

BUILDING CODE

DESIGN LIVE LOADS

CODE REFERENCE2006 INTERNATIONAL BUILDING CODE

DEAD LOADS:

3/4" GYP-CRETE 8 psf
INTERIOR PARTITIONS, ETC..... 20 psf
ROOFING AND INSULATION..... 2 psf
DECKING 2 psf
JOISTS..... 5 psf
CEILING..... 2 psf
HVAC..... 2 psf

GRAVITY LIVE LOADS:

ROOF 20 psf
GUEST ROOMS 40 psf
PUBLIC CORRIDORS 100 psf
STAIRS & LANDINGS 100 psf
BALCONIES..... 100 psf

HANDRAILS & GUARDRAILS:

HANDRAIL LOADS—250 lbs concentrated load at any point in any direction or 50 plf applied in any direction (WHICHEVER LOADING CONDITION PRODUCES THE HIGHEST STRESS WILL BE USED)

GUARDRAIL LOADS—250 lbs concentrated load at any point in any direction applied at the top of the rail, 50 plf in any direction applied horizontally at the required guardrail height w/ a simultaneous 100 plf load applied vertically downward, or a 250 lbs concentrated load at any point in any direction applied to a 1 sq. ft. area (WHICHEVER LOADING CONDITION PRODUCES THE HIGHEST STRESS WILL BE USED)

SEISMIC DESIGN DATA:

IMPORTANCE FACTOR, $I_e = 1.00$
SPECTRAL RESPONSE, $S_s = 0.181$, $S_1 = 0.081$
SITE CLASS = D
SPECTRAL RESPONSE CO-EFFICIENTS, $S_{DS} = 0.193$, $S_{D1} = 0.130$
SEISMIC DESIGN CATAGORY = b
SEISMIC FOR RESISTING SYSTEMS:
LIGHT FRAMED SHEAR WALLS w/ WOOD PANELS
SEISMIC BASE SHEAR = 17 KIPS
RESPONSE COEFFICIENT, $C_s = 0.0208$
EQUIVALENT LATERAL FORCE PROCEDURE

WIND LOAD DESIGN DATA:

WIND LOAD IMPORTANCE FACTOR (1) 1.00
BASIC WIND SPEED 90 MPH
WIND EXPOSURE CATEGORY C
WIND REFERENCE PRESSURE 22 psf
(AT TOP OF BUILDING)

GENERAL:

- SEE ARCHITECTURAL DRAWINGS FOR ANGLES, CLIPS, BARS, PLATES AND OTHER ITEMS ATTACHED TO STRUCTURAL MEMBERS.
- PROVIDE TEMPORARY BRACING AS REQUIRED TO MAINTAIN ALIGNMENT AND SECURITY OF STRUCTURES DURING CONSTRUCTION.
- DO NO CUTTING, DRILLING, OR MODIFYING OF STRUCTURAL MEMBERS WITHOUT THE APPROVAL OF THE ARCHITECT.
- THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS BEFORE STARTING WORK. THE ARCHITECT SHALL BE NOTIFIED OF ANY DISCREPANCY.
- THE DESIGN AGENCY AND SAFETY OF ERECTION BRACING, SHORING, ETC., IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- THE CONTRACTOR SHALL COORDINATE THE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS WITH THE STRUCTURAL DRAWINGS.
- CONTRACTOR TO VERIFY ALL WALL, COLUMN, AND SLAB LOCATIONS, THICKNESS, AND DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
- THE GENERAL CONTRACTOR SHALL COORDINATE THE PLACEMENT OF FOOTINGS, COLUMNS, SLAB, WALLS, SHAFTS, ETC., WITH ALL SUBCONTRACTORS INVOLVED.
- FOOTINGS ARE DESIGNED FOR A SOILS BEARING PRESSURE OF **2000 psf** AT THE BOTTOM OF THE FOOTINGS (24" MIN. TO BOTTOM FTG BELOW EXISTING GRADE). ALL FOOTINGS SHALL BE BELOW THE FROST DEPTH.
- ALL FOUNDATION & SOILS WORK SHALL BE DONE IN ACCORDANCE WITH THE PROJECT SOILS REPORT PREPARED BY **ARDAMAN & ASSOCIATES, INC.** DATED **1-27-10**.
- VERIFY ALL OPENING SIZES AND LOCATIONS ON THE STRUCTURAL DRAWINGS w/ THE MECHANICAL DRAWINGS.

