

H1 3 - 2x6 + PLYWOOD FILLER H2 3 - 2x8 + PLYWOOD FILLER H3 3 - 2x12 + PLYWOOD FILLER

S1 2 - 2x6 SILL

J1 2 - 2x6 JACK STUDS + 1 - 2x6 BYPASS STUDS J2 2 - 2x6 JACK STUDS + 2 - 2x6 BYPASS STUDS J3 2 - 2x6 JACK STUDS + 3 - 2x6 BYPASS STUDS J4 3 - 2x6 JACK STUDS + 1 - 2x6 BYPASS STUDS

1. ALL HEADERS ARE H1, ALL SILLS ARE S1 AND ALL JAMB STUDS ARE J1 UNLESS NOTED OTHERWISE.

2. PROVIDE DOUBLE JACK (CRIPPLE) STUDS UNDER EACH END OF ALL HEADERS AND DOUBLE BYPASS (KING) STUDS EACH SIDE OF JACK STUDS (U.N.O.) - SEE 8/S302.

PROVIDE #5 @ 24" O.C. VERTICAL — AT ALL CMU STAIR AND ELEVATOR

WALLS - PROVIDE 8" DEEP CONT.

- GROUT ALL CELLS SOLID

BOND BEAM AT LANDING ELEVATIONS

NOTE!

OPEN

23/32" TONGUE AND GROOVE PLYWOOD/OSB SUBFLOOR WITH SPAN RATING OF 48/24 OR BETTER. ATTACH TO SUPPORTING MEMBERS USING 10GA. x 2" LONG SCREWS @ 6" O.C. EDGES AND @ 12" O.C.

INTERMEDIATE - GLUE TO ALL SUPPORTING MEMBERS. LAYOUT

ARCH. FOR GYPCRETE TOPPING.

(21)

FIN. FL. (+22'-8") - T.O.PLYWOOD - 3RD FL.

FIN. FL. (+33'-4") - T.O.PLYWOOD - 4TH FL.

STAGGERED AND PERPENDICULAR TO SUPPORTING MEMBERS. SEE

10 S403

ALL EXTERIOR WALLS ARE SHEAR WALLS SW2. SW1 INDICATES INTERIOR SHEAR WALL. SEE SHEET S002 FOR SHEAR WALL SCHEDULE.

SEE SHEET S002 FOR WALL STUDS AND PLYWOOD/OSB WALL SHEATHING - TYPICAL

H1 - TYP. U.N.O. NOTE!

S402

INDICATES SIMPSON ANCHOR

ON SHEET S601 (22 REQ'D)

TIEDOWN SYSTEM - SEE DETAILS

NOTE!

ALL INTERIOR HEADERS ARE HI AND JAMBS ARE JI U.N.O. ALL EXTERIOR HEADERS ARE H2 AND JAMBS ARE J2 U.N.O.

4 \$402

S401

5

S402

MECHANICAL DUCTWORK RUNS THROUGH TRUSSES - COORDINATE DIAGONALS W/ MECH. CONTR.

NOTE:

\$402

J2 J2 | J2

24" DEEP PREFABRICATED WOOD FLOOR TRUSSES @

TRUSSES AS REQUIRED - GENERAL CONTRACTOR TO COORDINATE ALL SHAFT OPENING LOCATIONS AND

24" O.C. MAX. - PROVIDE HEADER AND GIRDER

DIMENSIONS - TYP.

(12)

13

(14)

J2 J2 J2 J2

ALL TEMPORARY TRUSS BRACING REQUIRED FOR ERECTION, AS PER THE GUIDELINES SET FORTH BY THE TRUSS PLATE INSTITUTE PUBLICATION "HIB-91" SHALL BE PERMANENTLY ATTACHED AND REMAIN IN PLACE TO SERVE AS PERMANENT TRUSS BRACING UNLESS NOTED OTHERWISE. PROVIDE TRUSS BOTTOM CHORD BRACING AS REQUIRED.

TRUSSES SHALL BE DESIGNED FOR ALL

5 \$402

(S401)

AND PROVIDE ADDITIONAL TRUSSES IF REQ'D.



6800 S Creek Rd, Charlotte, NC 28277 Ph:(704) 625-6554 Fax:(704) 919-5822 EMAIL:ashish@mishraarch.com WEB: www.mishraarch.com

ADDITIONAL LOADS SHOWN ON FRAMING PLANS CIVIL: James W Braswell and Associates 1005 North 4th Street Monroe, LA 71201 Phone:

Email:braswellassociates@yahoo.com

11220 Elm Lane, Suite 201

Charlotte, NC 28277 Phone: (704) 542-7199 Fax: (704) 542-7195 Email: lwright@wgpminc.com ELECTRICAL:
Ritter Engineering

1043 Arroyo Vista Lane Matthew, NC 28014 Phone: (704) 516-0385 Fax: (704) 644-1464 Email: shane@rittereng.com

MECHANICAL:
Shultz Engineering Group, P.C.
212 North McDowell St., Suite 204 Charlotte, NC 28204 Phone: (704) 334-7363 Fax:(704) 347-0093 Email: don@shultzeg.com

REVISIONS No. Date Description 1 11.01.13 Pool Equip., Ftgs & Rf Slope

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KEY PLAN Southern Hospitality

Hampton Inn and Suites

NOTE!
CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS

(9)

#5 @ 24" VERTICAL ALL 8" CMU WALLS - SEE

MASONRY WALLS - TYPICAL UNLESS NOTED

OTHERWISE

Wright + Gibson + Patton

STRUCTURAL ENGINEERING

11220 Elm Lane, Suite 201

Charlotte, North Carolina 28277 704-542-7199 Fax: 704-542-7195 www.wgpminc.com

JOB NUMBER: 57-13

5 \$402

H2 - TYP. U.N.O.

5400 I-20 & Frontage Rd. Monroe, LA 71201

Services

Third and Fourth

SHEET S302 - PROVIDE W1.7 (9 GA.) TRUSS TYPE
HOPIZONIAL JOINT PEINFORCING @ 16" MAY ALL
Plan

HORIZONTAL JOINT REINFORCING @ 16" MAX. ALL

Construction Documents

MARCUS C. GIBSON License No.32774

WGPM, Inc.

Prepared by Checked by

12-111 AB/LW HLW ate September 16, 2013

1. ALL ELEVATIONS REFERENCED FROM FINISH FLOOR ELEVATION 71.50' (0-0). 2. TRUSS BEARING - T.B. (+20'-7 1/4") THIRD FLOOR AND (+31'-3 1/4") FOURTH FLOOR - TYPICAL UNLESS NOTED OTHERWISE. 4. CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS. SEE SHEET S001 AND S002 FOR GENERAL NOTES, WALL STUDS AND WALL SHEATHING NOTES.

6. LVL INDICATES MICRO=LAM LVL BY I-LEVEL OR EQUIVALENT. BB INDICATES 8" DEEP BOND BEAM - SEE 5/S302. 8. SW1 INDICATES SHEAR WALL 1 - SEE SHEAR WALL SCHEDULE ON SHEET SO02 TYPICAL.

THIRD AND FOURTH FLOOR FRAMING PLAN

2 S404

S402

15)

J1 - TYP. U.N.O.

W12x22 W/ HSS 6x4x5/16

ELEVATOR GUIDE RAIL -SEE 6/S403 FOR BEAM

BEARING DETAIL - TYP.

6 S402

H1 - TYP. U.N.O.

J2 – TYP. U.N.O.

5 \$403

8 \$401

H2 - TYP. U.N.O.

NOTE!

BB INDICATES 8" DEEP CONTINUOUS BOND BEAM W/ 2 #4 CONTINUOUS GROUT SOLID - 16BB INDICATES 16" DEEP CONTINUOUS BOND BEAM W/ 2 #4 CONTNUOUS GROUT SOLID - SEE SECTIONS

H1 3 - 2x6 + PLYWOOD FILLER H2 3 - 2x8 + PLYWOOD FILLER H3 3 - 2x12 + PLYWOOD FILLER

S1 2 - 2x6 SILL

J1 2 - 2x6 JACK STUDS + 1 - 2x6 BYPASS STUDS J2 2 - 2x6 JACK STUDS + 2 - 2x6 BYPASS STUDS J3 2 - 2x6 JACK STUDS + 3 - 2x6 BYPASS STUDS J4 3 - 2x6 JACK STUDS + 1 - 2x6 BYPASS STUDS

1. ALL HEADERS ARE H1, ALL SILLS ARE S1 AND ALL JAMB STUDS ARE J1 UNLESS NOTED OTHERWISE.

2. PROVIDE DOUBLE JACK (CRIPPLE) STUDS UNDER EACH END OF ALL HEADERS AND DOUBLE BYPASS (KING) STUDS EACH SIDE OF JACK STUDS (U.N.O.) - SEE 8/S302.

NOTE!

ALL EXTERIOR WALLS ARE SHEAR WALLS SW2. SW1 INDICATES INTERIOR SHEAR WALL. SEE SHEET S002 FOR SHEAR WALL SCHEDULE.

SEE SHEET S002 FOR WALL STUDS AND PLYWOOD/OSB WALL SHEATHING - TYPICAL

S402

NOTE!

ALL INTERIOR HEADERS ARE HI AND JAMBS ARE JI U.N.O. ALL EXTERIOR HEADERS ARE H2 AND JAMBS ARE J2 U.N.O.

NOTE:

MECHANICAL DUCTWORK RUNS THROUGH TRUSSES - COORDINATE DIAGONALS W/ MECH. CONTR.

NOTE:

ALL TEMPORARY TRUSS BRACING REQUIRED FOR ERECTION, AS PER THE GUIDELINES SET FORTH BY THE TRUSS PLATE INSTITUTE PUBLICATION "HIB-91" SHALL BE PERMANENTLY ATTACHED AND REMAIN IN PLACE TO SERVE AS PERMANENT TRUSS BRACING UNLESS NOTED OTHERWISE. PROVIDE TRUSS BOTTOM CHORD BRACING AS REQUIRED.



NOTE:

TRUSSES SHALL BE DESIGNED FOR ALL ADDITIONAL LOADS SHOWN ON FRAMING PLANS CIVIL: AND PROVIDE ADDITIONAL TRUSSES IF REQ'D.

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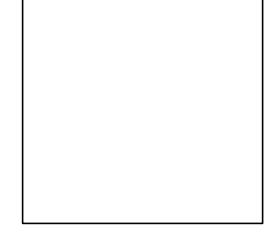
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MECHANICAL:
Shultz Engineering Group, P.C. 212 North McDowell St., Suite 204 Charlotte, NC 28204 Phone: (704) 334-7363 Fax:(704) 347-0093 Email: don@shultzeg.com

	REVISIONS					
No.	Date	Description				
1	11.01.13	Pool Equip.,				
		Ftgs & Rf Slope				

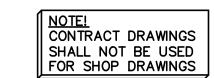
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KEY PLAN

Southern Hospitality Services

Hampton Inn and Suites



MASONRY WALLS - TYPICAL UNLESS NOTED

OTHERWISE

WGPM, Inc.

Wright • Gibson • Patton

STRUCTURAL ENGINEERING

11220 Elm Lane, Suite 201

Charlotte, North Carolina 28277 704-542-7199 Fax: 704-542-7195 www.wgpminc.com

JOB NUMBER: 57-13

5400 I-20 & Frontage Rd. SHALL NOT BE USED | Monroe, LA 71201

Third and Fourth

#5 @ 24" VERTICAL ALL 8" CMU WALLS — SEE Floor Framing Plan HORIZONTAL JOINT REINFORCING @ 16" MAX. ALL

Construction Documents

MARCUS C. GIBSON License No.32774

12-111 Prepared by AB/LW Checked by HLW ^{)ate}September 16, 2013

THIRD AND FOURTH FLOOR FRAMING PLAN

1. ALL ELEVATIONS REFERENCED FROM FINISH FLOOR ELEVATION 71.50' (0-0). 2. TRUSS BEARING - T.B. (+20'-7 1/4") THIRD FLOOR AND (+31'-3 1/4") FOURTH FLOOR - TYPICAL UNLESS NOTED OTHERWISE.

OPEN

J1 - TYP. U.N.O.

-24" DEEP PREFABRICATED WOOD FLOOR TRUSSES @

24" O.C. MAX. - PROVIDE HEADER AND GIRDER TRUSSES AS REQUIRED — GENERAL CONTRACTOR TO

COORDINATE ALL SHAFT OPENING LOCATIONS AND

DIMENSIONS - TYP.

H1 - TYP. U.N.O.

8 \$401

5 \$403

J2 - TYP. U.N.O.

4 \$402

PROVIDE #5 @ 24" O.C. VERTICAL AT ALL CMU STAIR AND ELEVATOR WALLS - PROVIDE 8" DEEP CONT. BOND BEAM AT LANDING ELEVATIONS

> 23/32" TONGUE AND GROOVE PLYWOOD/OSB SUBFLOOR WITH SPAN RATING OF 48/24 OR BETTER. ATTACH TO SUPPORTING MEMBERS USING 10GA. x 2" LONG SCREWS @ 6" O.C. EDGES AND @ 12" O.C.

INTERMEDIATE — GLUE TO ALL SUPPORTING MEMBERS. LAYOUT STAGGERED AND PERPENDICULAR TO SUPPORTING MEMBERS. SEE

ARCH. FOR GYPCRETE TOPPING.

FIN. FL. (+22'-8") - T.O.PLYWOOD - 3RD FL. FIN. FL. (+33'-4") - T.O.PLYWOOD - 4TH FL.

- GROUT ALL CELLS SOLID

4. CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS. SEE SHEET S001 AND S002 FOR GENERAL NOTES, WALL STUDS AND WALL SHEATHING NOTES.

6. LVL INDICATES MICRO=LAM LVL BY I-LEVEL OR EQUIVALENT. BB INDICATES 8" DEEP BOND BEAM - SEE 5/S302.

8. SW1 INDICATES SHEAR WALL 1 - SEE SHEAR WALL SCHEDULE ON SHEET S002 TYPICAL.

NOTE!

BB INDICATES 8" DEEP CONTINUOUS BOND BEAM W/ 2 #4 CONTINUOUS GROUT SOLID - 16BB INDICATES 16" DEEP CONTINUOUS BOND BEAM W/ 2 #4 CONTNUOUS GROUT SOLID - SEE SECTIONS

RH1 3 - 2x6 + PLYWOOD FILLER RH2 3 - 2x8 + PLYWOOD FILLER RH3 3 - 2x10 + PLYWOOD FILLER RH4 3 - 2x12 + PLYWOOD FILLER RH5 3 - 13/4"x91/4" LVL + PLYWOOD FILLER

RS1 1 - 2x6 SILL

RJ1 1 - 2x6 JACK STUD + 1 - 2x6 BYPASS STUD RJ2 1 - 2x6 JACK STUD + 2 - 2x6 BYPASS STUD RJ3 1 - 2x6 JACK STUD + 3 - 2x6 BYPASS STUD RJ4 2 - 2x6 JACK STUD + 1 - 2x6 BYPASS STUD

1. ALL HEADERS ARE RH1, ALL SILLS ARE RS1 AND ALL JAMB STUDS ARE RJ1 UNLESS NOTED OTHERWISE.

2. PROVIDE DOUBLE JACK (CRIPPLE) STUDS UNDER EACH END OF ALL HEADERS AND DOUBLE BYPASS (KING) STUDS EACH SIDE OF JACK STUDS (U.N.O.) - SEE 8/S302.

> PROVIDE #5 @ 24" O.C. VERTICAL -AT ALL CMU STAIR AND ELEVATOR

WALLS - PROVIDE 8" DEEP CONT.

- GROUT ALL CELLS SOLID

BOND BEAM AT LANDING ELEVATIONS

ROOF SHEATHING: 5/8" (19/32") EXTERIOR RATED

PLYWOOD/OSB ROOF DECK - LAYOUT STAGGERED

AND PERPENDICULAR TO ROOF TRUSSES -PROVIDE w" GAP AROUND EACH SHEET USE

SIMPSON PSCL SHEATHING CLIPS BETWEEN

TRUSSES (24" MAX AND OVER SUPPORTS) -

ATTACH TO ROOF TRUSSES W/ 10d x 2 1/2"

LONG NAILS AT 6" O.C. AROUND SUPPORTED

EDGES AND 12" O.C. INTERMEDIATE

NOTE!

NOTE!

PROVIDE DOUBLE 2x6 UNDER RTU

CURBS - SEE 6/S501 - TYPICAL

SOLID - SEE SECTIONS

BB INDICATES 8" DEEP CONTINUOUS BOND

BEAM W/ 2 #4 CONTINUOUS GROUT SOLID

- 16BB INDICATES 16" DEEP CONTINUOUS

BOND BEAM W/ 2 #4 CONTNUOUS GROUT

5 S501

NOTE!

NOTE!

NOTE!

OPEN

ALL EXTERIOR WALLS ARE SHEAR WALLS SW2. SW1 INDICATES INTERIOR SHEAR WALL. SEE SHEET S002 FOR SHEAR WALL SCHEDULE.

HEADERS ARE RH2 AND JAMBS ARE RJ2 U.N.O.

S501

SEE ARCHITECTURAL DRAWINGS FOR DRAFT STOP TRUSS LOCATIONS - DESIGN

ALL INTERIOR HEADERS ARE RH1 AND JAMBS ARE RJ1 U.N.O. ALL EXTERIOR

S501

TRUSS FOR ADDITIONAL GYPSUM DRYWALL DEAD LOAD - TYPICAL

SEE SHEET S002 FOR WALL STUDS AND PLYWOOD/OSB WALL SHEATHING - TYPICAL

RH1 - TYP. U,N.O. NOTE!

INDICATES SIMPSON ANCHOR TIEDOWN SYSTEM - SEE DETAILS ON SHEET S601 (22 REQ'D)

PREFABRICATE PITCHED FLAT WOOD -

ROOF TRUSSES @ 24" O.C. MAX. — PROVIDE ADDITIONAL TRUSSES IF

REQUIRED - TRUSS BRIDGING BY

SUPPLIER - 18" MAXIMUM DEPTH

AT CORRIDOR - TRUSSES

DESIGNED BY SUPPLIER

NOTE:

RJ2 - TYP. U.N.O.

RH2 - TYP. U.N.O.

3 S501

ATTACHMENT OF ALL MECHANICAL UNITS TO SUPPORTING STRUCTURE SHALL
BE DESIGNED AND FURNISHED BY
MECH. UNIT SUPPLIER TO RESIST LOCAL
SEISMIC AND WIND LOADS.

NOTE:

MECHANICAL CONTR. SHALL VERIFY SIZE, WEIGHT AND LOCATION OF ALL MECHANICAL UNITS AND OPENINGS PRIOR TO WOOD JOIST FABRICATION. MECH. CONTRACTOR SHALL VERIFY MAGNITUDE AND LOCATION OF POINT LOADS PRIOR TO TRUSS FABRICATION. ALL INFORMATION REGARDING MECHANICAL UNITS ARE BASED ON PROPOSED UNITS. PROVIDE DBL. 2x6 FRAME UNDER MECHANICAL UNIT CURBS BELOW ROOF DECK AND 2x6 FRAME AROUND ROOF OPENINGS - SEE 6/S501 - TYPICAL

open

RH1 - TYP. U.N.O.

\$501

(14)

(15)

13

(12)

NOTE:

TRUSSES SHALL BE DESIGNED FOR ALL

ADDITIONAL LOADS SHOWN ON FRAMING PLANS

S504

3- --

RH4 RH4

6 S502

AND PROVIDE ADDITIONAL TRUSSES IF REQ'D.

NOTE:

ALL TEMPORARY TRUSS BRACING REQUIRED FOR ERECTION, AS PER THE GUIDELINES SET FORTH BY THE TRUSS PLATE INSTITUTE PUBLICATION "HIB-91" SHALL BE PERMANENTLY ATTACHED AND REMAIN IN PLACE TO SERVE AS PERMANENT TRUSS BRACING UNLESS NOTED OTHERWISE. PROVIDE TRUSS BOTTOM CHORD BRACING AS REQUIRED.

MECHANICAL DUCTWORK RUNS

DIAGONALS W/ MECH. CONTR.

▭

- 3000# R U - PROVIDE DBL. 2x6

under RTU — provide RH4 as

ea. end of RH4

RH2 - TYP. U.N.O.

(9)

OTHERWISE

Wright + Gibson + Patton

STRUCTURAL ENGINEERING

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Charlotte, North Carolina 28277 704-542-7199 Fax: 704-542-7195 www.wgpminc.com

JOB NUMBER: 57-13



6800 S Creek Rd, Charlotte, NC 28277 Ph:(704) 625-6554 Fax:(704) 919-5822 EMAIL:ashish@mishraarch.com WEB: www.mishraarch.com

THROUGH TRUSSES — COORDINATE

DIACONALS W/ MECH CONTR

James W Braswell and Associates 1005 North 4th Street Monroe, LA 71201 Phone:

Fax: Email:braswellassociates@yahoo.com

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Email: lwright@wgpminc.com ELECTRICAL: Ritter Engineering 1043 Arroyo Vista Lane Matthew, NC 28014 Phone: (704) 516-0385

Fax: (704) 644-1464 Email: shane@rittereng.com MECHANICAL:
Shultz Engineering Group, P.C. 212 North McDowell St., Suite 204 Charlotte, NC 28204 Phone: (704) 334-7363

Fax:(704) 347-0093 Email: don@shultzeg.com REVISIONS No. Date Description 1 11.01.13 Pool Equip.,

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Ftgs & Rf Slope

KEY PLAN

Hampton Inn and Suites

Southern Hospitality

NOTE! CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS

5400 I-20 & Frontage Rd. Monroe, LA 71201

Services

#5 @ 24" VERTICAL ALL 8" CMU WALLS - SEE

SHEET S302 - PROVIDE W1.7 (9 GA.) TRUSS TYPE

HORIZONTAL JOINT REINFORCING @ 16" MAX. ALL

MASONRY WALLS - TYPICAL UNLESS NOTED Construction Documents

MARCUS C. GIBSON License No.32774

WGPM, Inc.

12-111 Prepared by AB/LW Checked by HLW ate September 16, 2013

4 S501



ALL ELEVATIONS REFERENCED FROM FINISH FLOOR ELEVATION 71.50' (0-0). TRUSS BEARING - T.B. (+43'-4") - TYPICAL UNLESS NOTED OTHERWISE.

CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS.

SEE SHEET SOO1 AND SOO2 FOR GENERAL NOTES, LOAD BEARING STUDS AND WALL SHEATHING NOTES.

LVL INDICATES LVL MICRO=LAM BY I-LEVEL OR EQUIVALENT.

SW1 INDICATES SHEAR WALL 1 - SEE SHEAR WALL SCHEDULE ON SHEET SOO2 TYPICAL.