CONCRETE & MASONRY:

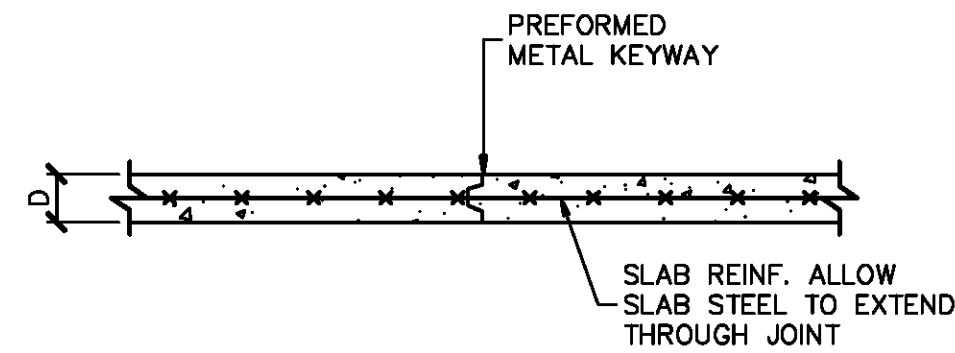
- CONCRETE STRENGTH TO BE A MINIMUM OF 3500 PSI AT 28 DAYS (SEE SPEC) (SEE NOTE #3 CONCERNING TESTING OF CONCRETE)
- CONCRETE EXPOSED TO WEATHER TO BE AIR - ENTRAINED (8% MAX. - 3% MIN.)
- CONCRETE AND REINFORCING STEEL TO BE AS PER LATEST ACI 318 AND ACI 301. TESTING OF CONCRETE TO BE IN ACCORDANCE w/ ACI 301 BY AN INDEPENDENT TESTING AGENCY AT CONTRACTORS EXPENSE.
- PROVIDE SHOP DRAWINGS DETAILING REINFORCING STEEL PRIOR TO FABRICATION OF SAME. (INCLUDE IN SUBMITTALS ELEVATIONS OF ALL REINFORCED WALLS AND COLUMNS UNLESS OTHERWISE NOTED). DETAILING SHALL BE ACCOMPLISHED BY AN EXPERIENCED DETAILER AND ACCORDING TO ACI 318-05.
- ALL SLAB ON GRADE TO BE 4" CONCRETE w/ 6x6 W2.1/W2.1 ON 4" GRANULAR FILL OR APPROVED FILL RECOMMENDED BY GEOTECHNICAL ENGINEER OF RECORD, UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR ANY DEPRESSED AREAS, VAPOR BARRIERS, ETC..
- THE MINIMUM CONCRETE COVER SHALL BE IN ACCORDANCE WITH A.C.I. 318.
- ALL HOLLOW CONCRETE MASONRY UNITS TO MEET A.S.T.M. SPECIFICATIONS C90, GRADE N, TYPE 1, WITH MINIMUM ULTIMATE COMPRESSIVE PRISM STRENGTH (f'm) OF 1,500 PSI.
- ALL MORTAR SHALL MEET A.S.T.M. SPECIFICATIONS FOR TYPE "S" MORTAR EXCEPT AS SHOWN OTHERWISE WITH A MINIMUM COMPRESSIVE STRENGTH OF 1,800 P.S.I. ADDITIVES CONTAINING CALCIUM CHLORIDE SHALL NOT BE USED.
- HORIZONTAL JOINT REINFORCING SHALL BE XTRA HEAVY AT 8" C.C. (3/16"Ø LONGT. WIRES) EXCEPT AS SHOWN OTHERWISE.
- ALL MASONRY CORNERS SHALL HAVE 3 VOIDS REINFORCED w/ (1) #5 EACH VOID AND GROUTED.
- ALL CELLS WHERE REINFORCING IS SPECIFIED SHALL BE FILLED w/ CONCRETE GROUT.
- REINFORCING SHALL BE A-615 GRADE 60 EXCEPT #3 BARS SHALL BE GRADE 40 IN ACCORDANCE WITH LATEST A.S.T.M. SPECIFICATIONS.
- REINFORCING IN ALL CONCRETE FOOTING AND WALLS SHALL BE CONTINUOUS AROUND CORNERS.
- LAP ALL STEEL 36 BAR DIAMETER OR 18" MINIMUM AT SPLICES AND CORNERS.
- GROUT SHALL CONFORM TO A.S.T.M. 476-76, 3000 PSI STRENGTH.
- ALL BLOCK CELLS BELOW GRADE SHALL BE FILLED SOLID WITH CONCRETE OR GROUT.
- CONSTRUCTION OF LOAD-BEARING CONCRETE MASONRY SHALL CONFORM TO SPECIFICATIONS BY THE NATIONAL CONCRETE MASONRY ASSOCIATION AND A.C.I. 531-79 (REVISED 1983).
- PROVIDE 2 #5 BARS EXTRA E.S. OF ALL OPENINGS IN CONCRETE SLABS U.N.O.
- PROVIDE 8"x16" U-BLOCK LINTEL w/ 2 #6 BOT. OVER ALL OPENINGS IN MASONRY WALLS U.N.O. BEAR LINTELS 16" MIN E.E.
- PROVIDE 1 #5 EXTRA VERTICAL BAR EACH SIDE OF OPENINGS IN MASONRY WALLS U.N.O.

STRUCTURAL STEEL:

- STRUCTURAL STEEL SHALL CONFORM TO ASTM SPECIFICATION A-992 (Fy=50ksi).
- STRUCTURAL STEEL SHALL BE DETAILED IN ACCORDANCE WITH STANDARD PRACTICES OF A.I.S.C.
- STRUCTURAL STEEL TO BE DESIGNED, FABRICATED, ERECTED, ETC., AS PER AISC MANUAL OF STEEL CONSTRUCTION, LATEST EDITION.
- SUBMIT SHOP DRAWINGS OF STRUCTURAL STEEL PRIOR TO FABRICATION OF SAME.
- VERIFY w/ ARCHITECT, MECHANICAL, AND ELECTRICAL DRAWINGS FOR STAIR DETAILS, RAILINGS, ANGLES, ETC., NOT SHOWN ON STRUCTURAL DRAWINGS.
- SEE ARCHITECTURAL DRAWINGS FOR MATERIAL AND TREADS.
- PROVIDE ALL NECESSARY CLIP ANGLES, BOLTS, HANDRAILS, ETC., TO COMPLETE STAIR PORTION OF PROJECT.
- STEEL TUBES SHALL BE ASTM A500 GRADE B.

WOOD NOTES:

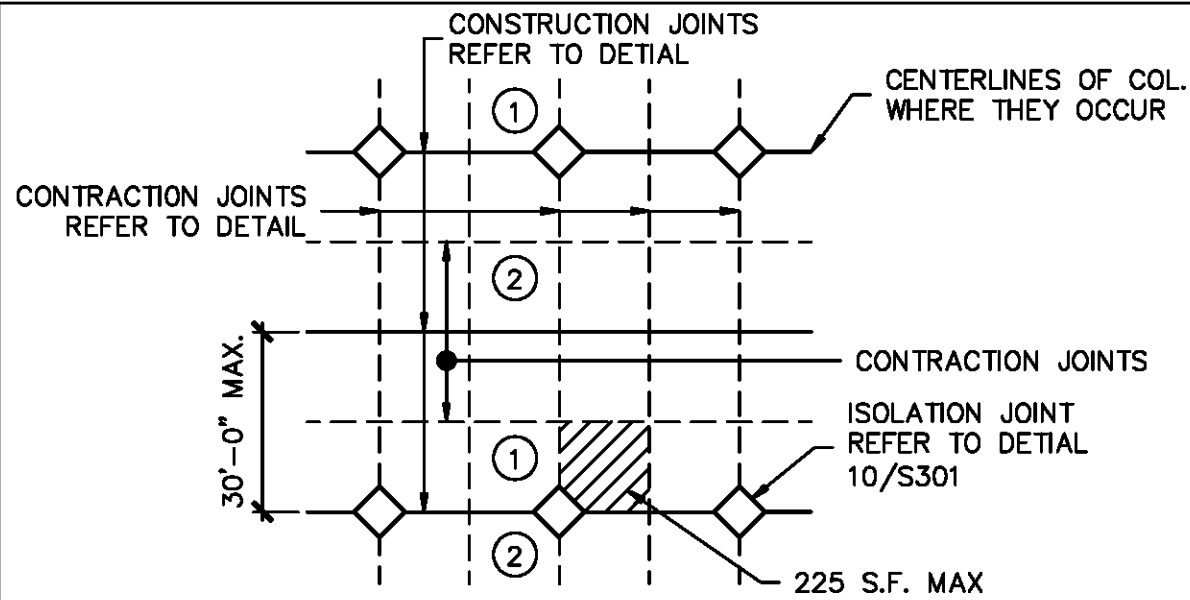
- STRESS GRADE SAWED LUMBER SHALL CONFORM TO SPECIFICATIONS FOR KILN DRIED NO. 2 SOUTHERN PINE WITH ALLOWABLE BENDING STRESS OF 1500 psi. MODULUS OF ELASTICITY= 1600 ksi (MINIMUM).
- USE JOIST HANGERS AND FRAMING ANCHORS, 18 ga. MINIMUM, GALVANIZED, SIZED FOR FULL LOAD CAPACITY OF SUPPORTED MEMBERS.
- PROVIDE DOUBLE STUDS (MINIMUM) EACH SIDE OF ALL WALL OPENINGS.
- PROVIDE SOLID BLOCKING IN WOOD FLOOR CONSTRUCTION UNDER POSTS, COLUMNS, AND MULTIPLE STUDS.
- PROVIDE BRIDGING IN FLOOR AND ROOF CONSTRUCTION PER APPLICABLE CODES. MINIMUM= 1 ROW OF BRIDGING AT MID SPAN.