NOTE! DESIGN ALL TRUSSES FOR ALL ADDITIONAL LOADS SHOWN ON CONTRACT DRAWINGS - TYPICAL

NOTE!
SEE SECTIONS FOR ADDITIONAL 2x BLOCKING—

REQUIRED AT HIGH PARAPET WALLS -

(21)

RH1 3 - 2x6 + PLYWOOD FILLER RH2 3 - 2x8 + PLYWOOD FILLER RH3 3 - 2x10 + PLYWOOD FILLER RH4 3 - 2x12 + PLYWOOD FILLER RH5 3 - 13/4"x91/4" LVL + PLYWOOD FILLER

RS1 1 - 2x6 SILL

RJ1 1 - 2x6 JACK STUD + 1 - 2x6 BYPASS STUD RJ2 1 - 2x6 JACK STUD + 2 - 2x6 BYPASS STUD RJ3 1 - 2x6 JACK STUD + 3 - 2x6 BYPASS STUD RJ4 2 - 2x6 JACK STUD + 1 - 2x6 BYPASS STUD

1. ALL HEADERS ARE RH1, ALL SILLS ARE RS1 AND ALL JAMB STUDS ARE RJ1 UNLESS NOTED OTHERWISE.

2. PROVIDE DOUBLE JACK (CRIPPLE) STUDS UNDER EACH END OF ALL HEADERS AND DOUBLE BYPASS (KING) STUDS EACH SIDE OF JACK STUDS (U.N.O.) - SEE 8/S302.

NOTE!

NOTE!

NOTE!

ALL EXTERIOR WALLS ARE SHEAR WALLS SW2. SW1 INDICATES INTERIOR SHEAR WALL. SEE SHEET S002 FOR SHEAR WALL SCHEDULE.

HEADERS ARE RH2 AND JAMBS ARE RJ2 U.N.O.

SEE ARCHITECTURAL DRAWINGS FOR DRAFT STOP TRUSS LOCATIONS - DESIGN

ALL INTERIOR HEADERS ARE RH1 AND JAMBS ARE RJ1 U.N.O. ALL EXTERIOR

TRUSS FOR ADDITIONAL GYPSUM DRYWALL DEAD LOAD - TYPICAL

SEE SHEET S002 FOR WALL STUDS AND PLYWOOD/OSB WALL SHEATHING - TYPICAL

-RJ2 -

1DE DEL. 2x6 @ 16 0.C. /IDE RH4 AS SHOWN W/ RJ4

╼═┿╼══

RJ2 - TYP. U.N.O.

RH2 - TYP. U.N.O.

RH1 - TYP. U.N.O.

RJ3 RH4 RJ3

1 S501

RJ3 RH4

S501

NOTE:

\$504

ATTACHMENT OF ALL MECHANICAL UNITS TO SUPPORTING STRUCTURE SHALL BE DESIGNED AND FURNISHED BY MECH. UNIT SUPPLIER TO RESIST LOCAL SEISMIC AND WIND LOADS.

NOTE:

- 2x8 @ 16" O.C. ROOF JOISTS - TYP.

PROVIDE #5 @ 24" O.C. VERTICAL AT ALL CMU STAIR AND ELEVATOR WALLS - PROVIDE 8" DEEP CONT. BOND BEAM AT LANDING ELEVATIONS - GROUT ALL CELLS SOLID

- SEE SECTIONS FOR ADDITIONAL 2x BLOCKING REQUIRED

-PREFABRICATE PITCHED FLAT WOOD ROOF TRUSSES @ 24" O.C. MAX.

- PROVIDE ADDITIONAL TRUSSES IF REQUIRED - TRUSS BRIDGING BY

SUPPLIER - 18" MAXIMUM DEPTH AT CORRIDOR - TRUSSES

ROOF SHEATHING: 5/8" (19/32") EXTERIOR RATED PLYWOOD/OSB ROOF DECK - LAYOUT STAGGERED AND PERPENDICULAR TO ROOF TRUSSES -PROVIDE w" GAP AROUND EACH SHEET USE SIMPSON PSCL SHEATHING CLIPS BETWEEN TRUSSES (24" MAX AND OVER SUPPORTS) -ATTACH TO ROOF TRUSSES W/ 10d x 2 1/2"

LONG NAILS AT 6" O.C. AROUND SUPPORTED

EDGES AND 12" O.C. INTERMEDIATE

AT HIGH PARAPET WALLS - TYPICAL

DESIGNED BY SUPPLIER

MECHANICAL CONTR. SHALL VERIFY SIZE, WEIGHT AND LOCATION OF ALL MECHANICAL UNITS AND OPENINGS PRIOR TO WOOD JOIST FABRICATION. MECH. CONTRACTOR SHALL VERIFY MAGNITUDE AND LOCATION OF POINT LOADS PRIOR TO TRUSS FABRICATION. ALL INFORMATION REGARDING MECHANICAL UNITS ARE BASED ON PROPOSED UNITS. PROVIDE DBL. 2x6 FRAME UNDER MECHANICAL UNIT CURBS BELOW ROOF DECK AND 2x6 FRAME AROUND ROOF OPENINGS - SEE 6/S501 - TYPICAL

NOTE:

TRUSSES SHALL BE DESIGNED FOR ALL

ADDITIONAL LOADS SHOWN ON FRAMING PLANS

AND PROVIDE ADDITIONAL TRUSSES IF REQ'D.

NOTE:

ALL TEMPORARY TRUSS BRACING REQUIRED FOR ERECTION, AS PER THE GUIDELINES SET FORTH BY THE TRUSS PLATE INSTITUTE PUBLICATION "HIB-91" SHALL BE PERMANENTLY ATTACHED AND REMAIN IN PLACE TO SERVE AS PERMANENT TRUSS BRACING UNLESS NOTED OTHERWISE. PROVIDE TRUSS BOTTOM CHORD BRACING AS REQUIRED.

MECHANICAL DUCTWORK RUNS

DIAGONALS W/ MECH. CONTR.

NOTE:



6800 S Creek Rd, Charlotte, NC 28277 Ph:(704) 625-6554 Fax:(704) 919-5822 EMAIL:ashish@mishraarch.com WEB: www.mishraarch.com

THROUGH TRUSSES — COORDINATE DIAGONALS W/ MECH CONTR James W Braswell and Associates 1005 North 4th Street Monroe, LA 71201 Phone:

Email:braswellassociates@yahoo.com

STRUCTURAL: WGPM, Inc.

Fax:

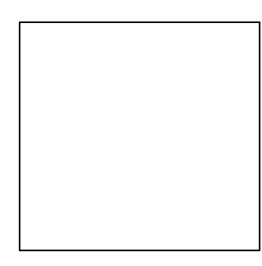
11220 Elm Lane, Suite 201 Charlotte, NC 28277 Phone: (704) 542-7199 Fax: (704) 542-7195 Email: lwright@wgpminc.com

ELECTRICAL:
Ritter Engineering 1043 Arroyo Vista Lane Matthew, NC 28014 Phone: (704) 516-0385 Fax: (704) 644-1464 Email: shane@rittereng.com

MECHANICAL:
Shultz Engineering Group, P.C. 212 North McDowell St., Suite 204 Charlotte, NC 28204 Phone: (704) 334-7363 Fax:(704) 347-0093 Email: don@shultzeg.com

	REVISIONS				
No.	Date	Description			
1	11.01.13	Pool Equip.,			
		Ftgs & Rf Slope			

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KEY PLAN

Southern Hospitality Services

Hampton Inn and Suites

FOR SHOP DRAWINGS

MASONRY WALLS - TYPICAL UNLESS NOTED

OTHERWISE

Wright + Gibson + Patton

STRUCTURAL ENGINEERING

11220 Elm Lane, Suite 201

Charlotte, North Carolina 28277 704-542-7199 Fax: 704-542-7195 www.wgpminc.com

JOB NUMBER: 57-13

NOTE! 5400 I-20 & Frontage Rd. SHALL NOT BE USED | Monroe, LA 71201

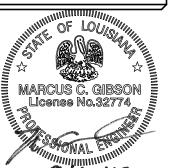
Roof Framing

#5 @ 24" VERTICAL ALL 8" CMU WALLS - SEE SHEET S302 - PROVIDE W1.7 (9 GA.) TRUSS TYPE

HORIZONTAL JOINT REINFORCING @ 16" MAX. ALL

Construction Documents





WGPM, Inc.

Prepared by Checked by

12-111 AB/LW HLW ate September 16, 2013

ROOF FRAMING PLAN

ALL ELEVATIONS REFERENCED FROM FINISH FLOOR ELEVATION 71.50' (0-0). TRUSS BEARING - T.B. (+43'-4") - TYPICAL UNLESS NOTED OTHERWISE.

CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS. SEE SHEET SOO1 AND SOO2 FOR GENERAL NOTES, LOAD BEARING STUDS AND WALL SHEATHING NOTES. LVL INDICATES LVL MICRO=LAM BY I-LEVEL OR EQUIVALENT. SW1 INDICATES SHEAR WALL 1 - SEE SHEAR WALL SCHEDULE ON SHEET SOO2 TYPICAL.

NOTE! PROVIDE DOUBLE 2x6 UNDER RTU CURBS - SEE 6/S501 - TYPICAL

NOTE! BB INDICATES 8" DEEP CONTINUOUS BOND BEAM W/ 2 #4 CONTINUOUS GROUT SOLID - 16BB INDICATES 16" DEEP CONTINUOUS BOND BEAM W/ 2 #4 CONTNUOUS GROUT SOLID - SEE SECTIONS

NOTE! DESIGN ALL TRUSSES FOR ALL ADDITIONAL LOADS SHOWN ON CONTRACT DRAWINGS - TYPICAL



MISHRA ARCHITECTURE PLLC

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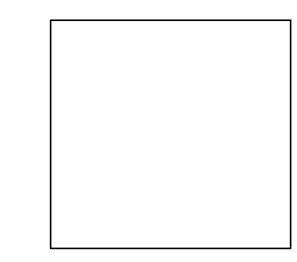
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	REVI	SIONS
No.	Date	Description
1	11.01.13	Pool Equip.,
		Ftgs & Rf Slope

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KEY PLAN

Southern Hospitality Services

Hampton Inn and Suites

NOTE!
CONTRACT DRAWINGS
SHALL NOT BE USED
FOR SHOP DRAWINGS

5400 I-20 & Frontage Rd.
Monroe, LA 71201

WGPM, Inc.
Wright · Gibson · Patton
STRUCTURAL ENGINEERING

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JOB NUMBER: 57-13

Construction Documents

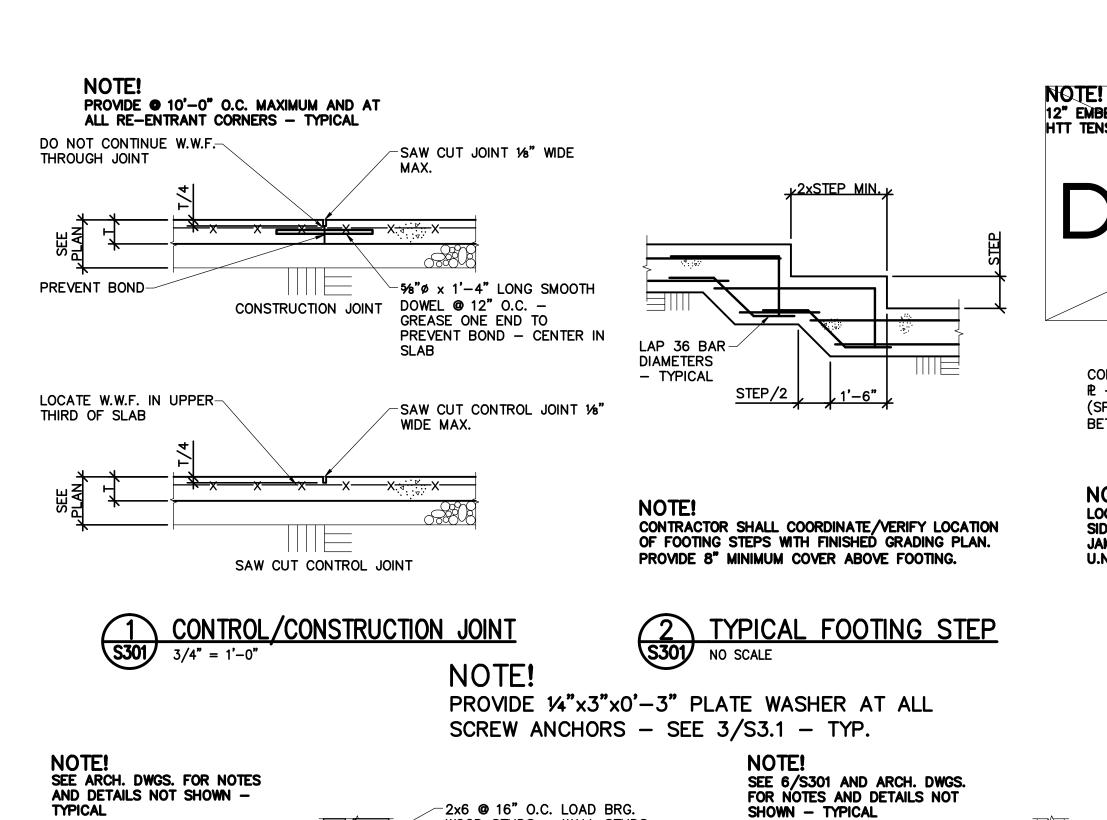
MARCUS C. GIBSON License No.32774

Prepared by Checked by

12-111 AB/LW HLW Date September 16, 2013

NOTE! BB INDICATES 8" DEEP CONTINUOUS BOND BEAM W/ 2 #4 CONTINUOUS GROUT SOLID - 16BB INDICATES 16" DEEP CONTINUOUS BOND BEAM W/ 2 #4 CONTNUOUS GROUT SOLID - SEE SECTIONS HIGH ROOF STAIR AND ELEVATOR FRAMING PLAN

ALL ELEVATIONS REFERENCED FROM FINISH FLOOR ELEVATION 71.50' (0-0).
 PLYWOOD ROOF DECK BEARING ELEVATION - SEE ARCHITECTURAL DRAWINGS.
 CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS.
 SEE SHEET S001 AND S002 FOR GENERAL NOTES.



2x6 @ 16" O.C. LOAD BRG.

SCHEDULE ON SHEET S002

-P.T. CONT. 2x6 BOTT. t R W/

5/8" Ø SCREW ANCHOR AND

SIMPSON H3 HURRICANE TIE

EVERY OTHER STUD - SEE

SLAB ON GRADE - SEE

3" CLR. BOTT. AND

TRANSVERSE REINF. — SEE PLAN AND SCHED. — TYP.

#4 DOWELS W/ STD HOOK @

2 #4 CONTINUOUS -

SEE CIVIL/SITE DWGS.-

LONGITUDINAL REINF. – SEE PLAN AND SCHED. – TYP.

- TYPICAL

TO ALIGN WITH STUDS ABOVE - SEE PLAN AND

TYP.

3/S301

PLAN - TYP.

EXTERIOR MASONRY VENEER-

- SEE ARCH. DWGS. - TYP.

- SEE PLAN - TYP.

#4 @ 18" O.C. DOWELS

48"

SEE CIVIL/SITE DWGS.-

4" CMU BELOW GRADE -- TYP.

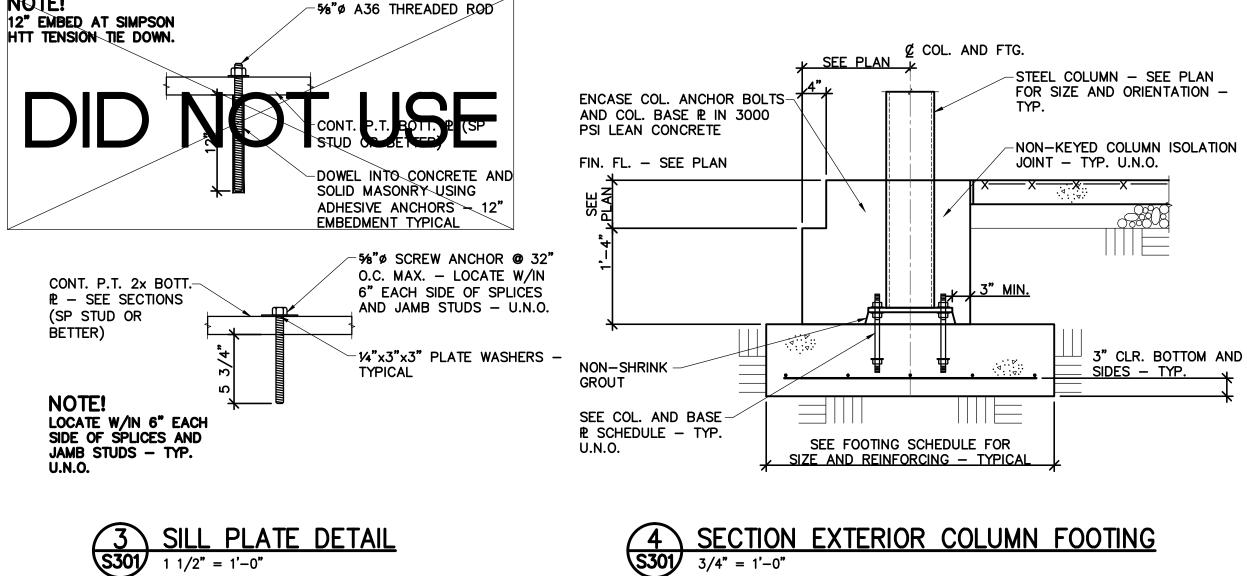
2 #4 CONTINUOUS

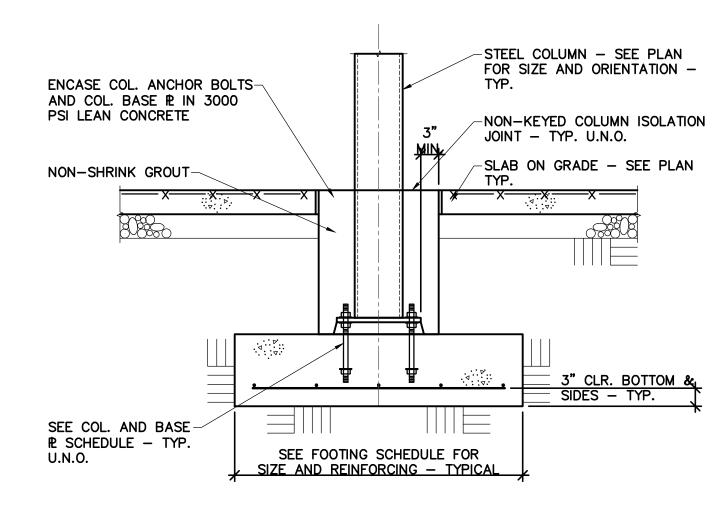
LONGITUDINAL REINF. – SEE PLAN AND SCHED. – TYP.

- TYPICAL

PLYWOOD/OSB WALL SHEATHING-

WOOD STUDS - WALL STUDS





SECTION INTERIOR COLUMN FOOTING

SEE ARCH.

- 1 #4 CONT. (4 SIDES)

-#4 @ 18" O.C. W/

12" HOOK @ EACH

#4 **@** 12" O.C.

– TYP.

#4 @ 18" DOWELS-

~ 24" ~

HOOK AT CORNERS

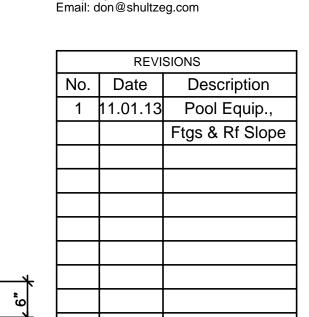
L11/2×11/2×1/8 × CONT. FRAME-

HEADED STUDS @ 24" O.C.

4 #4 CONT.

GALV. - W/ 3/8" Ø x 4"

LONG SHOP WELDED



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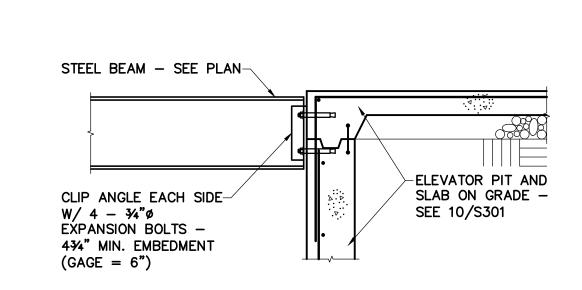
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1'-0"

 $^{-}$ 3 #4 CONT.



Southern Hospitality Services

KEY PLAN

Hampton Inn and

Suites

13 W12 AT ELEVATOR PIT \$301 3/4" = 1'-0"

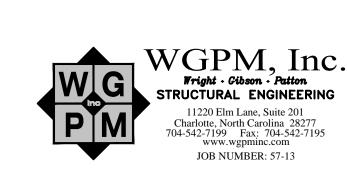
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Drawing Title Foundation Sections and Details

Construction Documents

12-111 Prepared by AB/LW Checked by HLW ^{ate}September 16, 2013

WGPM, Inc. Wright · Gibson · Patton
STRUCTURAL ENGINEERING 11220 Elm Lane, Suite 201 Charlotte, North Carolina 28277 704-542-7199 Fax: 704-542-7195 www.wgpminc.com JOB NUMBER: 57-13

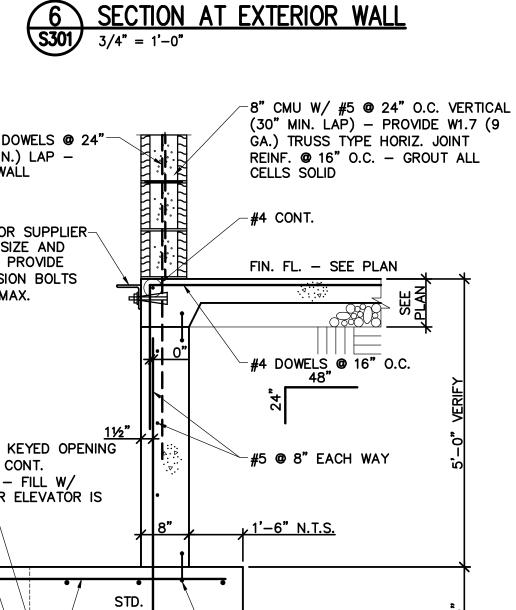


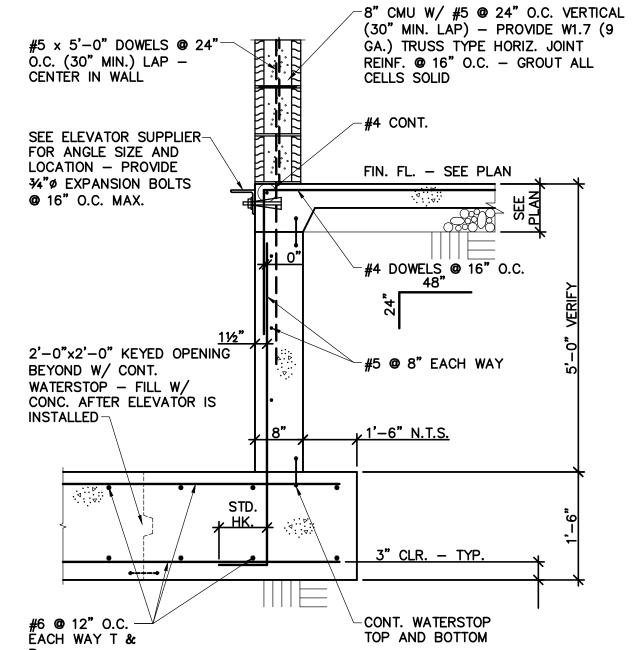












10 ELEVATOR PIT WALL

3/4" = 1'-0"

EQ. 11 1/4" EQ.



3 SILL PLATE DETAIL \$301 1 1/2" = 1'-0"

STOREFRONT/CURTAIN WALL

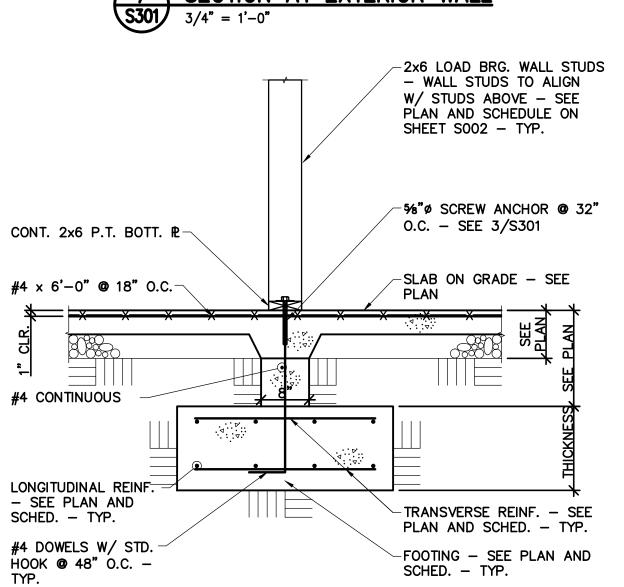
-#4 DOWELS @ 18" O.C.

TRANSVERSE REINF. — SEE PLAN AND SCHED. — TYP.

#4 DOWELS W/ STD HOOK @

48" O.C.

SYSTEM - SEE ARCH. DWGS.



NOTE! PROVIDE 1/4"x3"x0'-3" PLATE WASHER AT ALL SCREW ANCHORS - SEE 3/S301 - TYP.

THICKENED SLAB AT LOAD BEARING WALL 11 THICKENE \$301 3/4" = 1'-0"

SUMP PUMP PIT DETAIL

#4 DOWELS @ 12"

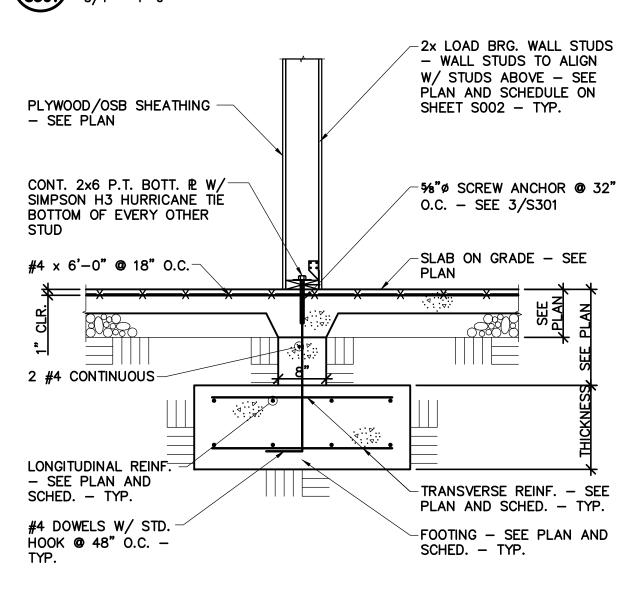
EACH DIRECTION

, 1'-6" SQ.

ELEVATOR PIT FLOOR

ELEV. SLAB REINF.

- SEE 10/S301



-1"x1/8" BAR GRATE (McNICHOLS GW

-L1x1x1⁄a x CONT. W/ 3⁄a"ø x 4" LONG HEADED STUDS (3 PER SIDE)

OR EQUIVALENT)

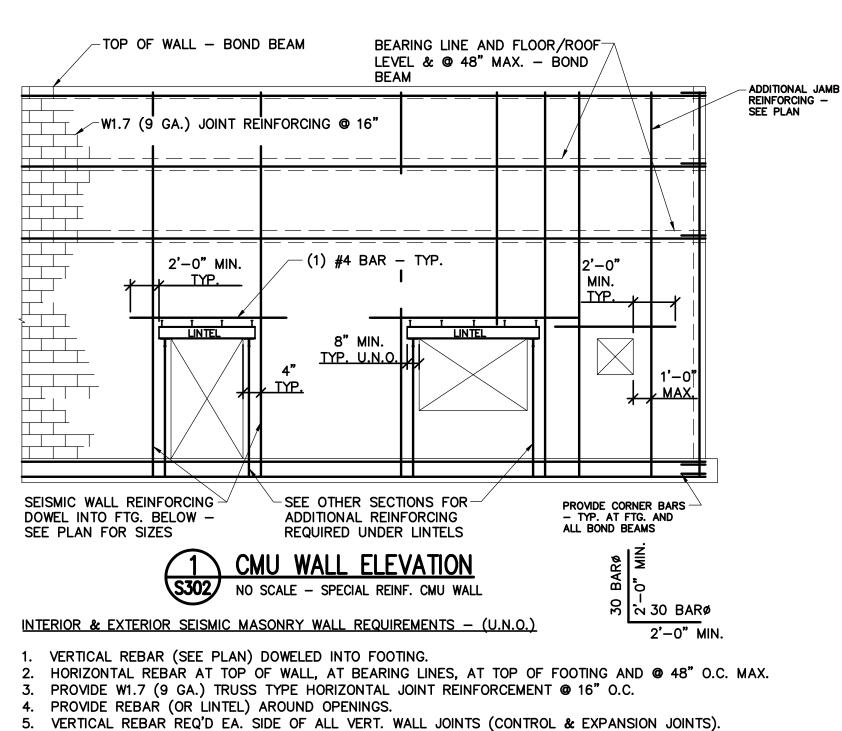
WATERSTOP

2'-2"

CONT.

NOTE! THIS DETAIL OCCURS ONLY AT SHEAR WALLS SW1 -SPACE SCREW ANCHORS @ 24" O.C. AT ALL SHEAR WALLS - SEE PLANS - TYPICAL

THICKENED SLAB AT SHEAR WALL 3/4" = 1'-0"



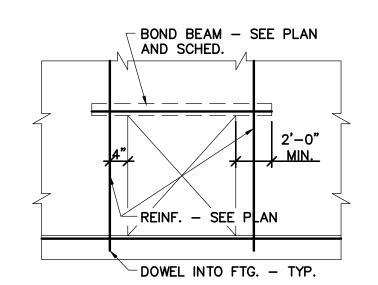
6. LAP VERTICAL REBAR 48 BARØ (MIN. LAP 24"). GROUT ALL CELLS SOLID WHERE REBAR OCCURS.

8. ALL VERTICAL REBAR TO BE POSITIONED IN BLOCK CELL BY PREFABRICATED WIRE POSITIONER - SUBMIT

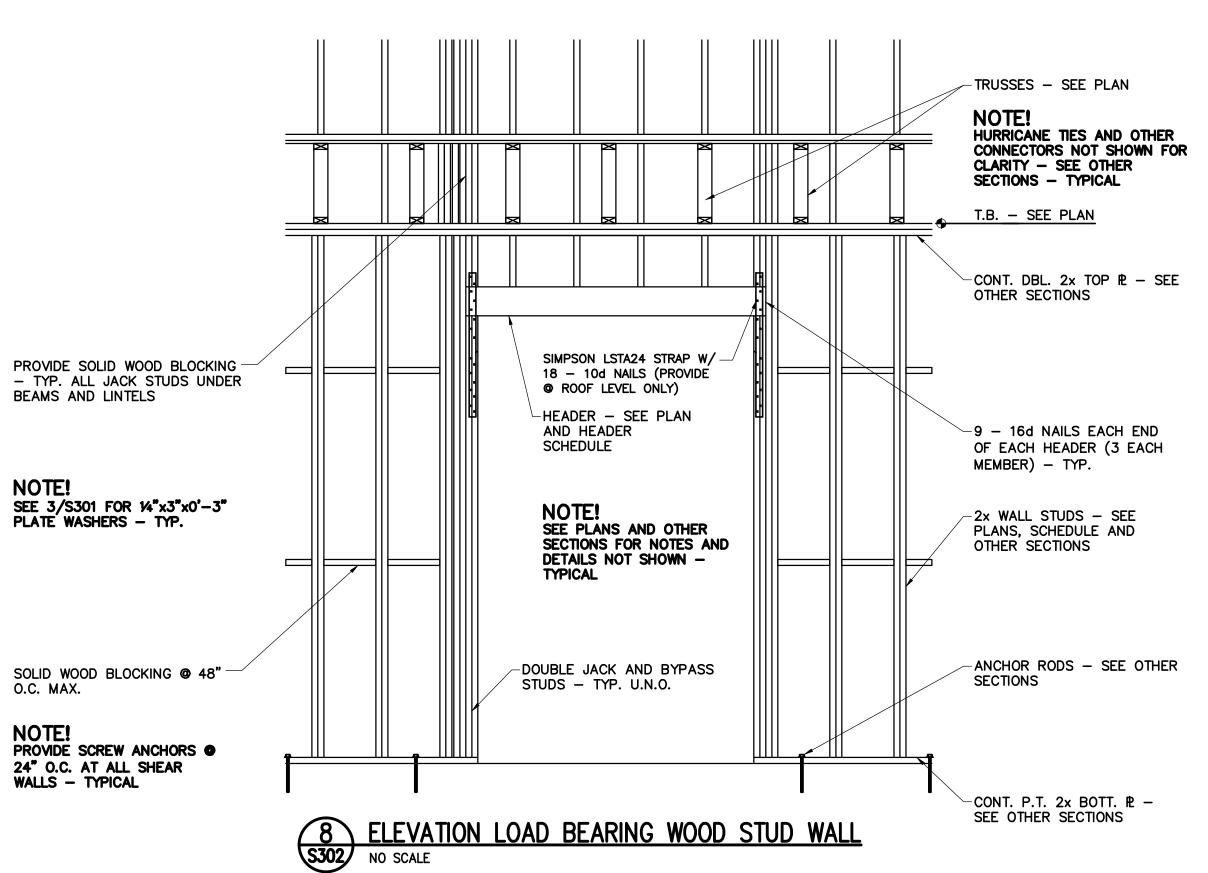
9. SEE BEAM OR LINTEL BEARING DETAILS FOR ADDITIONAL WALL REINFORCING REQUIRED UNDER LINTELS.

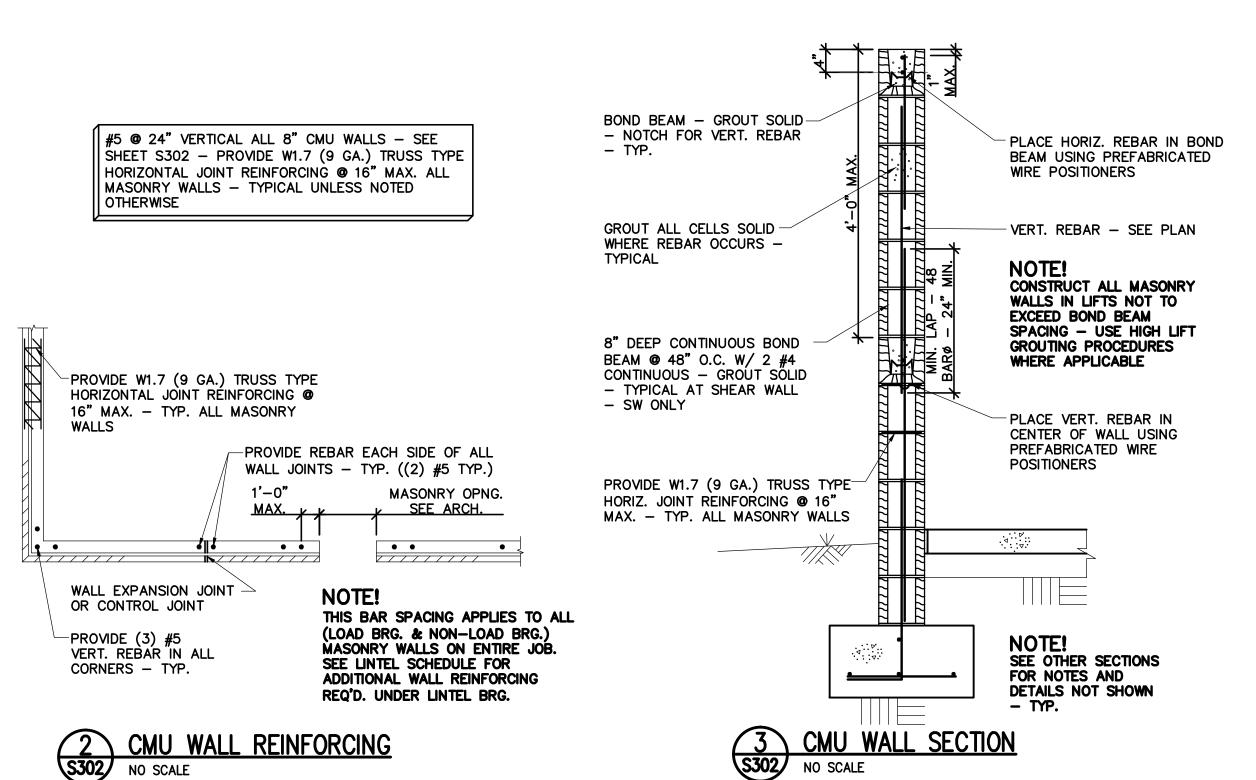
10. ALL BRICK TIES SHALL CONFORM WITH SEISMIC DESIGN CATEGORY REQUIREMENTS - SUBMIT FOR

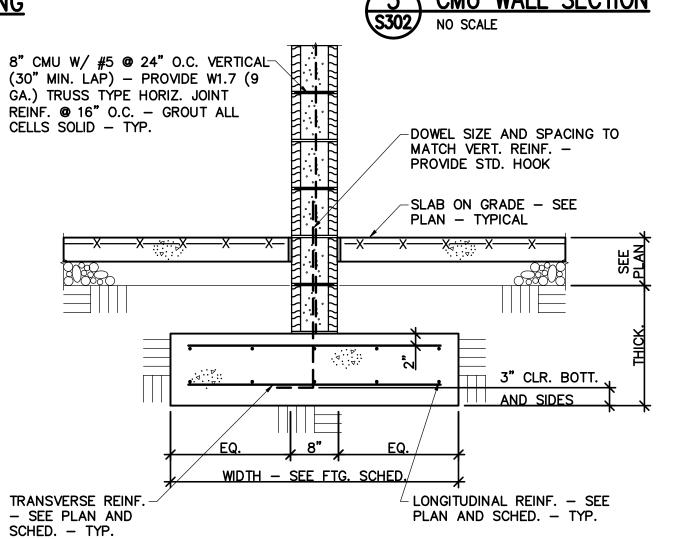
11. CONSTRUCT ALL WALLS IN 4'-0" LIFTS. 12. SEE OTHER SECTIONS FOR NOTES AND DETAILS NOT SHOWN.

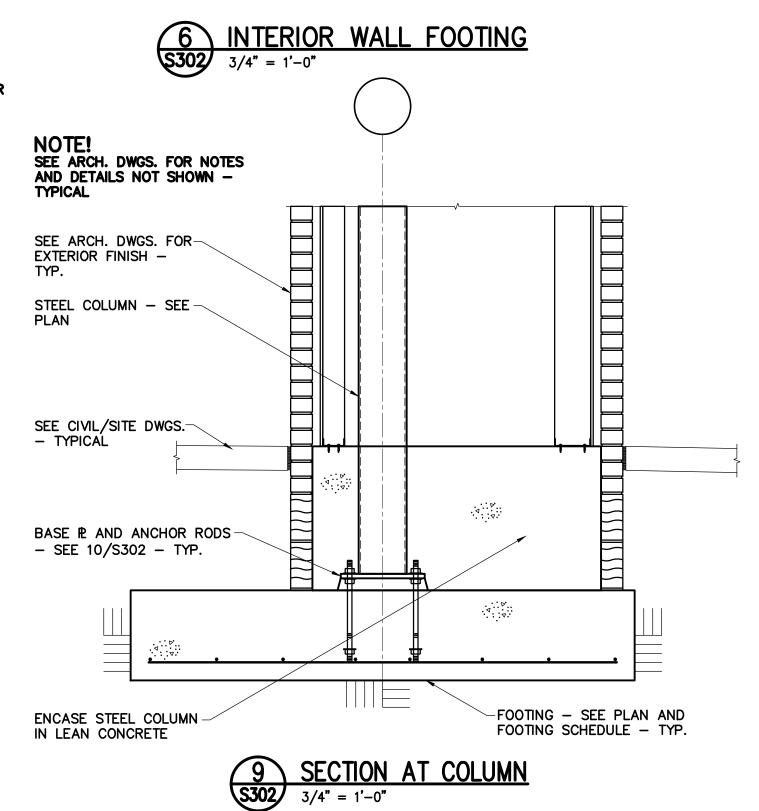




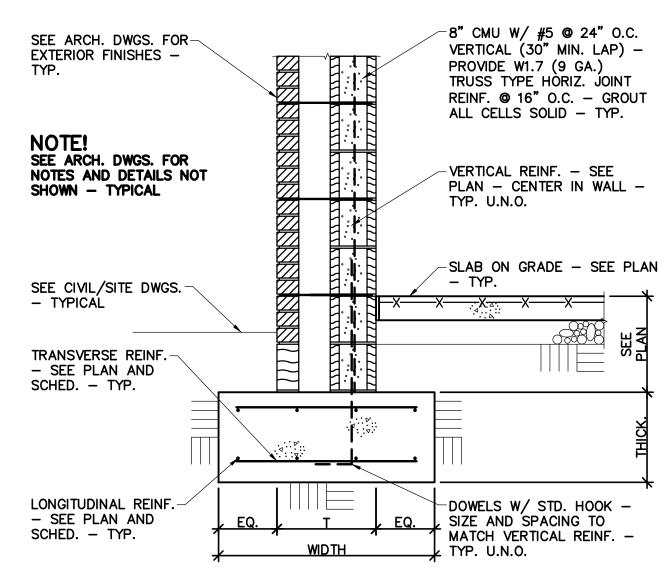




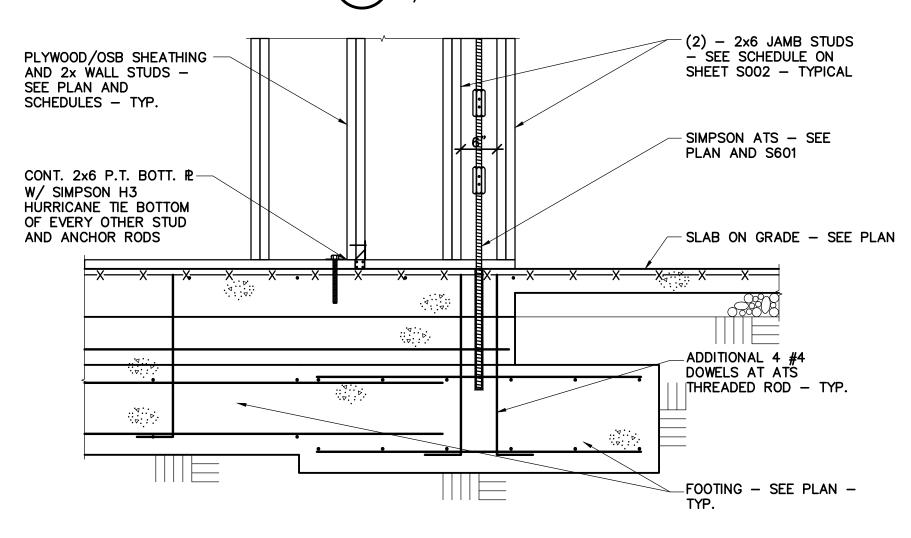




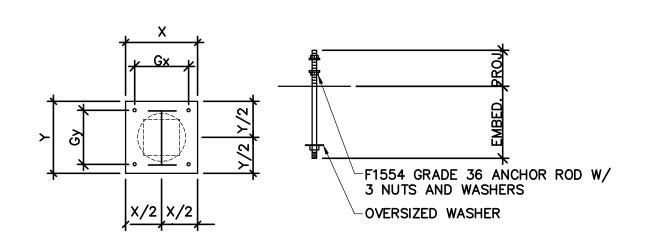




SECTION AT EXTERIOR WALL 3/4" = 1'-0"



7 ELEVATION AT ATS \$302 3/4" = 1'-0"



COLUMN	BASE	BASE PLATE		GAGE		ANCHOR ROD			GROUT
	THICK.	×	Υ	Gx	Gy	DIA.	PROJ.	EMBED.	
HSS 5x5	1"	12"	12"	9"	9"	3/4"	5"	9"	2"
HSS 8x8	11⁄4"	16"	16"	12"	12"	1"	6"	15"	2"

NOTE!

PROVIDE PJP GROOVE WELD (1/8" ROOT FACE) ALL AROUND FROM COLUMNS TO BASE R'S - TYPICAL







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Email:braswellassociates@yahoo.com STRUCTURAL: WGPM, Inc.