- PROVIDE CONTINUOUS HORIZONTAL BLOCKING IN ALL STUD WALLS PER APPLICABLE CODES. MINIMUM= 1 ROW OF BLOCKING AT MID HEIGHT.
- CONSTRUCT WOOD CONNECTIONS PER RECOMMENDATIONS OF THE NATIONAL FOREST PRODUCTS ASSOCIATION.
- USE THE LATEST EDITION OF THE IBC FOR MINIMUM ACCEPTABLE NAILING (FASTENER SCHEDULE).
- WHERE PLYWOOD FILLERS ARE USED WITH LINTELS OR BEAMS, THEY SHALL BE CONTINUOUS FOR THE FULL LENGTH OF THE BEAM OR LINTEL, AND THEY SHALL BE GLUED AND NAILED TO 2x's WITH 2 ROWS OF 10d NAILS AT 12" O.C. MINIMUM.
- MICROLAM AND PARALLAM LUMBER SHALL CONFORM TO MACMILLAN SPECIFICATIONS, OR EQUAL.
- FLOOR AND ROOF TRUSSES SHALL BE DESIGNED BY A REGISTERED ENGINEER FOR THE MAXIMUM LOAD APPLICABLE PER LOCAL AND NATIONAL CODES. L/480 SHALL BE THE MAXIMUM TOTAL LOAD DEFLECTION AT MID-SPAN FOR ROOF AND FLOOR JOISTS. SUBMIT CALCULATIONS AND SHOP DRAWINGS TO THE ARCHITECT FOR APPROVAL PRIOR TO FABRICATING.
- VERSALAM BEAMS SHALL CONFORM TO THE FOLLOWING SPECIFICATIONS:
Fb=2,900 PSI MINIMUM BENDING STRESS; E=2,000,000 PSI; Fv=280 PSI;
Fc. PARALLEL = 3,000 PSI
- FLOOR JOISTS TO BE EQUAL TO 11 1/8" T/J-L6S JOISTS AS MANUFACTURED BY WEYERHAEUSER OR EQUAL.
- ALL EXTERIOR WOOD TO BE PRESSURE TREATED LUMBER, TYP.



3 CONSTRUCTION JOINT IN SLAB ON GRADE

SCALE: 3/4"=1'-0"