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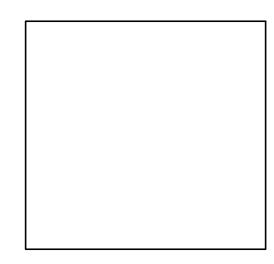
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REVISIONS					
No.	Date	Description			
1	11.01.13	Pool Equip.,			
		Ftgs & Rf Slope			

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KEY PLAN

Southern Hospitality Services

Hampton Inn and Suites

5400 I-20 & Frontage Rd. Monroe, LA 71201

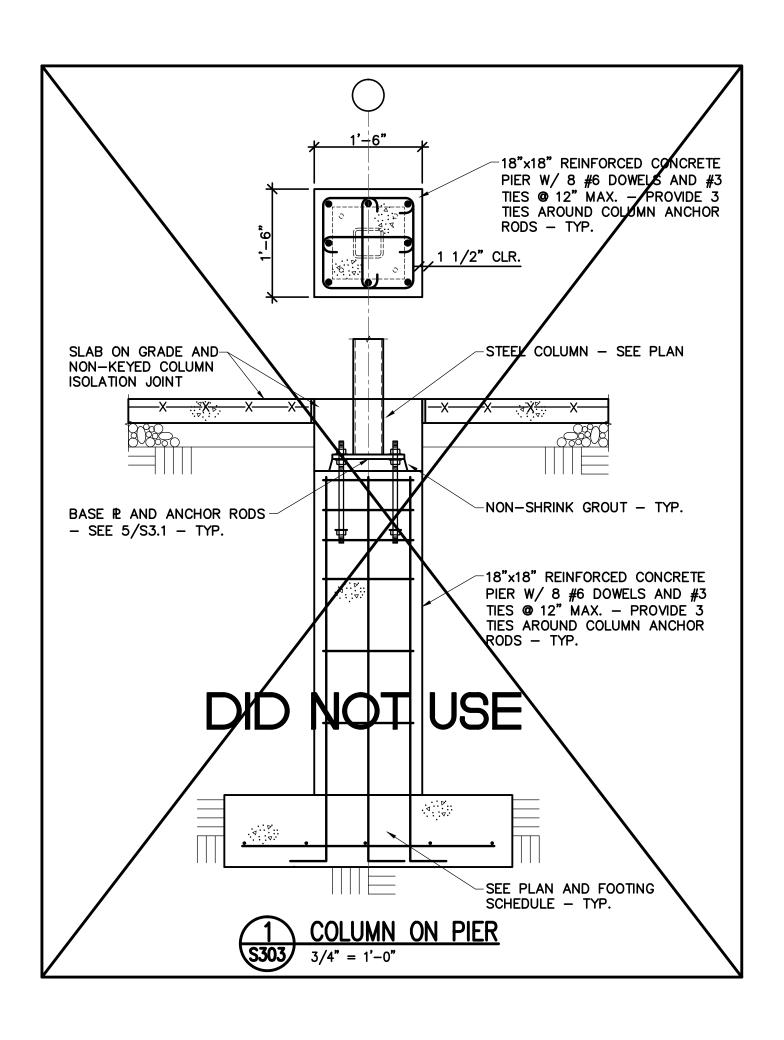
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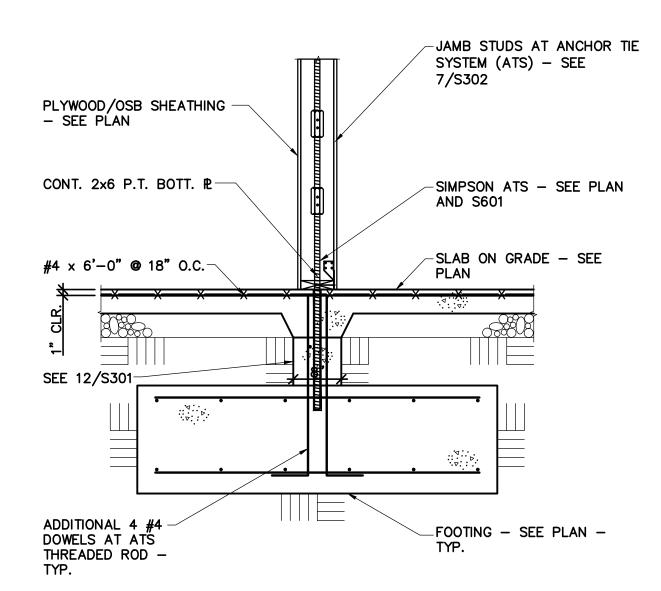
Construction Documents

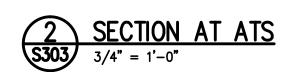
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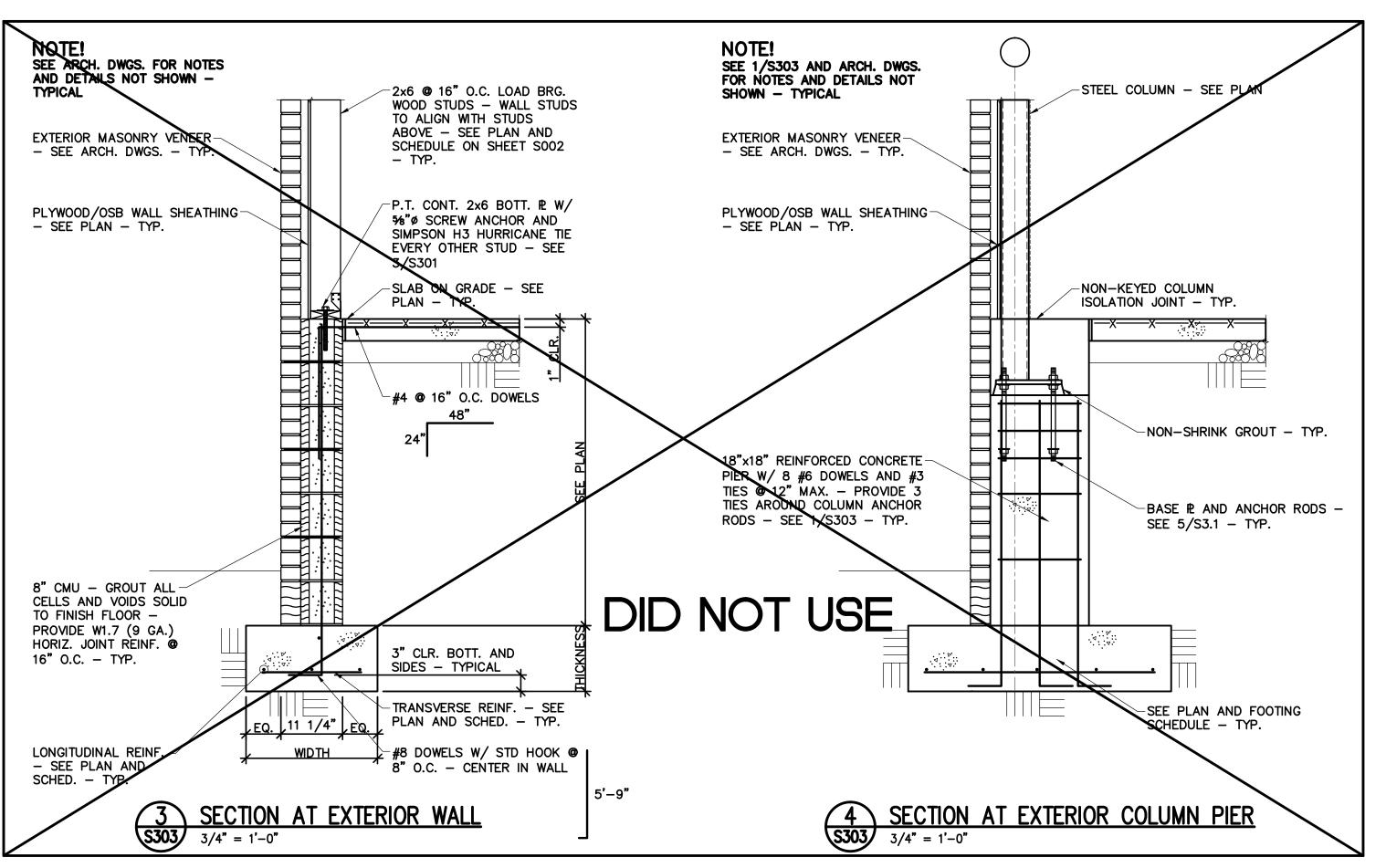
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S302











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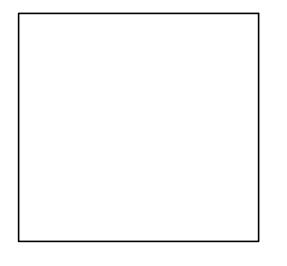
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KEY PLAN

Southern Hospitality Services

Hampton Inn and Suites

5400 I-20 & Frontage Rd. Monroe, LA 71201

Drawing Title Foundation Sections and

Details

Construction Documents

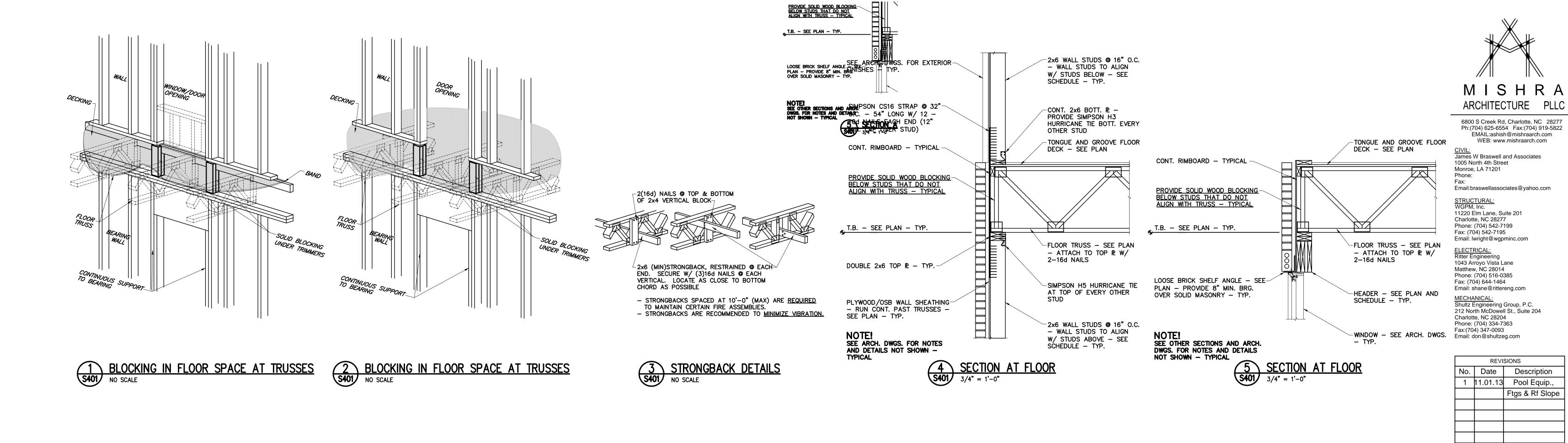


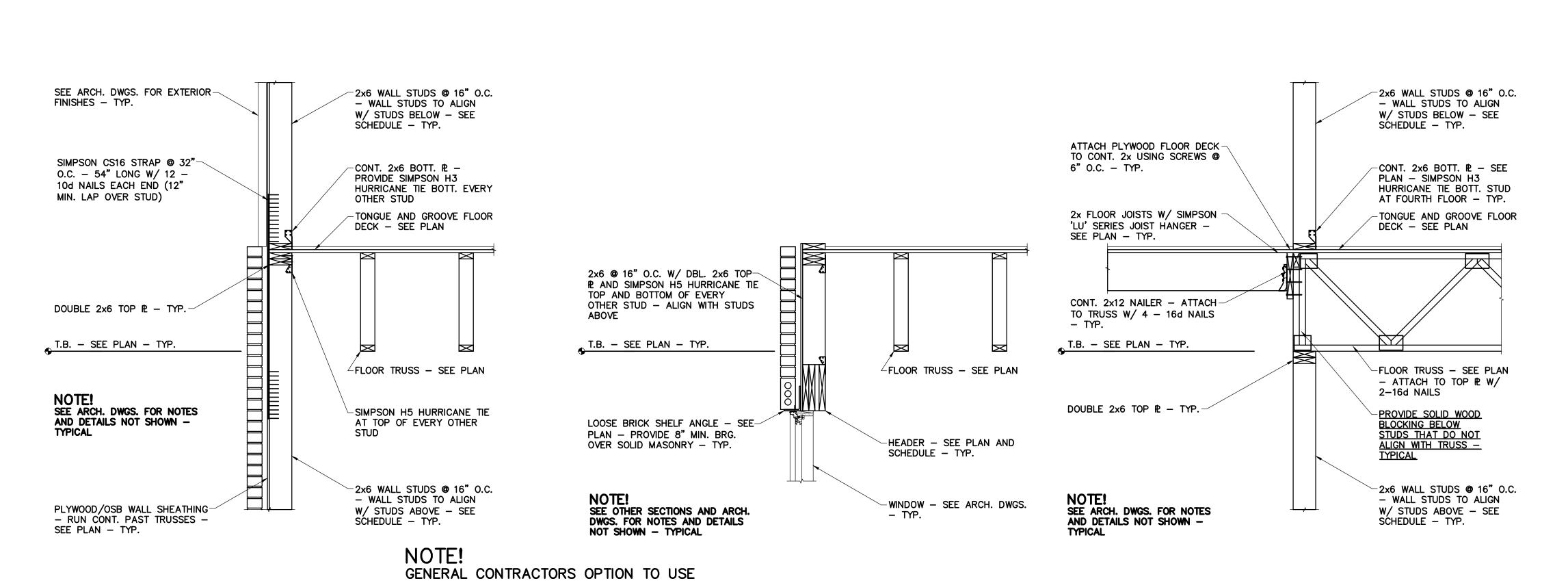
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	WAL ENNINE
	Q 1, 1, 2
	9/16/13
	, , ,

Project No.	12-111
Prepared by	AB/LW
Checked by	HLW

Date September 16, 2013

S303





NOTE! SEE ARCH. DWGS. FOR NOTES AND DETAILS NOT SHOWN -9 SECTION AT CORRIDOR 3/4" = 1'-0"



5400 I-20 & Frontage Rd. Monroe, LA 71201

Drawing Title Floor Framing Sections and Details

12-111 AB/LW HLW ate September 16, 2013

8 SECTION AT CORRIDOR
3/4" = 1'-0"

W/ STUDS ABOVE - SEE MISHRA ARCHITECTURE PLLC SCHEDULE - TYP. ATTACH PLYWOOD FLOOR DECK-TO LVL USING SCREWS @ 6" -CONT. 2x6 BOTT. P. - SEE PLAN FOR SIMPSON H3 HURRICANE TIE BOTT. STUD AT THIRD FLOOR - TYP. 2x FLOOR JOISTS W/ SIMPSON TONGUE AND GROOVE FLOOR DECK - SEE PLAN 'LU' SERIES JOIST HANGER -SEE PLAN - TYP. T.B. - SEE PLAN - TYP. LVL - SEE PLAN - TYPICAL -FLOOR TRUSS - SEE PLAN - HANGER/CONNECTIONS TO BE DESIGNED BY SUPPLIER

2x6 WALL STUDS @ 16" 0.C.

- WALL STUDS TO ALIGN

KEY PLAN

Southern Hospitality Services

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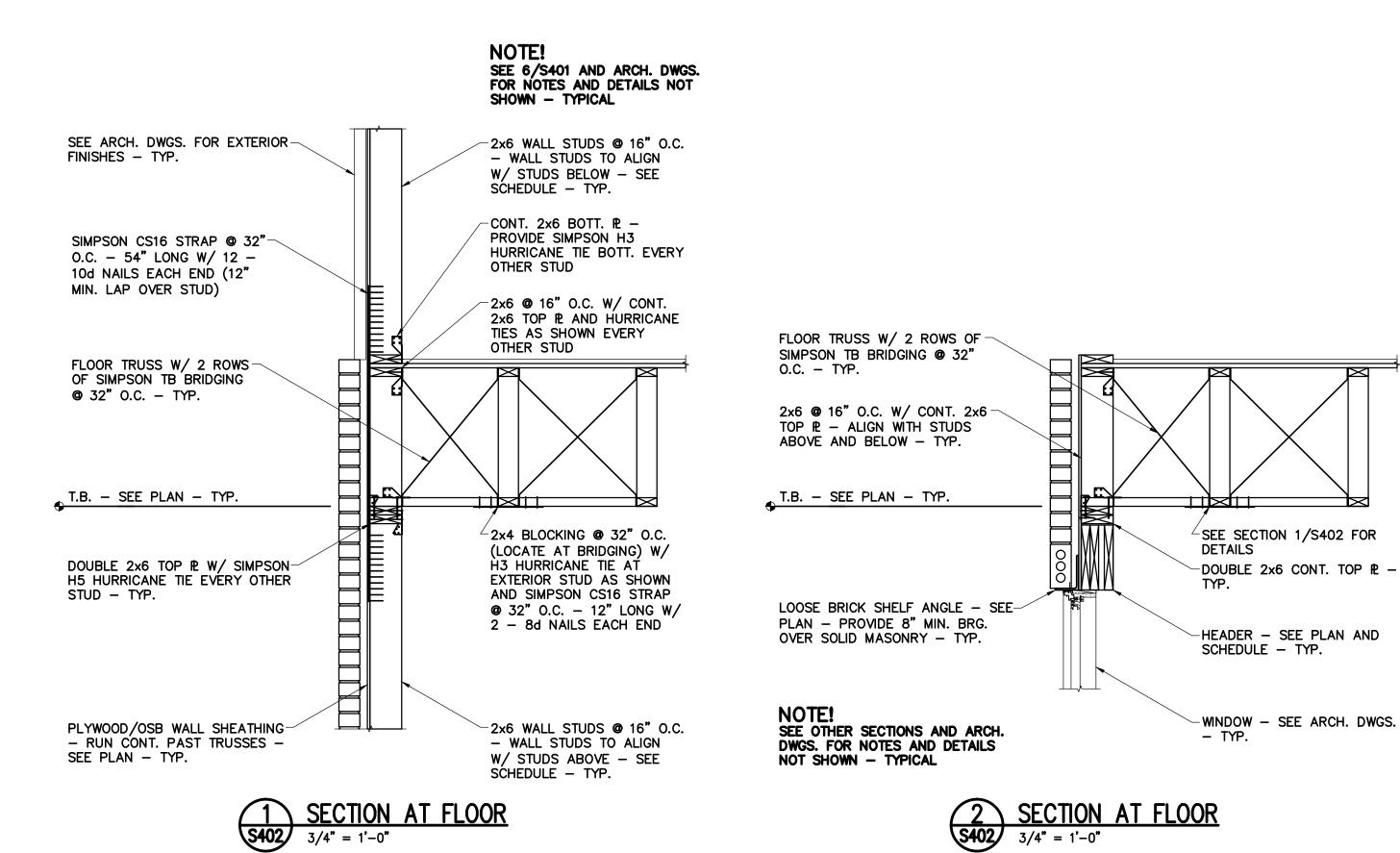
Hampton Inn and Suites

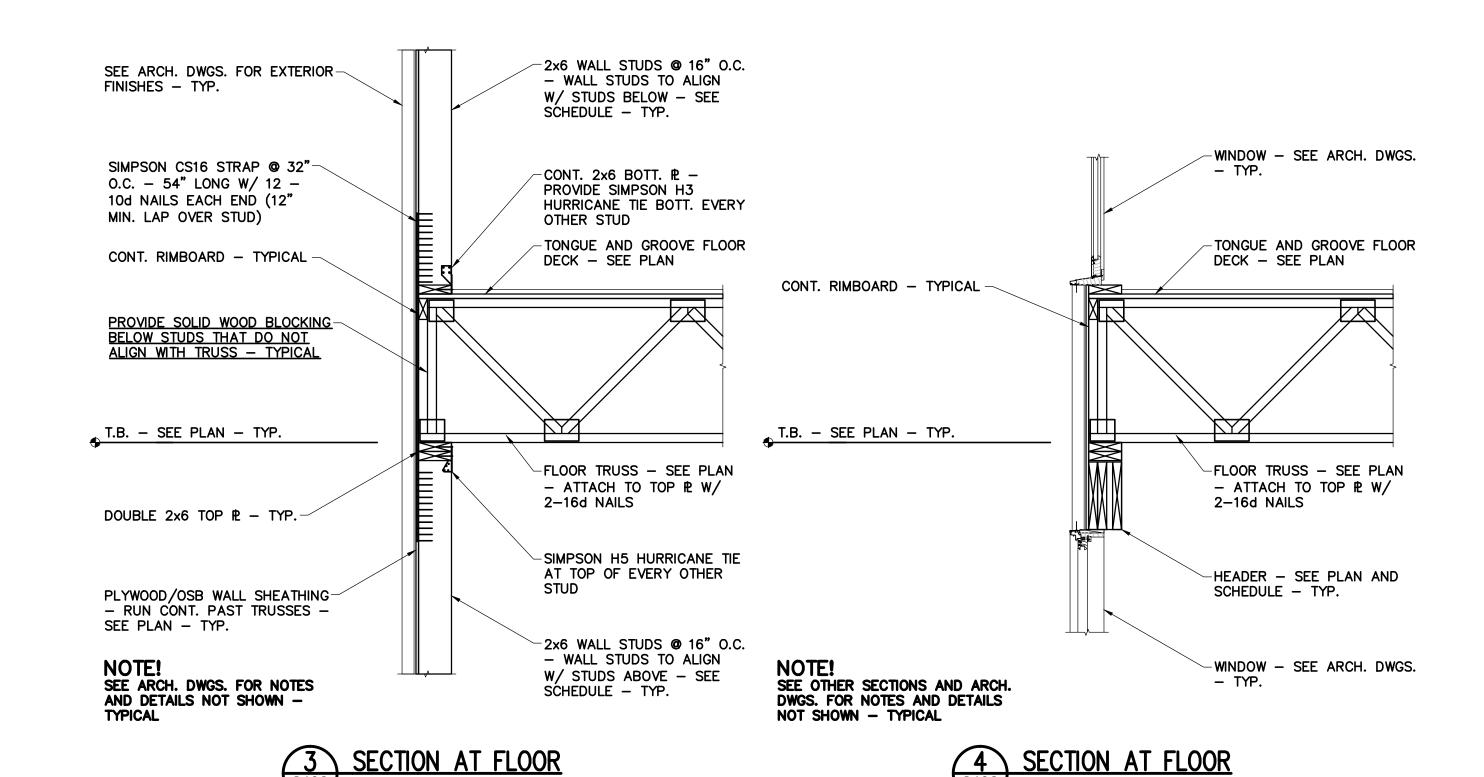
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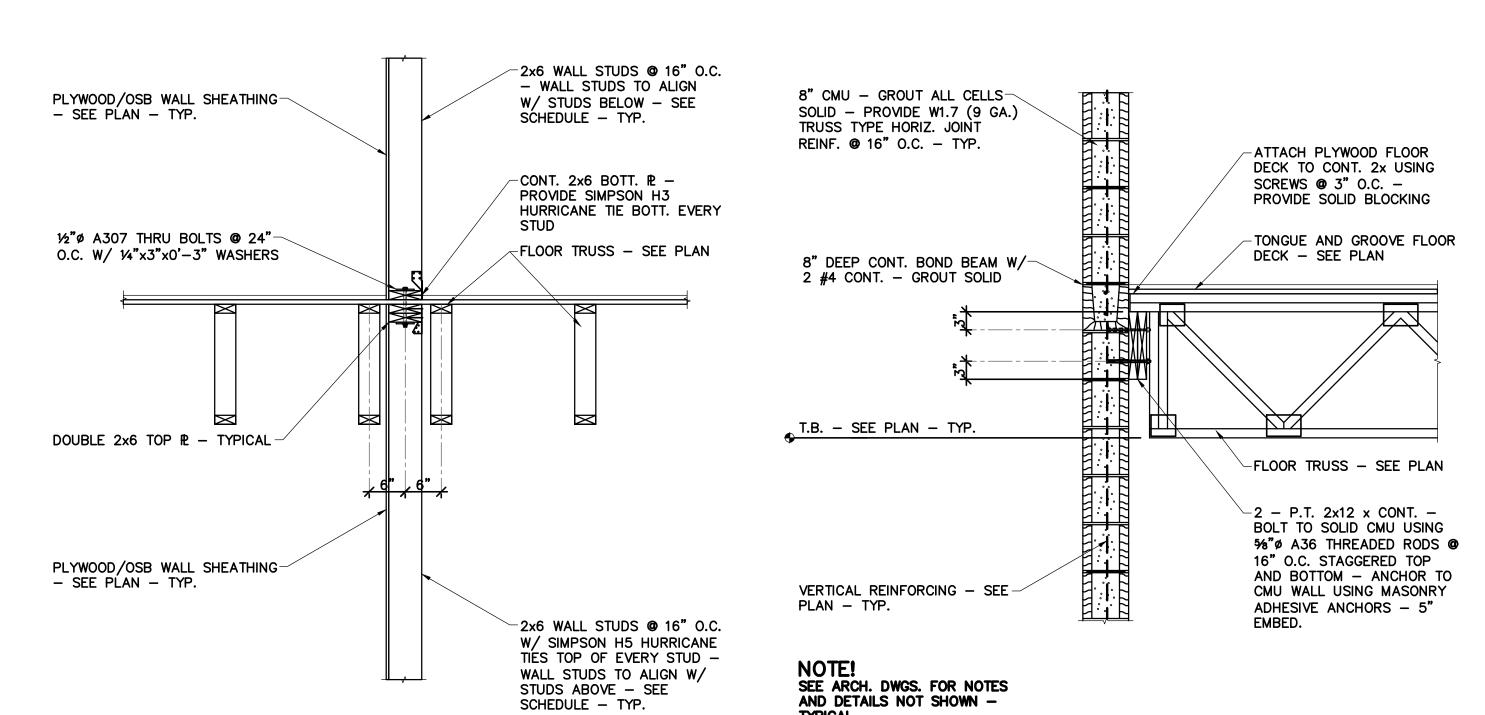
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6 SECTION AT FLOOR \$401 3/4" = 1'-0"

SECTIONS 6 & 7/S401 OR 1 & 2/S402

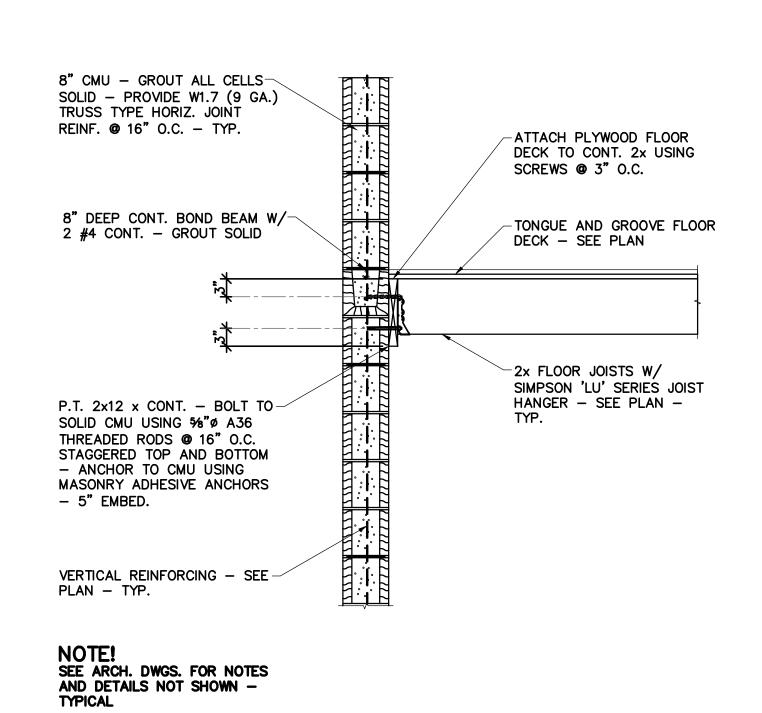


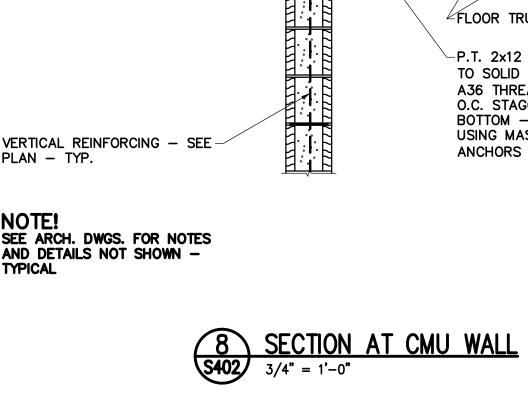




SECTION AT SHEAR WALL

3/4" = 1'-0"









5400 I-20 & Frontage Rd. Monroe, LA 71201 Drawing Title

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212 North McDowell St., Suite 204

REVISIONS

No. Date Description

1 11.01.13 Pool Equip.,

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Email: don@shultzeg.com

Monroe, LA 71201

Phone:

Fax:

Floor Framing Sections and Details

KEY PLAN

Services

Suites

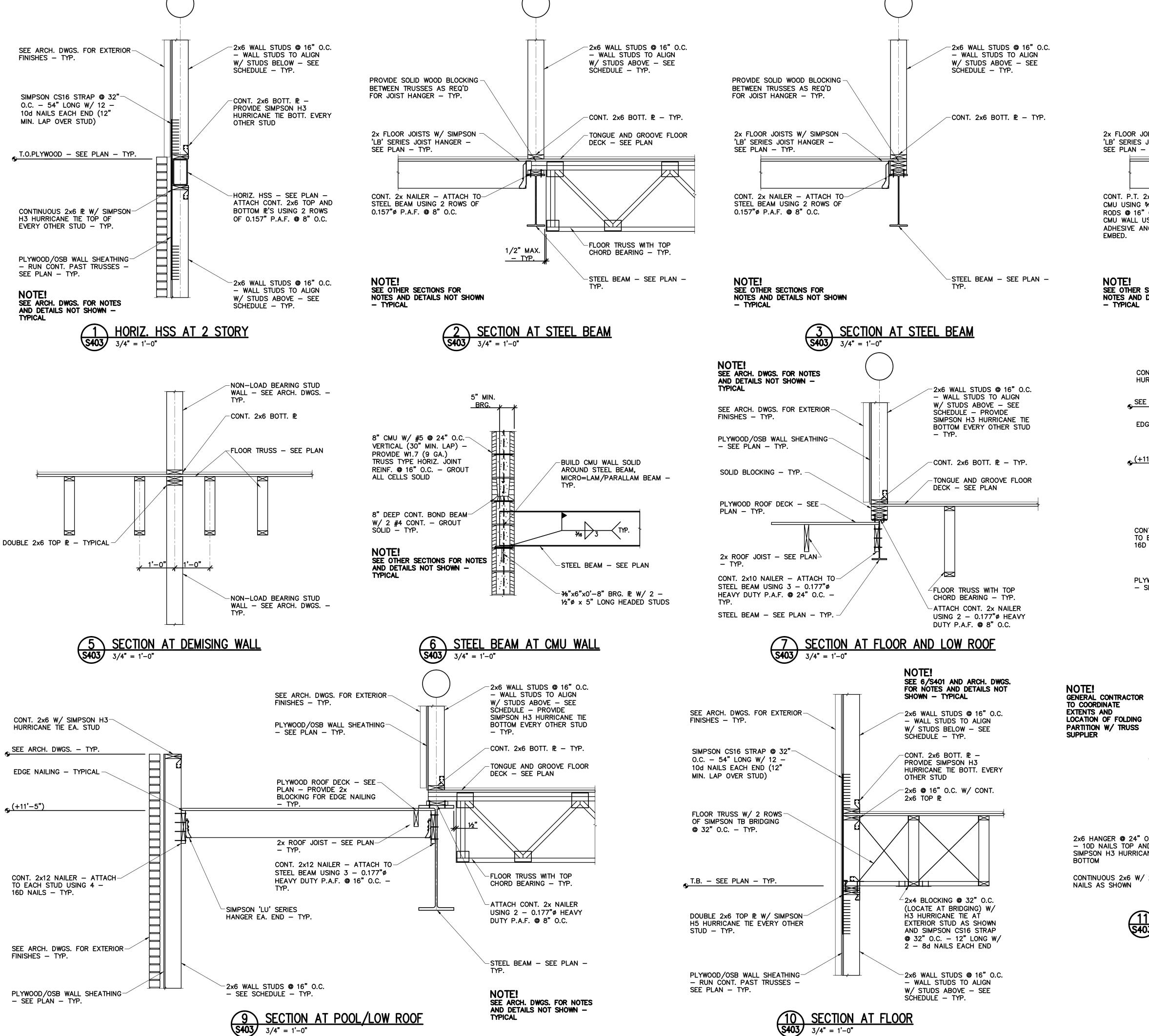
Construction Documents

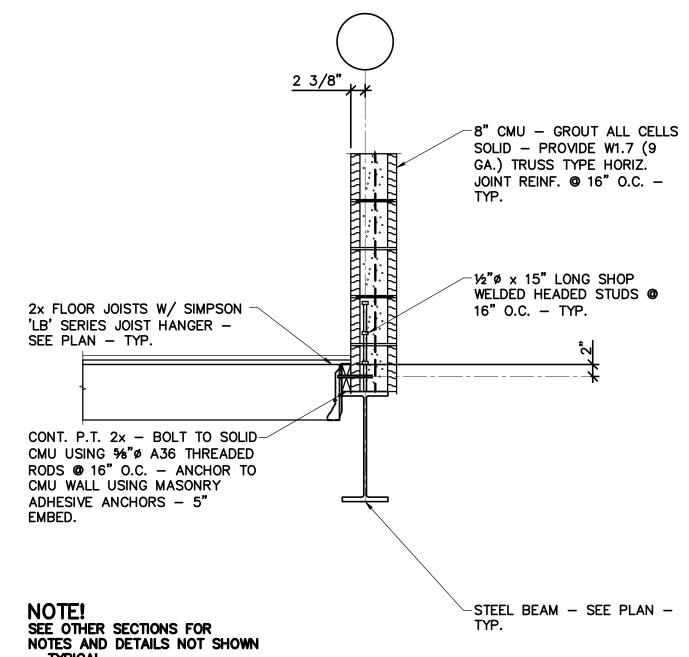
12-111 AB/LW HLW ^{lle}September 16, 2013

8" CMU - GROUT ALL CELLS-SOLID - PROVIDE W1.7 (9 GA.) TRUSS TYPE HORIZ. JOINT REINF. @ 16" O.C. - TYP. -ATTACH PLYWOOD FLOOR DECK TO CONT. 2x USING SCREWS @ 3" O.C. 8" DEEP CONT. BOND BEAM W/ 2 #4 CONT. - GROUT SOLID TONGUE AND GROOVE FLOOR DECK - SEE PLAN T.B. - SEE PLAN - TYP. FLOOR TRUSS - SEE PLAN -P.T. 2x12 x CONT. - BOLT TO SOLID CMU USING 5/8"Ø A36 THREADED RODS @ 16" O.C. STAGGERED TOP AND BOTTOM - ANCHOR TO CMU USING MASONRY ADHESIVE VERTICAL REINFORCING - SEE ANCHORS - 5" EMBED. PLAN - TYP.

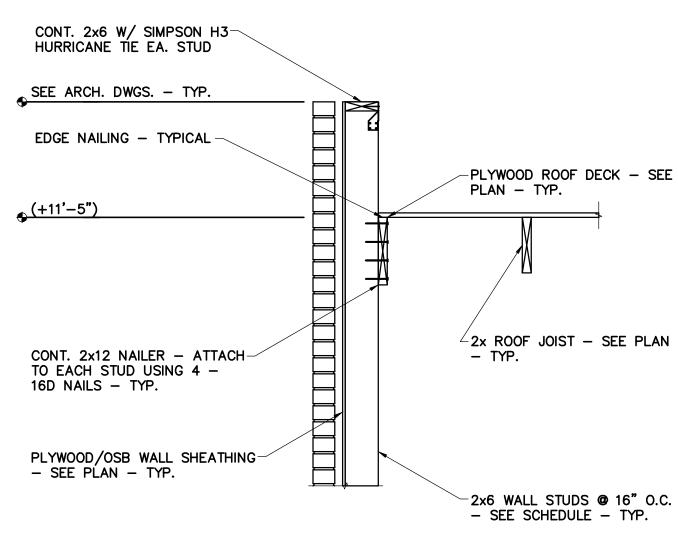
NOTE! SEE ARCH. DWGS. FOR NOTES AND DETAILS NOT SHOWN -

Prepared by Checked by

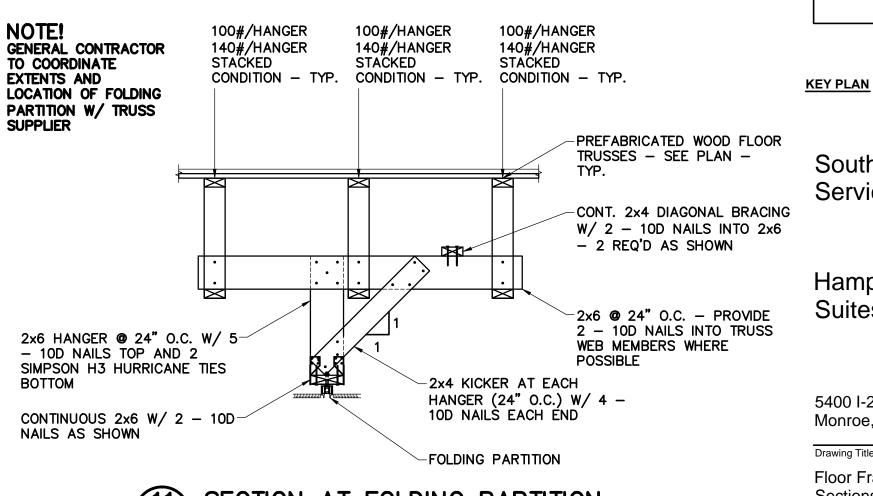






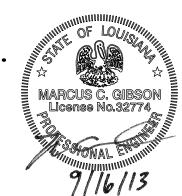


SECTION AT LOW ROOF 3/4" = 1'-0"











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MECHANICAL: Shultz Engineering Group, P.C. 212 North McDowell St., Suite 204 Charlotte, NC 28204 Phone: (704) 334-7363 Fax:(704) 347-0093 Email: don@shultzeg.com

REVISIONS						
No.	Date Description					
1	11.01.13	Pool Equip.,				
		Ftgs & Rf Slope				

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Southern Hospitality Services

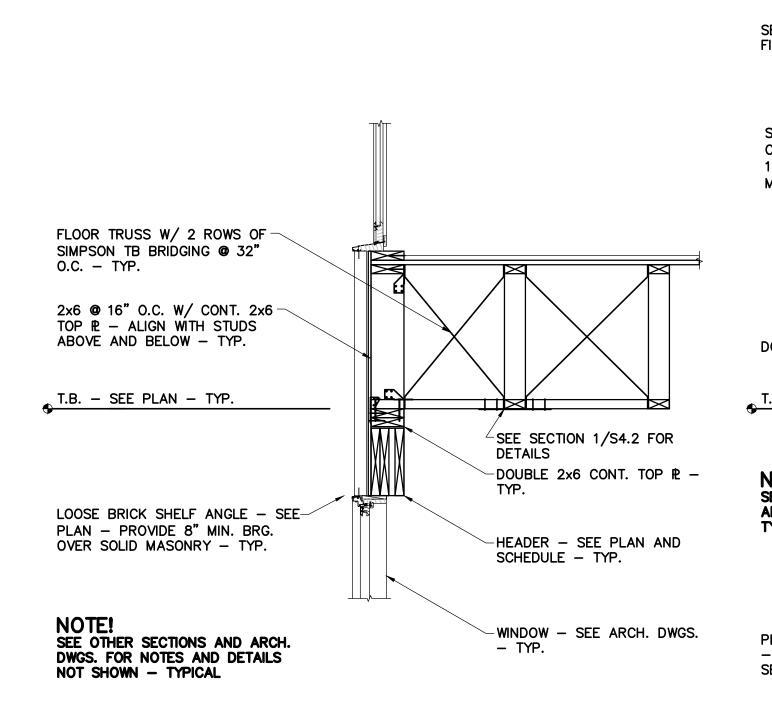
Hampton Inn and

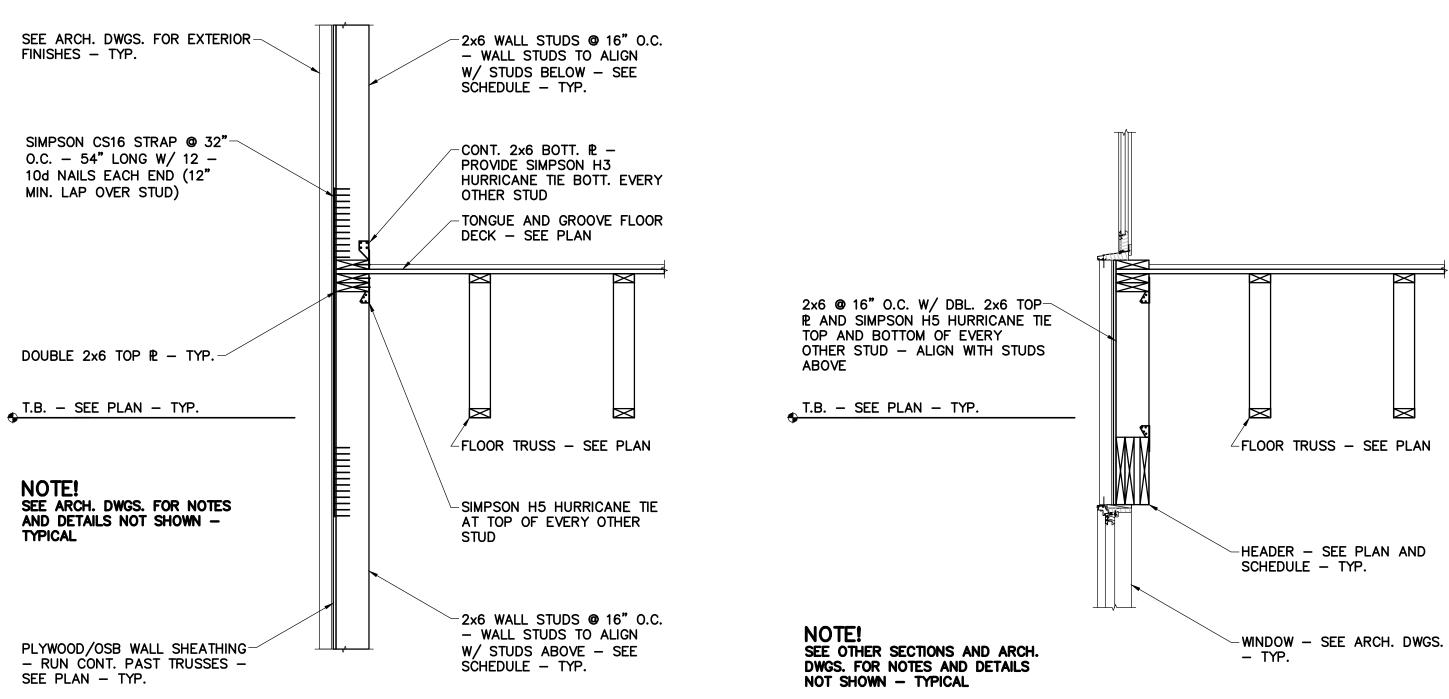
5400 I-20 & Frontage Rd. Monroe, LA 71201

Drawing Title Floor Framing Sections and Details

Construction Documents

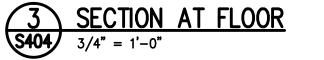
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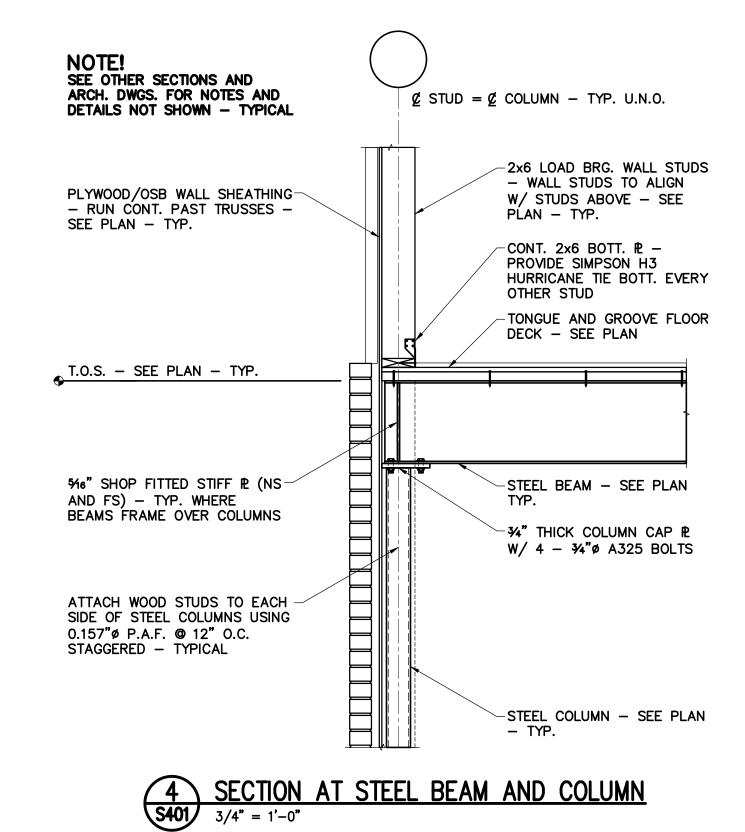




1 SECTION AT FLOOR 3/4" = 1'-0"

NOTE! GENERAL CONTRACTORS OPTION TO USE SECTIONS 10/S403 & 1/S404 OR 2 & 3/\$404

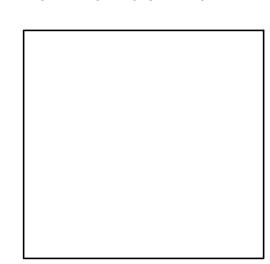






Fax: Email:braswellassociates@yahoo.com STRUCTURAL: WGPM, Inc. 11220 Elm Lane, Suite 201 Charlotte, NC 28277 Phone: (704) 542-7199 Fax: (704) 542-7195 Email: lwright@wgpminc.com ELECTRICAL: Ritter Engineering 1043 Arroyo Vista Lane Matthew, NC 28014 Phone: (704) 516-0385 Fax: (704) 644-1464 Email: shane@rittereng.com MECHANICAL:
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212 North McDowell St., Suite 204 Charlotte, NC 28204 Phone: (704) 334-7363 Fax:(704) 347-0093 Email: don@shultzeg.com REVISIONS No. Date Description 1 11.01.13 Pool Equip., Ftgs & Rf Slope

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KEY PLAN

Southern Hospitality Services

Hampton Inn and Suites

5400 I-20 & Frontage Rd. Monroe, LA 71201

Drawing Title Floor Framing

Sections and Details

Construction Documents

MARCUS C. GIBSON License No.32774

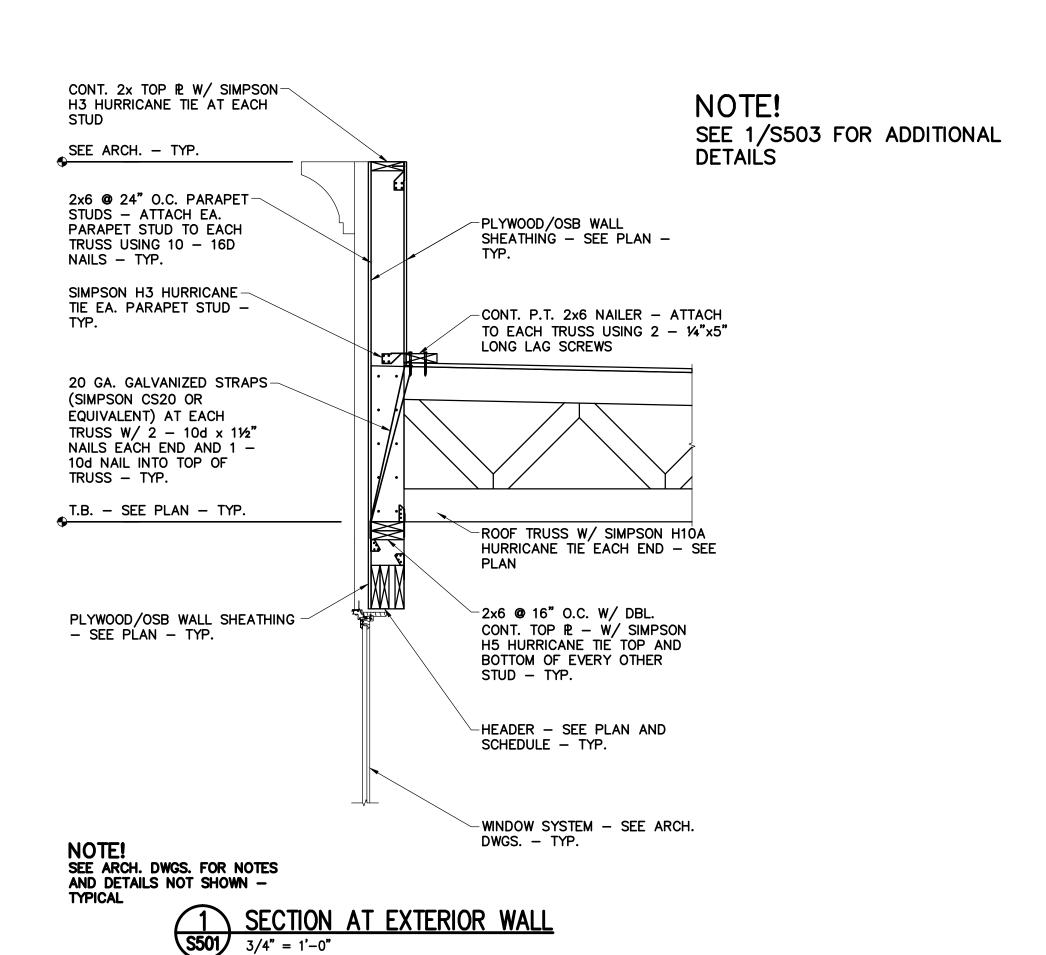
WGPM, Inc.