DET069



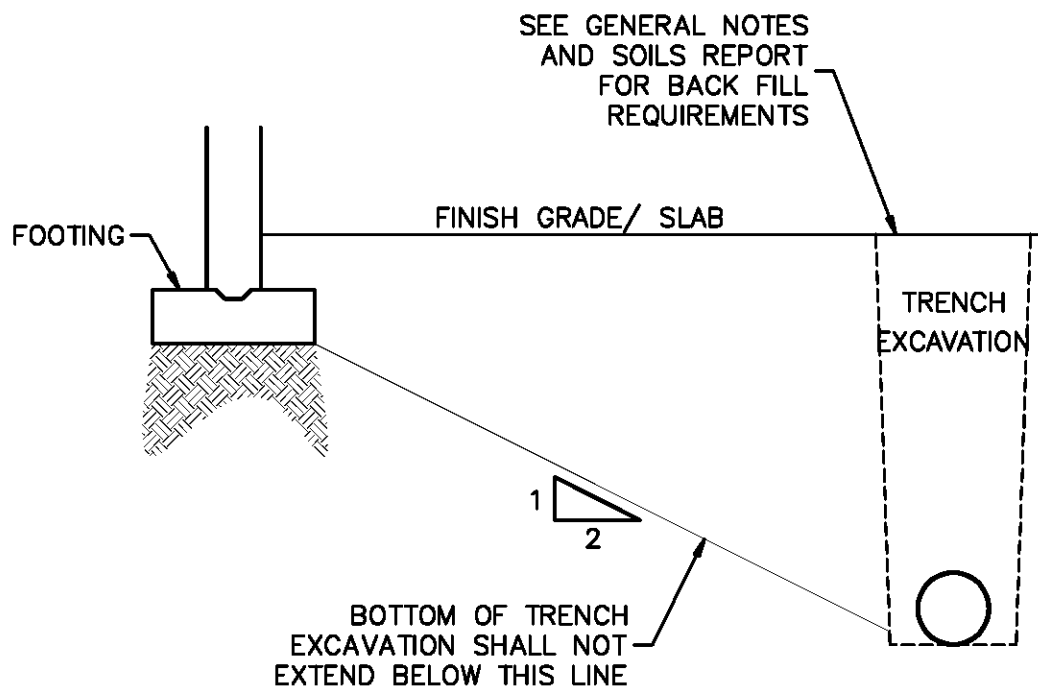
NOTES:

- SLAB SHALL BE POURED IN STRIP PATTERN.
①=FIRST POUR ②=SECOND POUR
- STRIPS TO BE DIVIDED BY CONSTRUCTION JOINTS AT THE CENTERLINE OF COLS. AND SUBDIVIDED AS REQ'D WITH CONTRACTION JOINTS SPACED AT INTERVALS NOT EXCEEDING 15' IN EITHER DIRECTION.
- IN AREAS WHERE COLUMNS DO NOT OCCUR PROVIDE CONSTRUCTION AND/OR CONTRACTION JOINTS AS SHOWN ABOVE.
- CONTRACTOR CAN SUBMIT A PLAN WITH SAW CUT JOINTS IN BOTH DIRECTIONS FOR APPROVAL. MAX AREA BETWEEN JOINTS SHALL BE 225 S.F.

1 SLAB ON GRADE POUR DETAIL

SCALE: N.T.S.

DET067



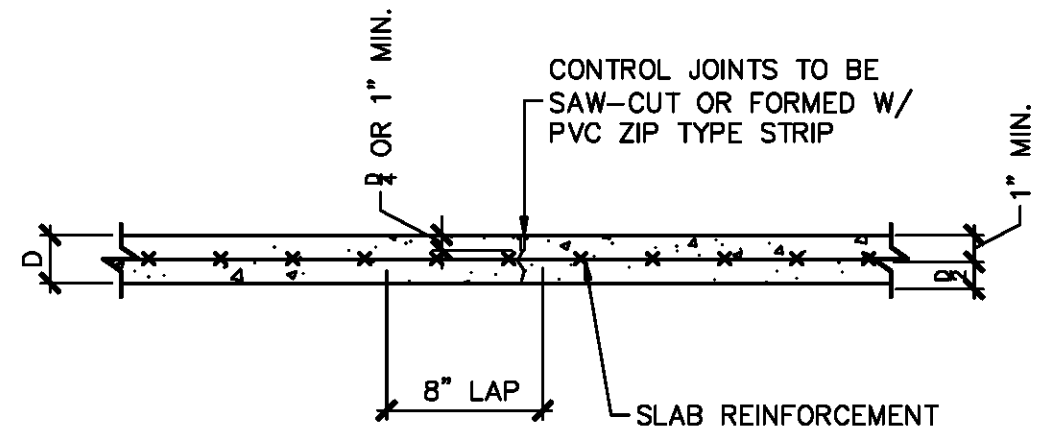
NOTES:

- THE CONTRACTOR SHALL COORDINATE ALL EXCAVATION OPERATIONS WITH BUILDING FOUNDATION REQUIREMENTS.
- SLOPE MAY BE ALTERED TO 1:1 IF CONDITIONS ALLOW AND APPROVED BY ARCHITECT/ENGINEER.