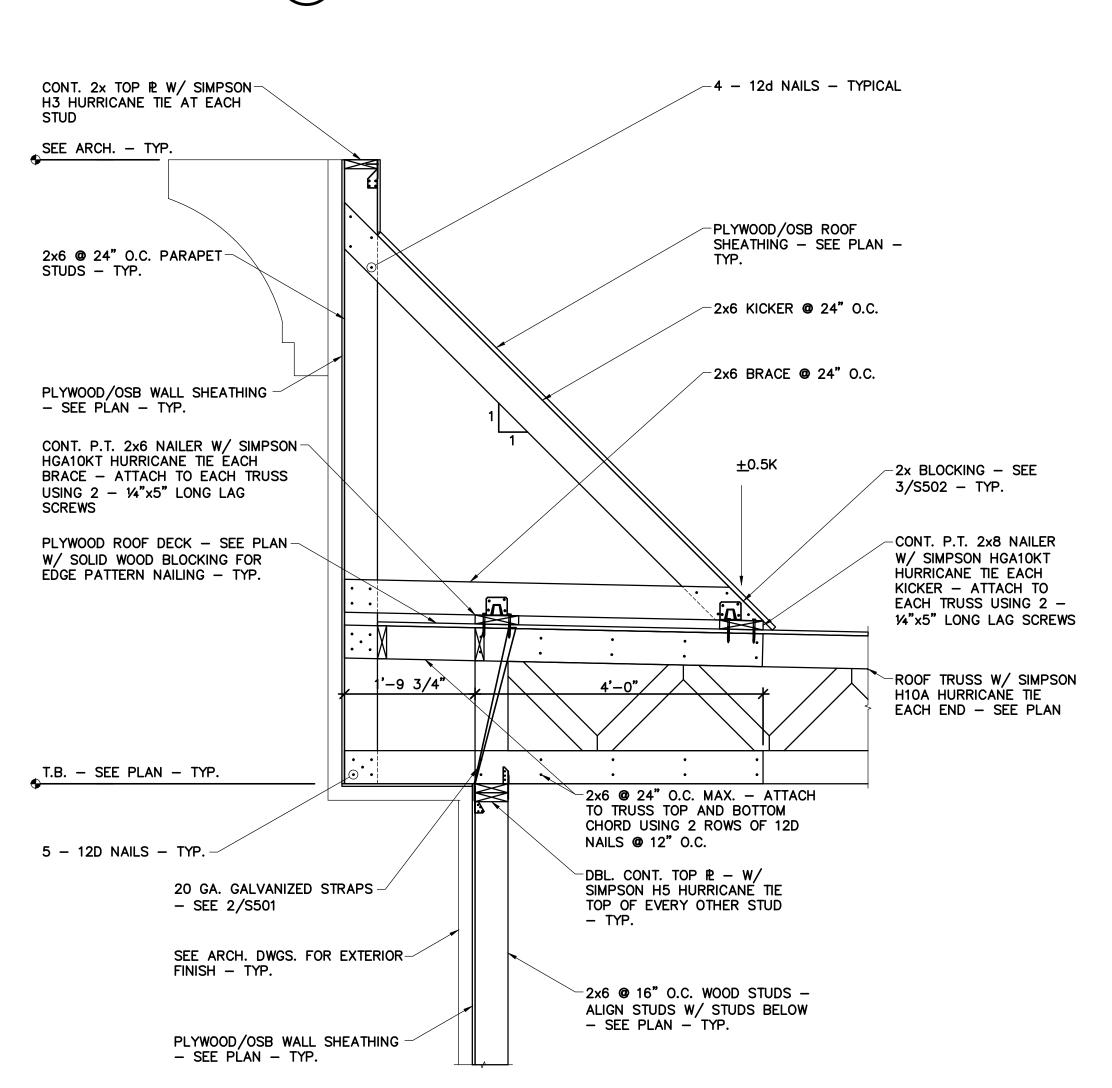
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JOB NUMBER: 57-13

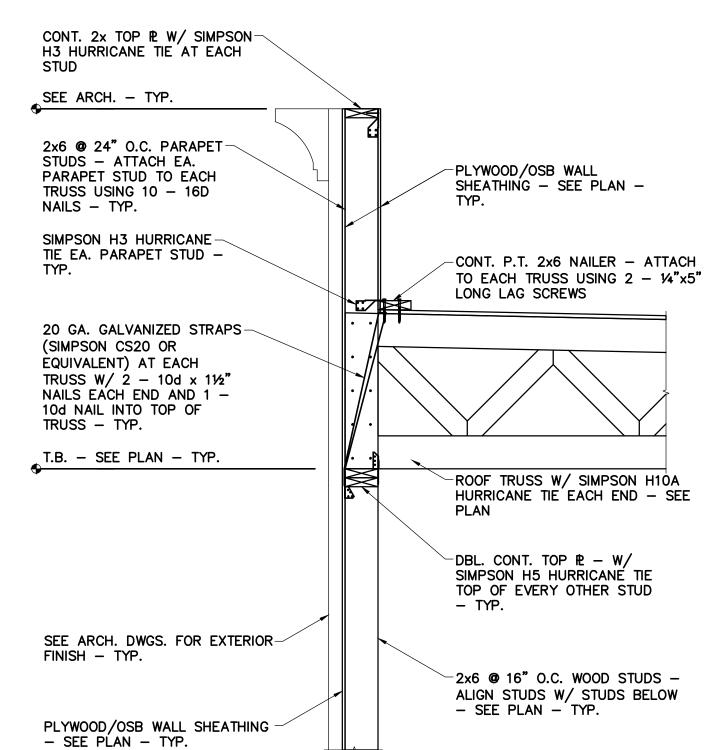
12-111 Prepared by AB/LW Checked by HLW Date September 16, 2013





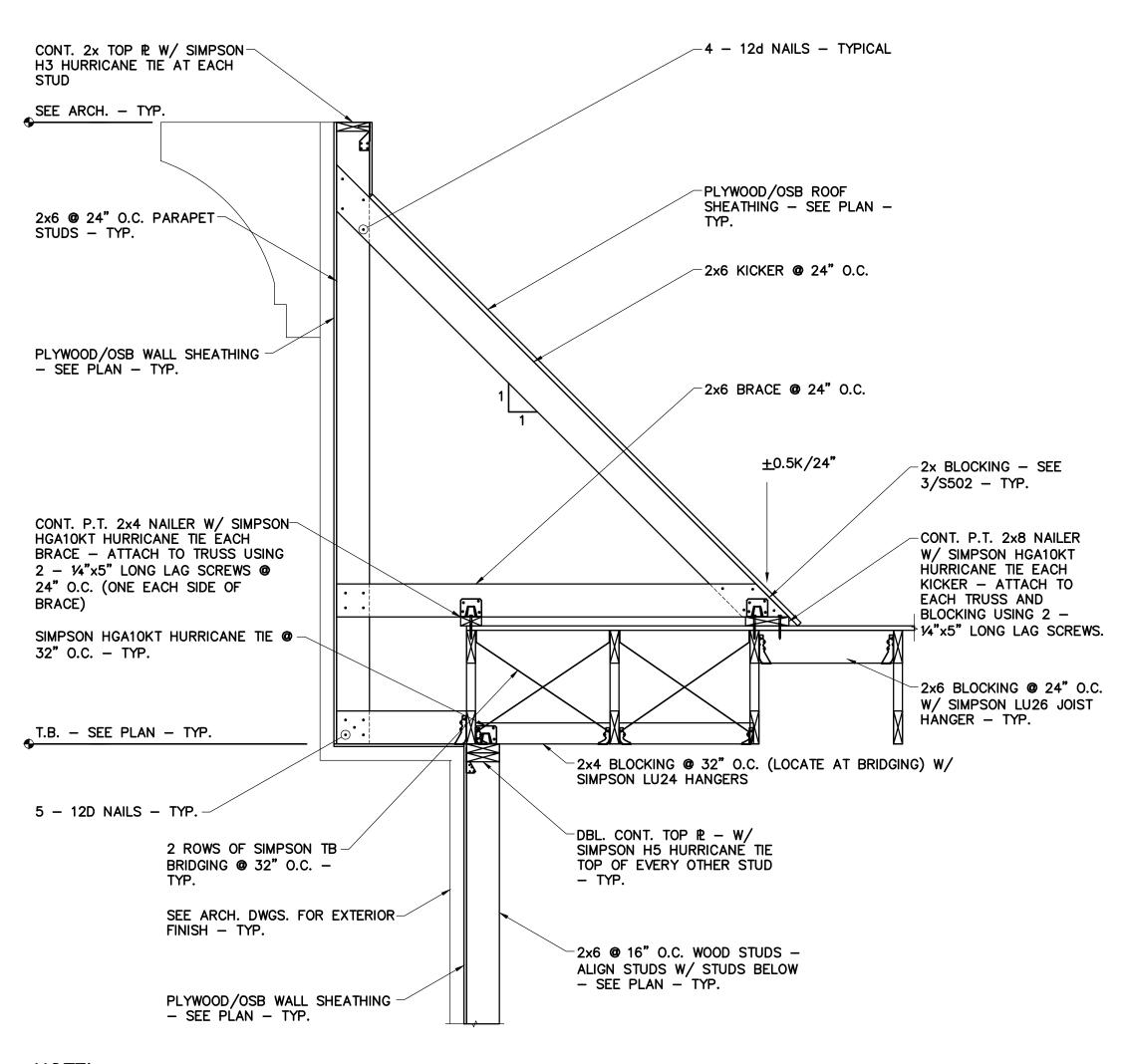
NOTE! SEE OTHER SECTIONS AND ARCH. DWGS. FOR NOTES AND DETAILS NOT SHOWN - TYPICAL





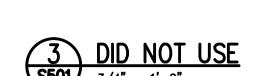
SEE ARCH. DWGS. FOR NOTES AND DETAILS NOT SHOWN -**TYPICAL**

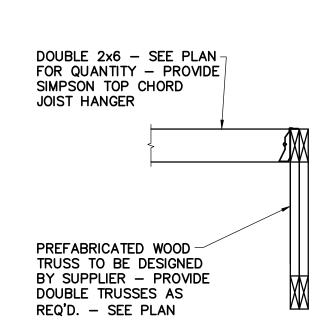
<u>\ SECTION AT EXTERIOR WALL</u>



NOTE! SEE OTHER SECTIONS AND ARCH. DWGS. FOR NOTES AND DETAILS NOT SHOWN - TYPICAL







WGPM, Inc.

Wright + Gibson + Patton

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Charlotte, North Carolina 28277 704-542-7199 Fax: 704-542-7195 www.wgpminc.com

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KEY PLAN

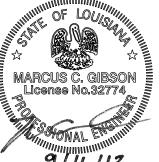
Southern Hospitality

Hampton Inn and Suites

Drawing Title

Construction Documents







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Email:braswellassociates@yahoo.com 11220 Elm Lane, Suite 201 Charlotte, NC 28277 Phone: (704) 542-7199 Fax: (704) 542-7195

Email: lwright@wgpminc.com

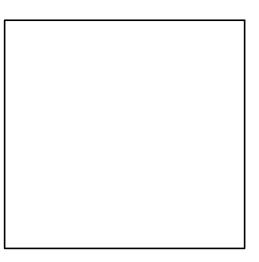
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REVISIONS							
No.	Date	Description					
1	11.01.13	Pool Equip.,					
		Ftgs & Rf Slope					

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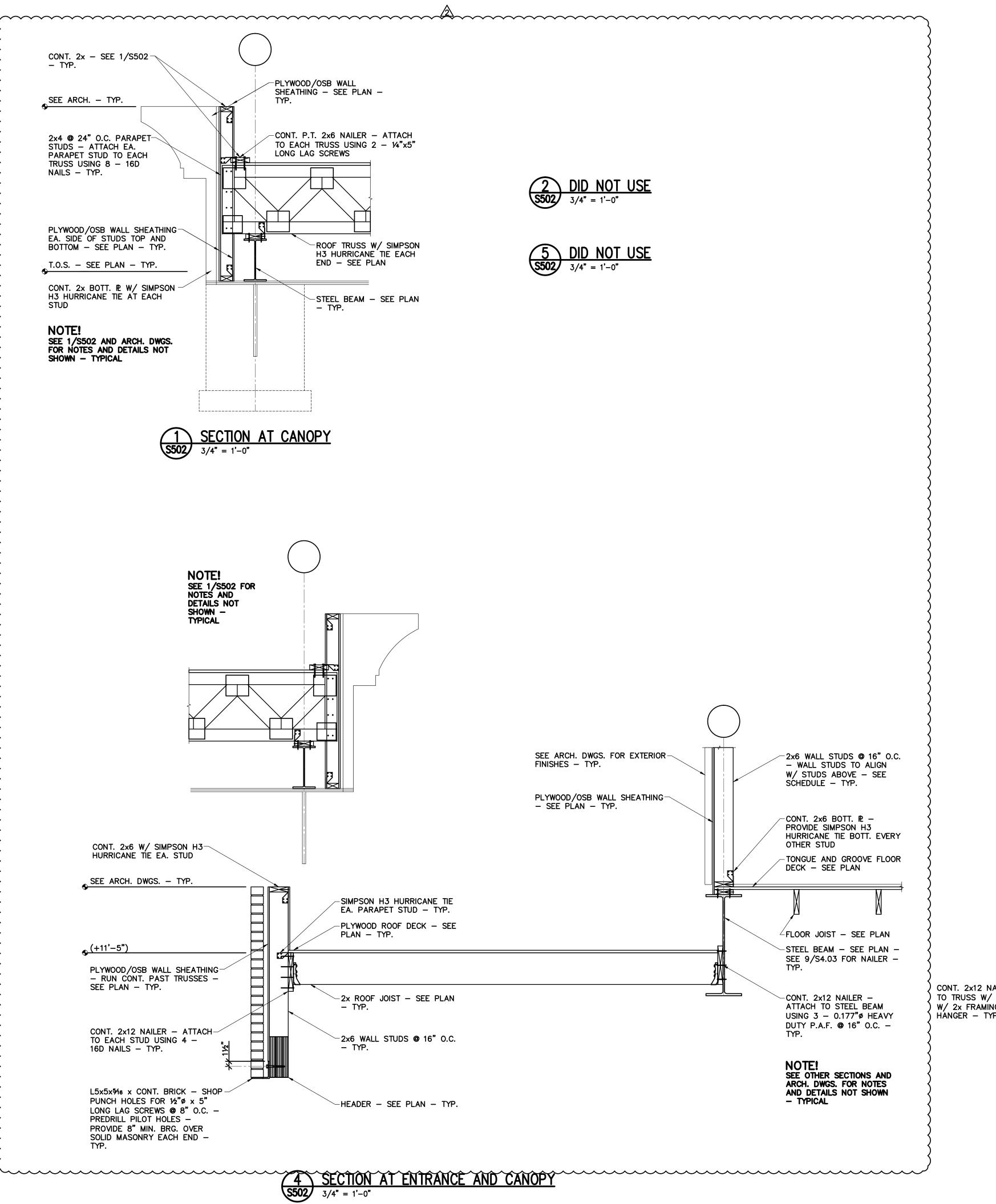
Services

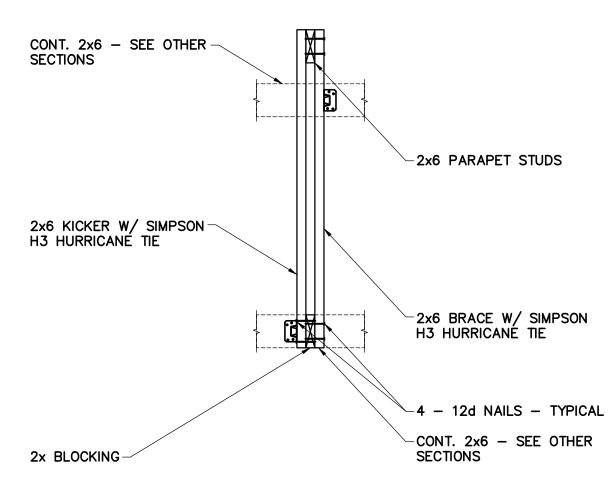
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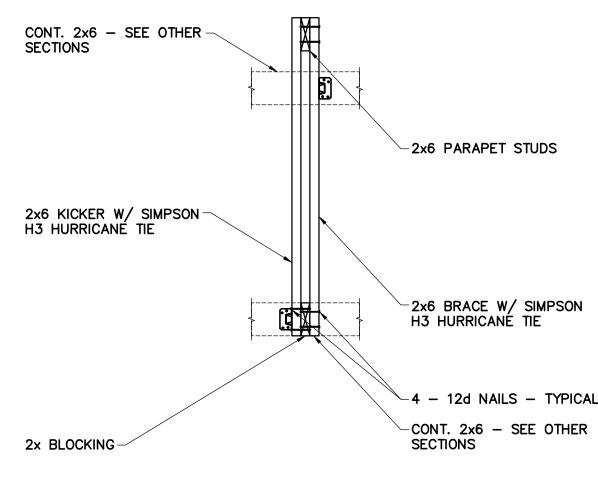
Roof Framing Sections and Details

12-111 Prepared by

AB/LW HLW ate September 16, 2013





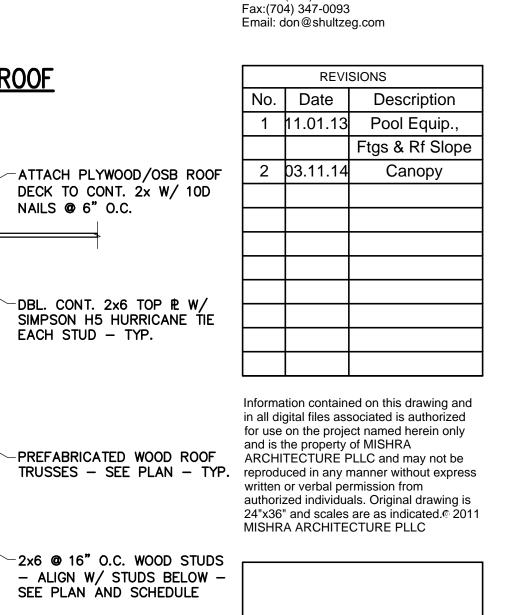


6 SECTION AT SHEAR WALL

\$502 3/4" = 1'-0"

NAILS @ 6" O.C.

EACH STUD - TYP.



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Fax: (704) 644-1464

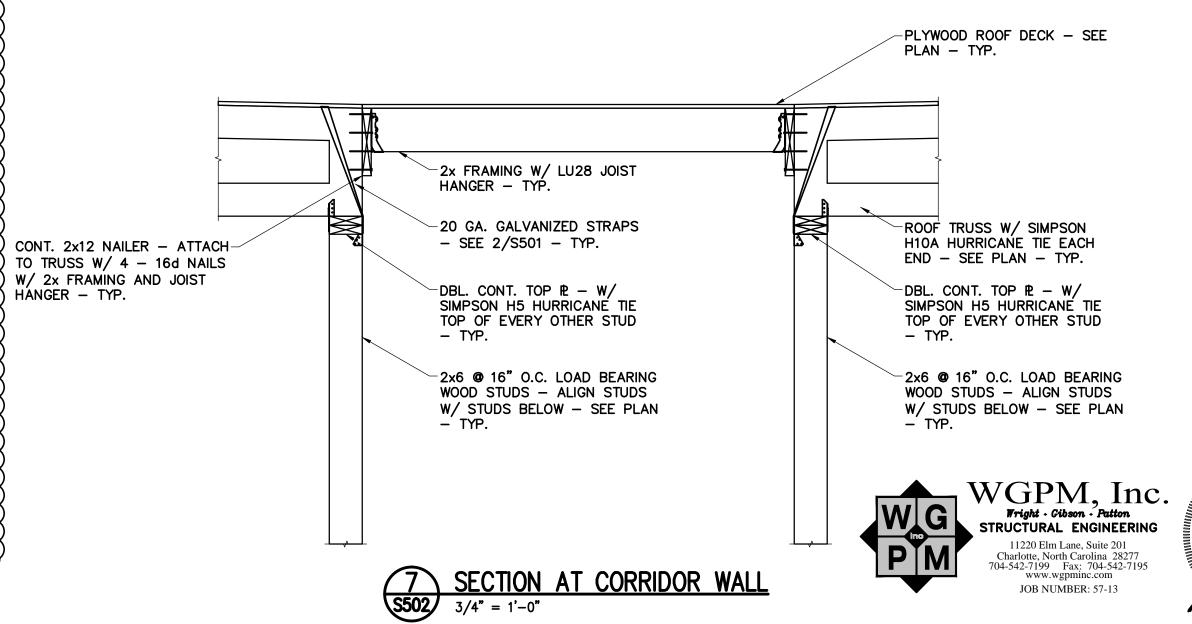
Charlotte, NC 28204

Phone: (704) 334-7363

Phone:

Fax:

KEY PLAN



PLYWOOD ROOF DECK - SEE-

PLYWOOD/OSB WALL SHEATHING— — SEE PLAN — TYP.

PLAN - TYP.

Southern Hospitality Services

Hampton Inn and Suites

5400 I-20 & Frontage Rd.

Monroe, LA 71201

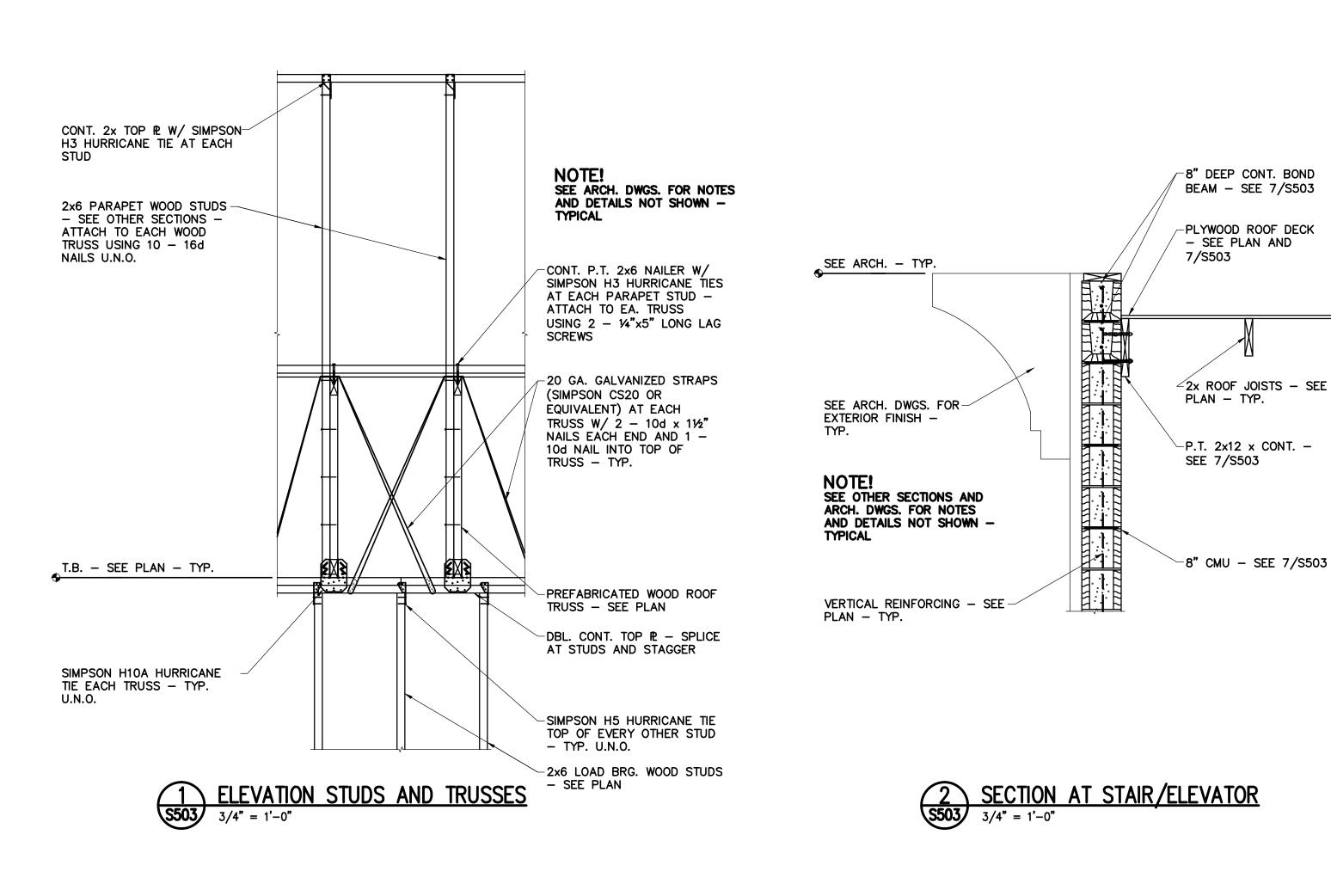
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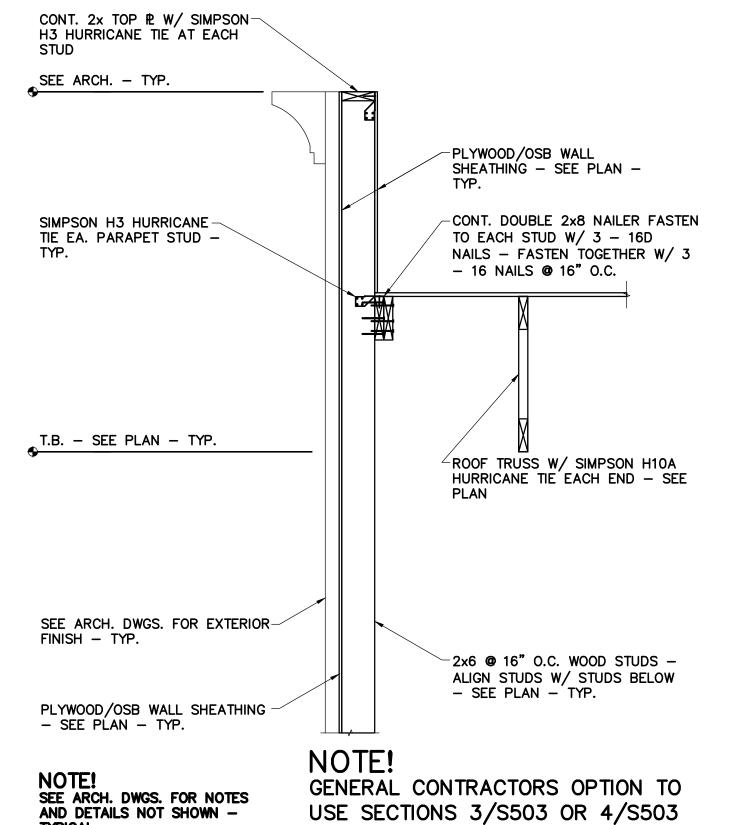
Construction Documents

12-111 Prepared by AB/LW Checked by HLW

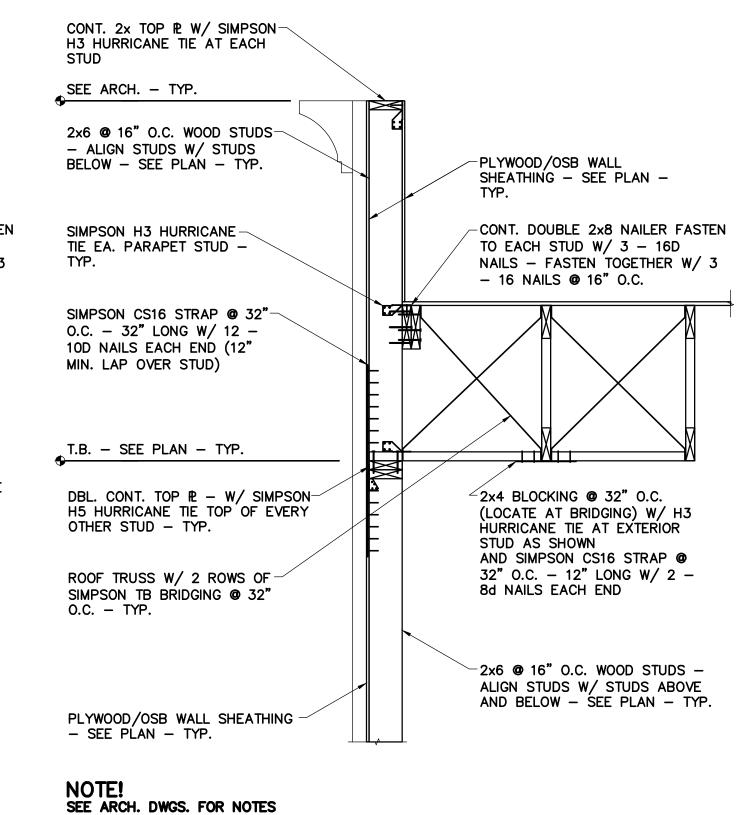
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3 SECTION AT EXTERIOR WALL



4 SECTION AT EXTERIOR WALL

AND DETAILS NOT SHOWN -

TYPICAL

REVISIONS No. Date Description 1 11.01.13 Pool Equip., Ftgs & Rf Slope

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MISHRA

ARCHITECTURE PLLC

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Email: don@shultzeg.com

Phone: (704) 516-0385

Email: shane@rittereng.com

MECHANICAL: Shultz Engineering Group, P.C.

212 North McDowell St., Suite 204

Monroe, LA 71201

Phone:

Fax:

-8" DEEP CONT. BOND BEAM W/ 2 #4 CONT. - GROUT -ATTACH PLYWOOD ROOF DECK TO CONT. 2x USING 10D NAILS @ 6" O.C. - TYP. U.N.O. -PLYWOOD ROOF DECK -2x ROOF JOISTS W/ SIMPSON 'LU' SERIES JOIST HANGER -SEE PLAN - TYP. **KEY PLAN** \neg P.T. 2x12 x CONT. - BOLT TO SOLID CMU USING 5/8" Ø A36 THREADED RODS @ 16" O.C. STAGGERED TOP AND BOTTOM -ANCHOR TO CMU USING MASONRY ADHESIVE ANCHORS -Services 5" EMBED. -8" CMU - GROUT ALL CELLS SOLID - PROVIDE W1.7 (9

GA.) TRUSS TYPE HORIZ.

JOINT REINF. @ 16" O.C. -

Suites

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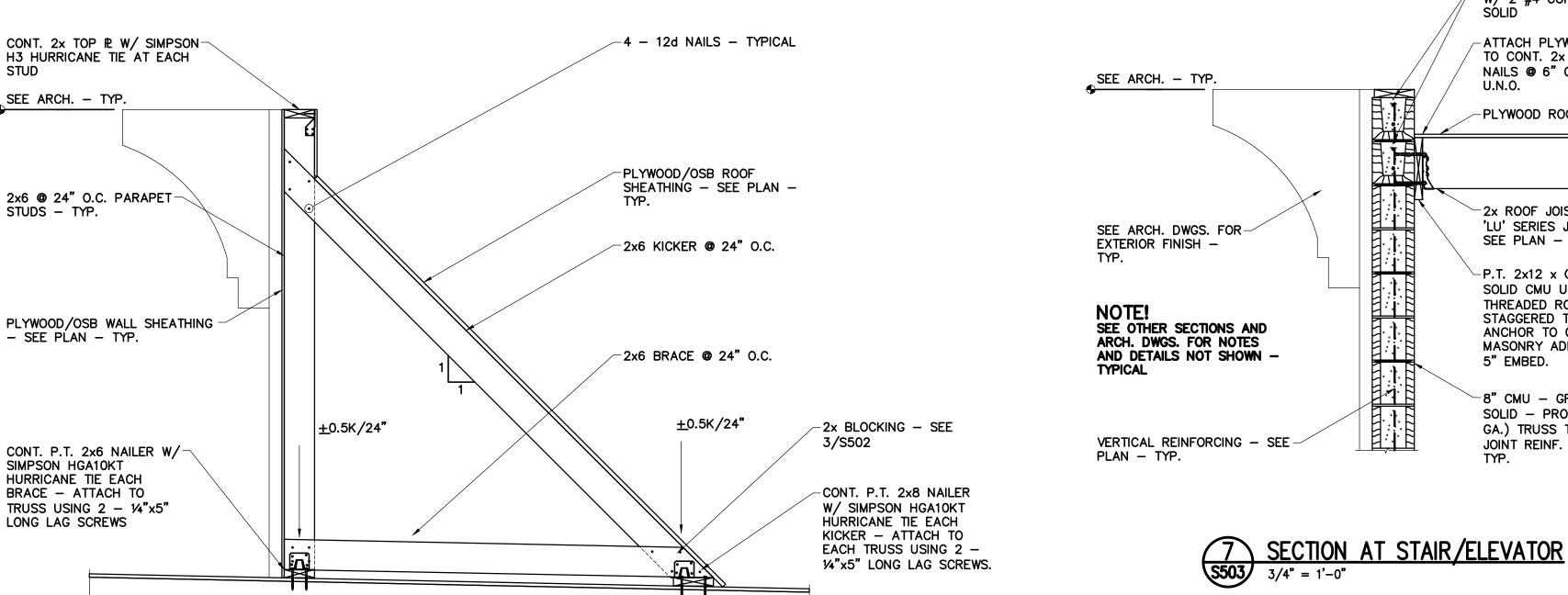
Drawing Title

Construction Documents

12-111 AB/LW HLW

Checked by ^{le}September 16, 2013

ARCH. DWGS. FOR NOTES AND DETAILS NOT SHOWN - TYPICAL 6 SECTION AT PARAPET FRAMING



SEE OTHER SECTIONS AND

TYPICAL



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PLYWOOD/OSB ROOF SHEATHING - SEE PLAN -2x6 @ 24" O.C. PARAPET-STUDS - TYP. -2x6 KICKER @ 24" O.C. PLYWOOD/OSB WALL SHEATHING - SEE PLAN - TYP. -2x6 BRACE @ 24" O.C. ±0.5K/24" ±0.5K/24" CONT. P.T. 2x6 NAILER W/ SIMPSON HGA10KT HURRICANE TIE EACH BRACE - ATTACH TO 2x BLOCKING USING 2 -1/4"x5" LONG LAG SCREWS

CONT. 2x TOP P W/ SIMPSON-

H3 HURRICANE TIE AT EACH

SEE ARCH. - TYP.

STUD

-2x BLOCKING - SEE 3/S502 - TYP.CONT. P.T. 2x8 NAILER W/ SIMPSON HGA10KT HURRICANE TIE EACH KICKER - ATTACH TO 2x BLOCKING USING 2 -1/4"x5" LONG LAG SCREWS.