4 EXCAVATION PARALLEL TO FOOTING

SCALE: 1/2"=1'-0"

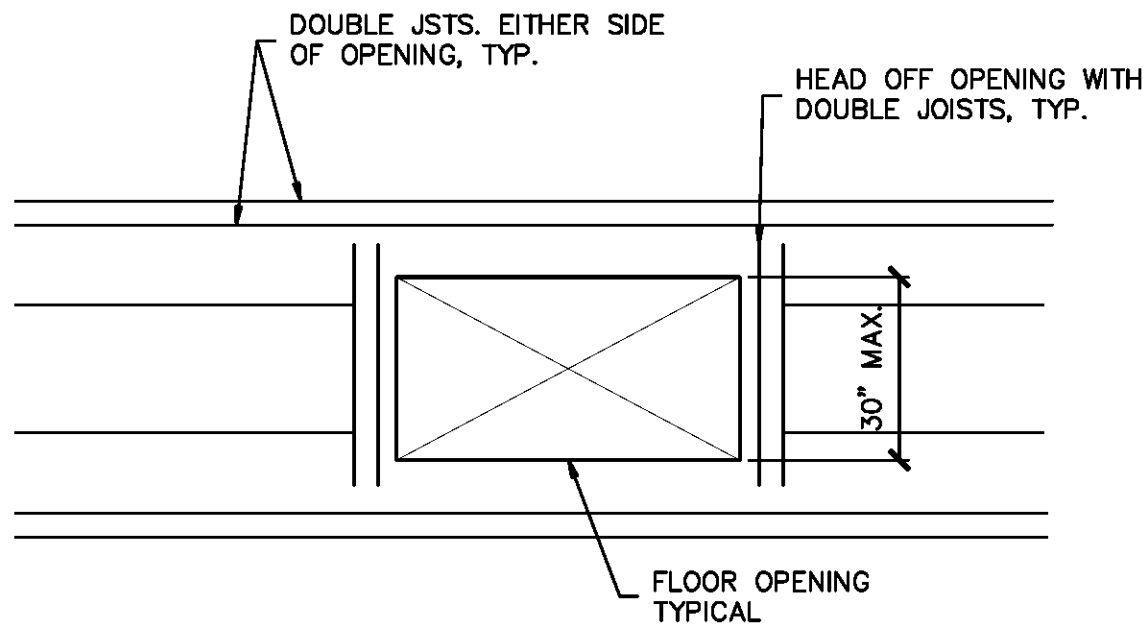
DET070



2 CONTROL JOINT IN SLAB ON GRADE

SCALE: 3/4"=1'-0"

DET068

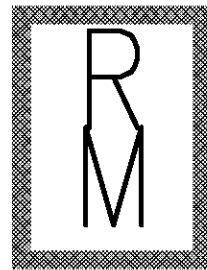


5 TYPICAL FLOOR PENETRATION DETAIL

SCALE: 1/2"=1'-0"

DET061

COLUMN SCHEDULE				
	C1	C2	C3	C4
ROOF EL=131.25'			HSS 4x4x1/4 Fy = 46 ksi	
3rd FLR. EL=122.15'			HSS 4x4x1/4 Fy = 46 ksi	
2nd FLR. EL=111.91'	HSS 6x6x1/2 Fy = 46 ksi 12"x1"x12" PL. w/ (4) 5/8" x 2'-0" A.B.	HSS 8x8x3/8 Fy = 46 ksi 14"x1 1/2"x14" PL. w/ (4) 1 1/8" x 2'-0" A.B.	HSS 4x4x1/4 Fy = 46 ksi 10"x3/4"x10" PL. w/ (4) 3/8" x 1'-6" A.B.	HSS 4x4x1/4 Fy = 46 ksi 10"x3/4"x10" PL. w/ (4) 3/8" x 1'-6" A.B.
GRND. FLR. EL=100.00'				



Richard Molenaar

ARCHITECT



PROJECT NAME:

Hampton Inn
Main Street
West Monroe, Louisiana

SHEET TITLE:

GENERAL NOTES
& DETAILS

PROJECT NO.

2010046

LATEST REVISION:

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