DETAILS NOT SHOWN - TYPICAL

-4 - 12d NAILS - TYPICAL

2x6 BLOCKING W/ SIMPSON-LU26 JOIST HANGER @ 24" SEE OTHER SECTIONS AND O.C. MAX. - TYP. ARCH. DWGS. FOR NOTES AND

SECTION AT PARAPET FRAMING

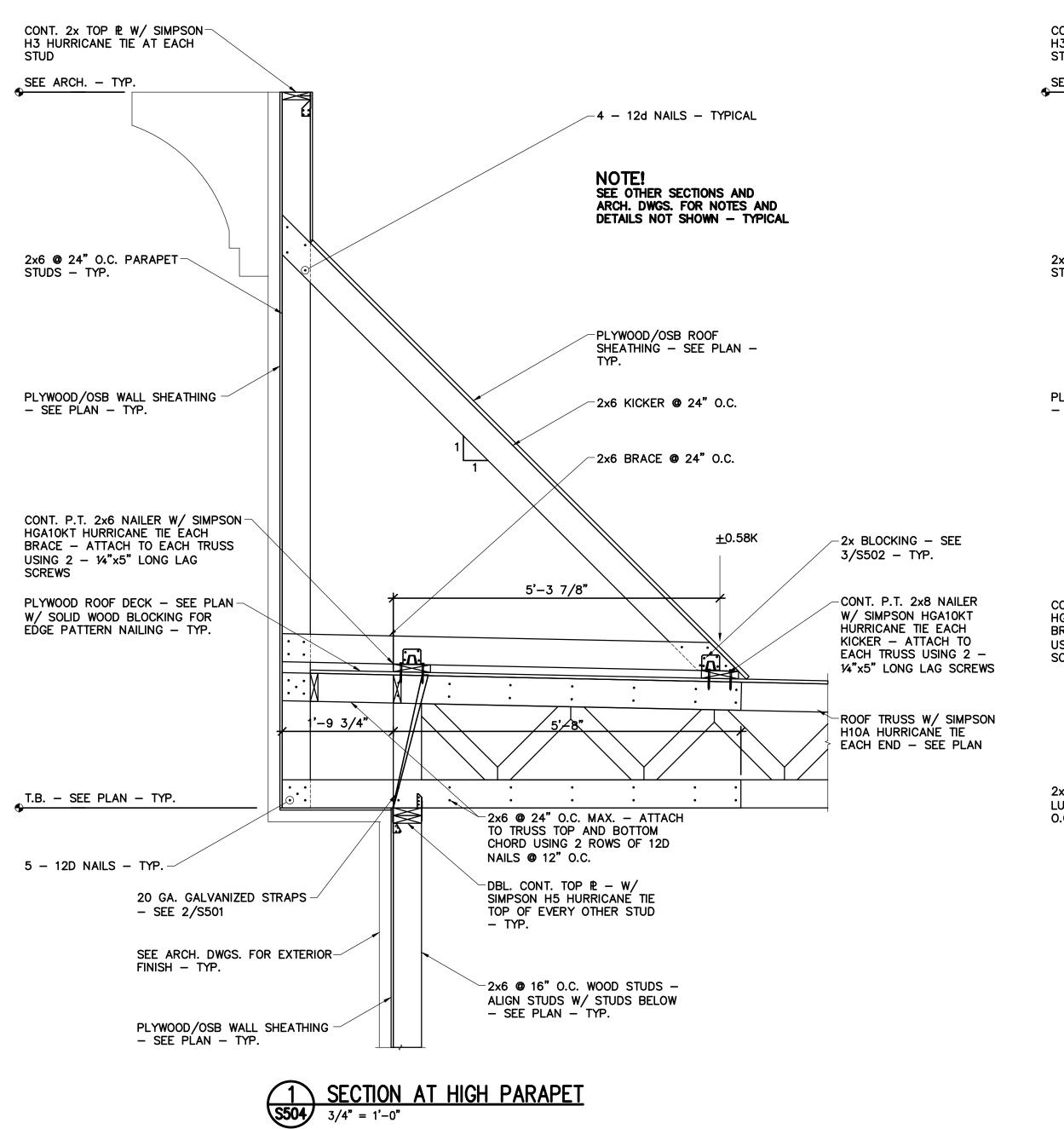
Southern Hospitality

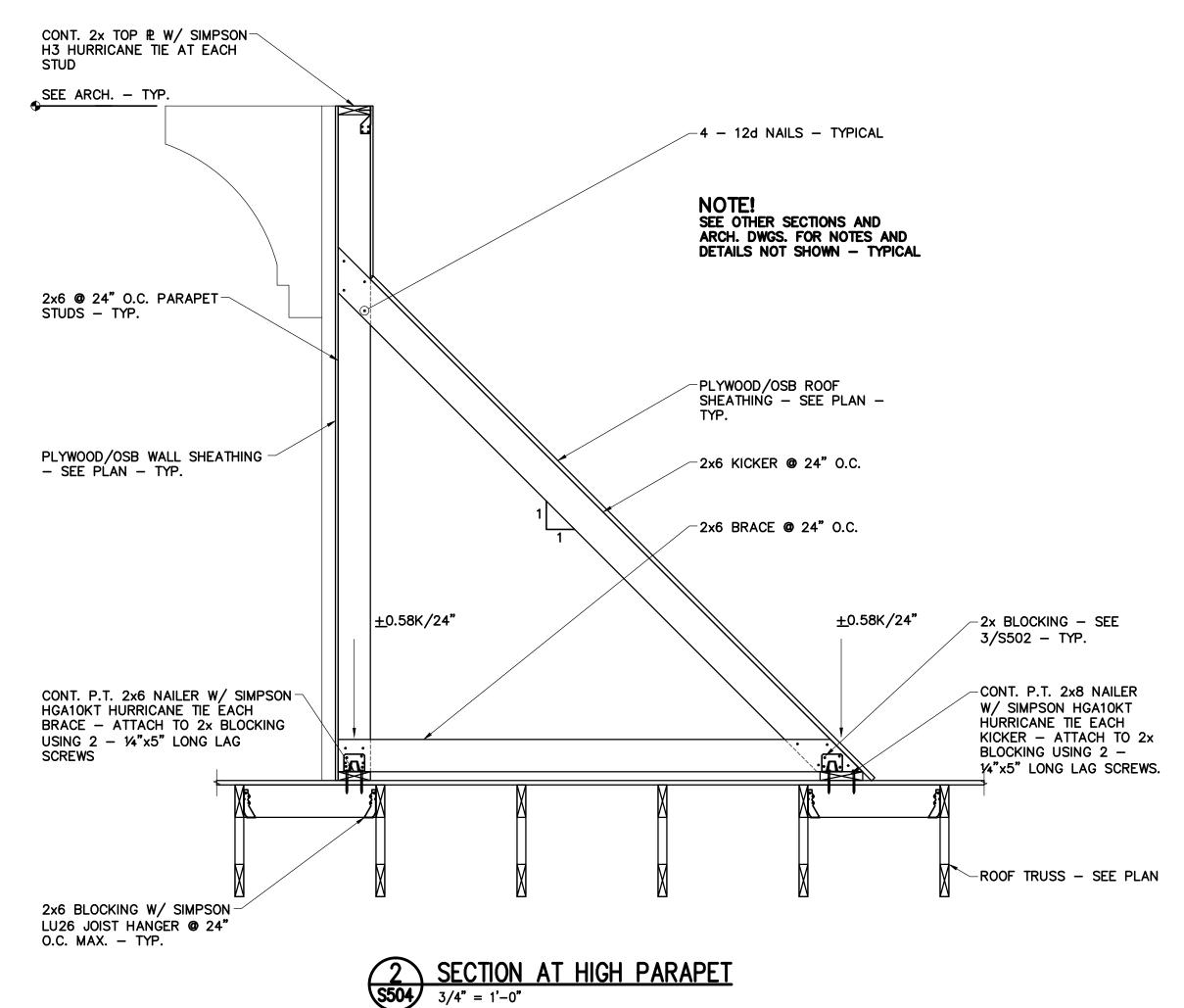
Hampton Inn and

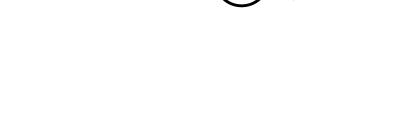
Roof Framing Sections and Details

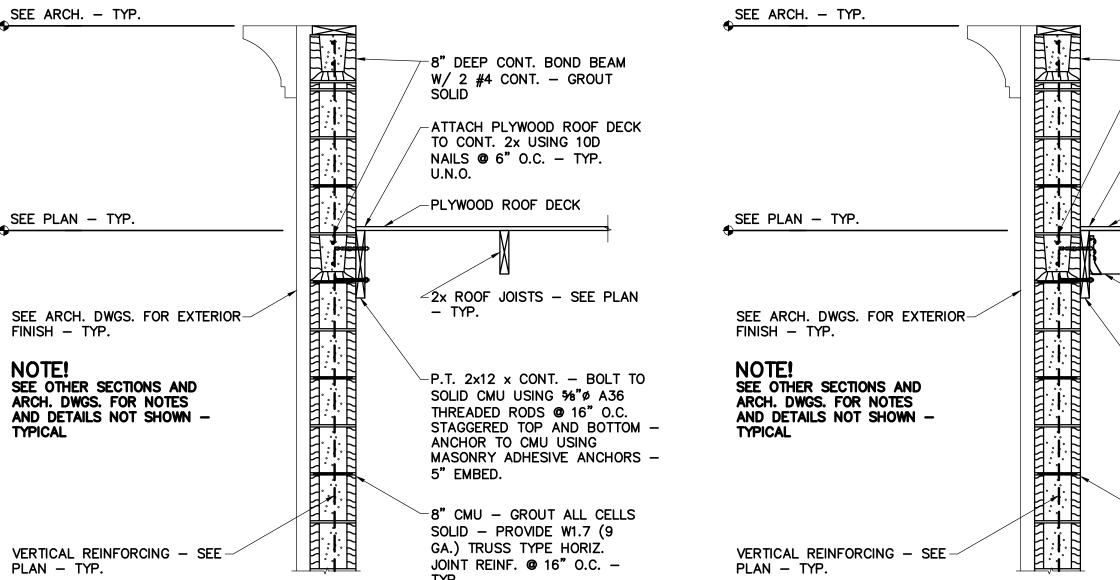
Prepared by

S503

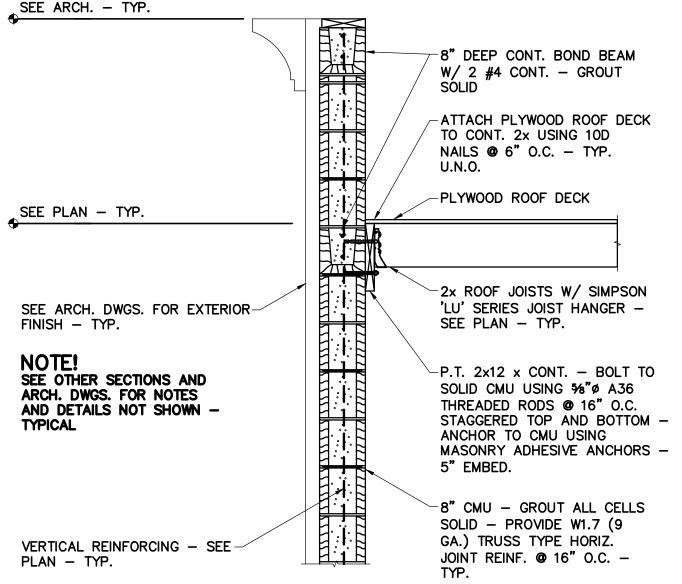




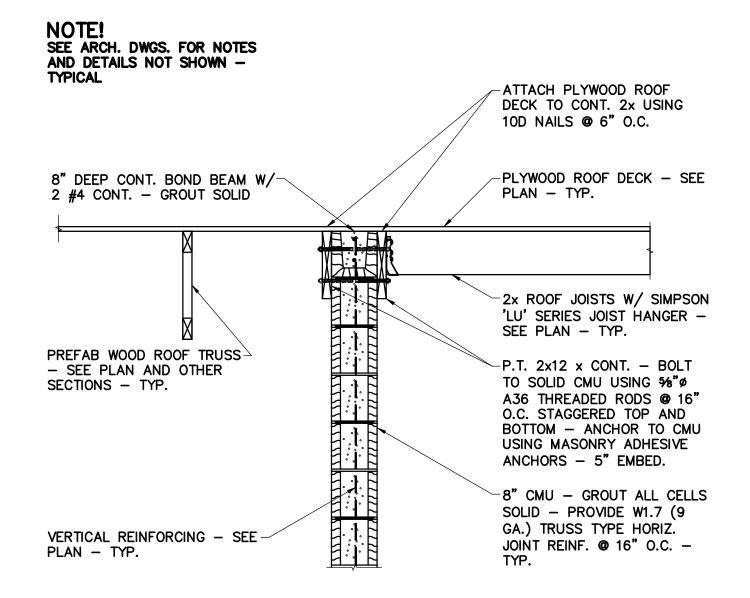




SECTION AT STAIR ROOF \$504 3/4" = 1'-0"



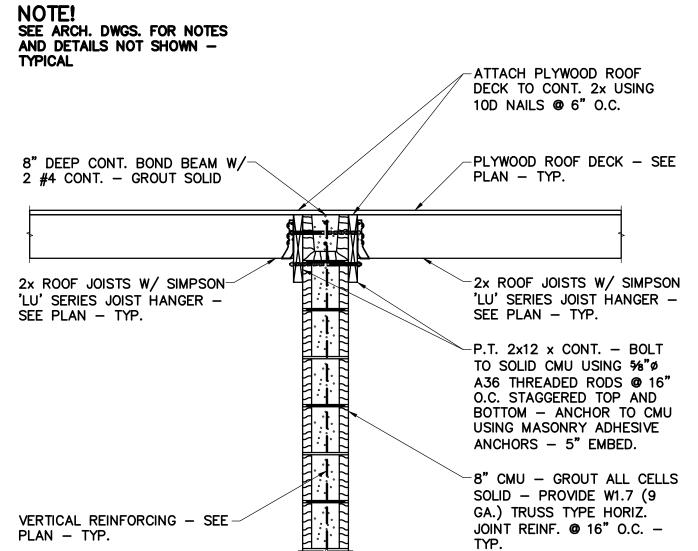
5 SECTION AT STAIR ROOF





7 SECTION AT CMU WALL

\$504 3/4" = 1'-0"





Suites

KEY PLAN

Services

Roof Framing Sections and Details

Construction Documents

MARCUS C. GIBSON License No.32774

roject No.	12-111	Sh
repared by	AB/LW	
hecked by	HLW	
^{late} Septem	ber 16, 2013	

S504

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Email: don@shultzeg.com

MECHANICAL: Shultz Engineering Group, P.C.

212 North McDowell St., Suite 204

REVISIONS

No. Date Description

1 |11.01.13 | Pool Equip.,

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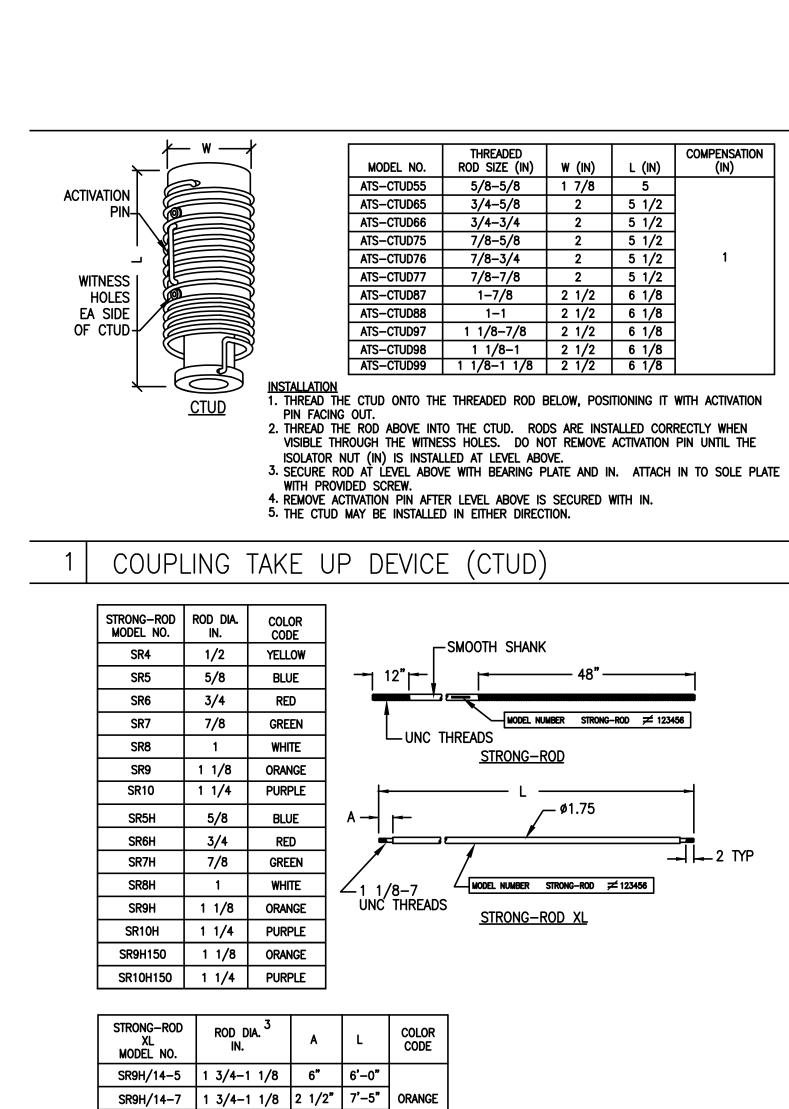
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Ftgs & Rf Slope

Phone: Fax:

6 SECTION AT CMU WALL
3/4" = 1'-0"



2. STRONG-ROD MODEL DESIGNATION: THE NUMBER FOLLOWING ATS-SR(#) REPRESENTS THE ROD DIAMETER IN 1/8" (i.e.

4. ATS-R MAY BE SUPPLIED IN PLACE OF SAME SIZE ATS-SR. ATS-HSR MAY BE SUPPLIED IN PLACE OF SAME SIZE

-1 DEGREE MAX.

PER FLOOR SEE TABLE

AT PLUMB OF ANCHOR

—1 DEGREE MAX.

PER FLOOR SEE TABLE

- G AT PLUMB OF ANCHOR

- ANCHOR BOLT AS

SPECIFIED

 3 Strong-Rod XL are 1 3/4" dia with 1 1/8" threaded ends and are used to reduce elongation.

COUPLING TAKE UP DEVICE (CTUD)

STRONG-ROD-

ISOLATOR NUT & PLATE-

OFFSET

1.625"

1.875"

ALLOWABLE ROD OFFSET

A 1 DEGREE MAXIMUM OFFSET OF

THE ROD IS PERMITTED AT EACH

4'-4" (BRIDGE BLOCK) 0.875"

ALLOWABLE HORIZONTAL OFFSET

9'-0"

10'-0" AND GREATER

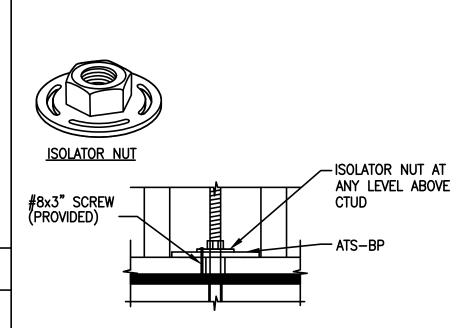
PLATE HEIGHT

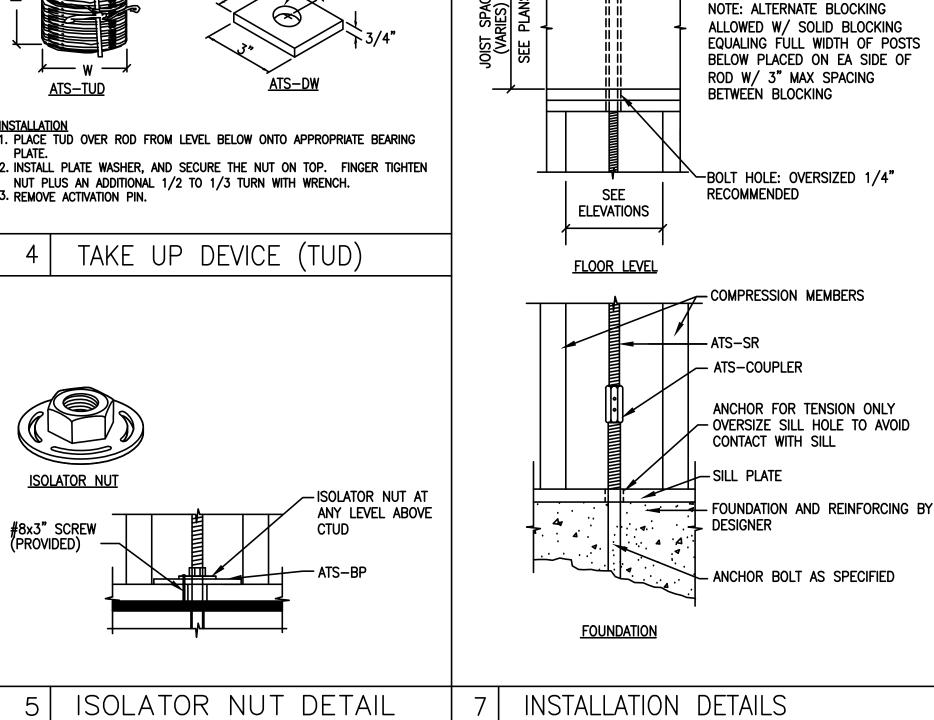
COUPLER—

SR9H150/14-7 | 1 3/4-1 1/8 | 2 1/2" | 7'-5"

1. STRONG-ROD IS UNCOATED OR PLAIN BLACK STEEL.

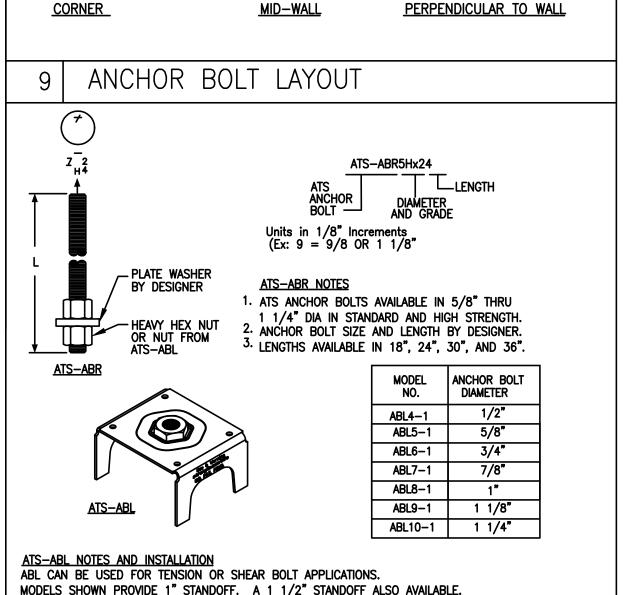
ATS-SR5 IS A 5/8" DIAMETER ROD).

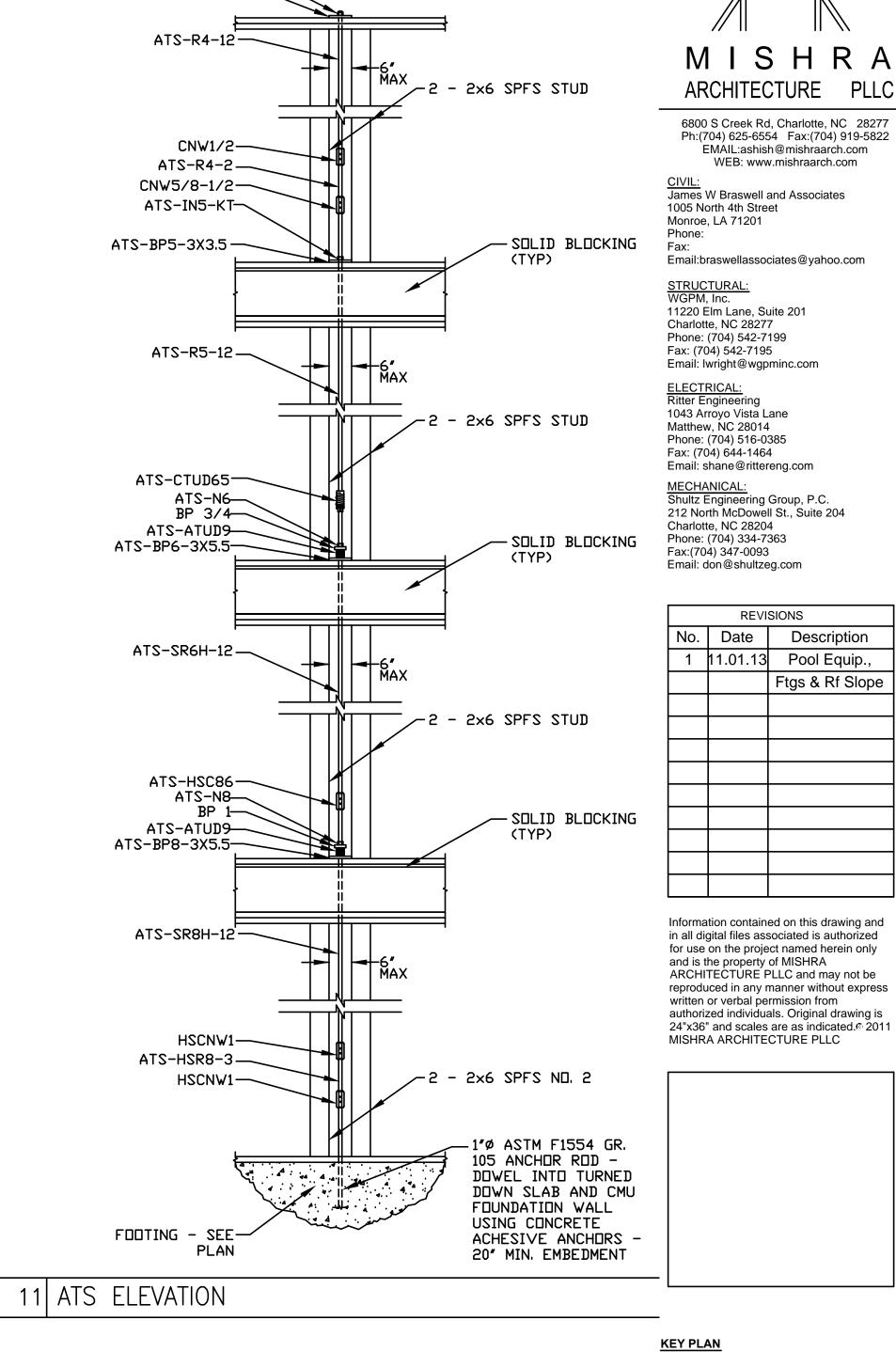




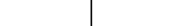
	ANCHOR BOLT LAYOUT	•		
POST SIZE	"a" W/ 6" SPACING	"a" W/ 9" SPACING		
PUST SIZE	BETWEEN STUDS	BETWEEN STUDS		
2x4 or 2x6	4 1/2"	6"		
3x4 or 3x6	5 1/2"	7"		
2-2x4 or 2-2x6	6 "	7 1/2"		
2-3x4 or 2-3x6	8"	9 1/2"		
4x6 or 6x6	8 1/2"	10"		
4x8	10 1/4"	11 3/4"		
6x8	10 1/2"	12"		
4x10	12 1/4"	13 3/4"		
6x10	12 1/2"	14"		
LOCATE ANCHOR BOLT W WHEN SHEARWALLS MEET	DO NOT INCLUDE TRIMMERS. // NARROWEST COMPRESSION I IN A CORNER AND SHARE A OF THAT SPECIFIED FOR EACH	MEMBER. A COMMON CORNER POST, TH		

MEMBER





ATS-BP4-3X3.5-



Southern Hospitality Services

> Hampton Inn and Suites

5400 I-20 & Frontage Rd. Monroe, LA 71201

Drawing Title Simpson Anchor Tiedown System (ATS)

12-111 Prepared by AB/LW Checked by HLW ate September 16, 2013

MARCUS C. GIBSON License No.32774





Construction Documents

MODELS WITH OST NUTS AVAILABLE FOR USE WITH HDG RODS.

ATTACH ABL TO FRAMEWORK WITH (2) MIN NAILS OR SCREWS.

ATTACH THE UPPER NUT AND PLATE WASHER TO THE ABR OR THREADED ROD.

THREAD THE ABR OR THREADED ROD INTO THE ABL UNTIL IT BOTTOMS OUT ON THE PLATE WASHER.

10 ANCHOR BOLTS & ANCHOR BOLT LOCATOR

ISOLATOR NUT DETAIL

		ATS PLATE DIMENSIONS				
MODEL	W (IN)	L (IN)	t (IN)	D hole (IN)	D _{rod} (IN)	CODE
ATS-BP5-3x3.5	3	3 1/2	¾	1×6	%	BLUE
ATS-BP5-3x5.5	3	5 1/2	%	¹ / ₁₆	%	BLUE
ATS-BP5-5x5.5	5	5 1/2	3⁄4	11/16	%	BLUE
ATS-BP6-3x5.5	3	5 1/2	%	13 16	3	RED
ATS-BP6-5x5.5	5	5 1/2	%	13 16	3	RED
ATS-BP7-3x5.5	3	5 1/2	%	15/16	%	GREEN
ATS-BP7-5x5.5	5	5 1/2	%	15/16	%	GREEN
ATS-BP8-3x5.5	3	5 1/2	1/2	1 16	1	WHITE
ATS-BP8-5x5.5	5	5 ½	1/2	1 16	1	WHITE
ATS-BP9-3x5.5	3	5 ½	%	1 ¾6	1 1/6	ORANGE
ATS-BP9-5x5.5	5	5 ½	%	1 ¾6	1 1/8	ORANGE
ATS-BP10-3x9	3	9	1	1 1/6	1 1/4	PURPLE
ATS-BP10-3x12	3	12	1 1/2	1 3/6	1 1/4	PURPLE
ATS-BP10-3x15	3	15	1 ¾	1 3/6	1 1/4	PURPLE
ATS-BP10-5x9	5	9	1	1 3/6	1 1/4	PURPLE
ATS-BP10-5x12	5	12	1 1/2	1 %	1 1/4	PURPLE

BEARING PLATES ARE COLOR CODED TO MATCH RODS SCHEDULE

BEARING PLATES

NAIL TO EA STUD W/ SPECIFIED EDGE NAILING BY DESIGNER X TOTAL NUMBER OF STUDS BUT NOT TO EXCEED MEMBERS ON EITHER SIDE OF STRONG ROD -SHEARWALL EDGE NAILING

NOTE: NO SPLICE IN TOP PLATE IS PERMITTED

/ ISOLATOR NUT W/ #8x3" SCREW

ATS BEARING PLATE

-BOLT HOLE OVERSIZED

1/4" RECOMMENDED

- COMPRESSION MEMBERS

- COMPRESSION MEMBERS

- ATS-TUD OR ATUD

ATS BEARING PLATE

POSTS BELOW (TYP)

-SOLID BLOCKING CONTINUOUS

BETWEEN OUTSIDE EDGES OF

TOP PLATES

- ATS-SR

AT LEVEL BELOW

NOTE: USE HEAVY HEX NUT IF CTUD NOT

WITHIN 8" OF ATS-SR

1/4" RECOMMENDED

(FLUSH MIN.)

ATS-CTUD -

ATS-CTUD

STRONG-ROD

- SOLID BLOCKING

COMPENSATION

3/4

1

1

2

L (IN)

1 3/8 | 2

2 1/8 | 2 1/4

2 1/8 | 2 1/4

2 1/8 | 3 7/8

2 3/8 | 2 1/4

- ATS-TUD

ATS-PLATE

ATS-BEARING

ATS-ATUD5

ATS-ATUD9

ATS-TUD9

ATS-ATUD9-2

ATS-TUD10 1 1/4

WASHER -

PLATE -

THREADED ROD W (IN)

SIZE (IN)

5/8

1 1/8

1 1/8

1 1/8

NOMENTCLATURE

ATS - ANCHOR TIEDOWN SYSTEM

HSCN - HIGH STRENGTH COUPLER NUT CN - COUPLER NUT HSR - HIGH STRENGTH ROD

SR8H - STRONG ROD _ HIGH STRENGTH BP - BEARING PLATE TUD - TAKE UP DEVICE HSC - HIGH STRENGTH COUPLER