GENERAL NOTES:

COORDINATION:

1. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH AND COORDINATED WITH ARCHITECTURAL DRAWINGS AND OTHER CONTRACT DOCUMENTS.

2. THE PROJECT ARCHITECT SHALL BE RESPONSIBLE FOR REVIEWING/COORDINATING ALL DIMENSIONS, ELEVATIONS AND DETAILS SHOWN ON THE STRUCTURAL DRAWINGS WITH THE ARCHITECTURAL DRAWINGS.

3. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING ALL OF THE CONTRACT DOCUMENTS AND LATEST ADDENDA AND FOR SUBMITTING SUCH DOCUMENTS TO SUBCONTRACTORS AND MATERIAL SUPPLIERS PRIOR TO THE SUBMITTAL OF SHOP DRAWINGS, FABRICATION OF ANY STRUCTURAL MEMBERS, AND ERECTION IN THE FIELD. THE GENERAL CONTRACTOR SHALL COMPARE THE STRUCTURAL DRAWINGS AND OTHER CONTRACT DRAWINGS AND REPORT ANY DISCREPANCY BETWEEN AND WITHIN EACH SET OF DRAWINGS WITH THE PROJECT ARCHITECT AND THE STRUCTURAL ENGINEER PRIOR TO THE FABRICATION AND INSTALLATION OF ANY STRUCTURAL MEMBERS.

4. DRAWINGS SHOW GENERAL AND TYPICAL SECTIONS/DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY SHOWN, SIMILAR SECTIONS/DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO THE APPROVAL OF THE ENGINEER.

5. THE STRUCTURAL MEMBERS OF THIS PROJECT HAVE BEEN DESIGNED BY THE STRUCTURAL ENGINEER TO RESIST THE REQUIRED CODE GRAVITY AND LATERAL FORCES THAT COULD OCCUR IN THE FINAL COMPLETED STRUCTURE ONLY. THE GENERAL CONTRACTOR IS RESPONSIBLE FOR PROVIDING ALL REQUIRED BRACING/SHORING DURING CONSTRUCTION TO MAINTAIN THE STABILITY AND SAFETY OF ALL STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PROCESS UNTIL THE STRUCTURE IS TIED TOGETHER AND COMPLETED.

6. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR THE METHODS, TECHNIQUES AND SEQUENCES OF PROCEDURES TO PERFORM THE WORK. THE SUPERVISION OF THE WORK IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.

7. LOADS APPLIED TO THE STRUCTURE DURING CONSTRUCTION SHALL NOT EXCEED THE SAFE LOAD-CARRYING CAPACITY OF THE STRUCTURAL MEMBERS. THE LIVE LOADS USED FOR THE DESIGN OF THE STRUCTURE ARE INDICATED IN THE GENERAL NOTES. DO NOT APPLY ANY CONSTRUCTION LOADS UNTIL STRUCTURAL FRAMING IS PROPERLY INSTALLED AND ALL TEMPORARY BRACING IS IN PLACE.

8. ALL ASTM AND OTHER REFERENCES ARE PER THE LATEST EDITIONS UNLESS NOTED OTHERWISE.

9. EQUIPMENT PADS SHALL BE PROVIDED BY THE MECHANICAL, ELECTRICAL, OR PLUMBING CONTRACTORS REQUIRING THE PAD.

10. COORDINATE THE EXACT SIZE AND LOCATION OF ALL SLEEVES AND OPENINGS THROUGH CONCRETE WALLS, CONCRETE SLABS OR MASONRY WALLS WITH ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS.

11. SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. CONTRACTOR SHALL REVIEW, APPROVE AND SIGN EACH SHEET PRIOR TO SUBMISSION. THE STRUCTURAL ENGINEER'S REVIEW SHALL BE FOR CONFORMANCE WITH THE DESIGN CONCEPT AND GENERAL COMPLIANCE WITH THE CONTRACT DOCUMENTS. THE ENGINEER'S REVIEW DOES NOT RELIEVE THE CONTRACTOR OF THE SOLE RESPONSIBILITY TO REVIEW, CHECK AND COORDINATE THE SHOP DRAWINGS PRIOR TO SUBMISSION. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF THE SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS, DIMENSIONS, ETC. CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS. SUBMIT PDF FILES FOR REVIEW.

12. CONTRACTOR SHALL VISIT THE SITE PRIOR TO BID TO ASCERTAIN CONDITIONS WHICH MAY ADVERSELY AFFECT THE WORK OR COST THEREOF.

13. WHERE CONFLICTS OCCUR BETWEEN GENERAL NOTES, STRUCTURAL DRAWINGS AND SPECIFICATIONS THE MOST STRINGENT REQUIREMENT SHALL APPLY.

14. THE GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR COMPLYING WITH ALL SAFETY PRECAUTIONS AND REGULATIONS DURING THE WORK. THE ENGINEER WILL NOT ADVISE NOR ISSUE DIRECTION AS TO SAFETY PRECAUTIONS AND PROGRAMS.

FOUNDATIONS:

1. FOUNDATION DESIGN OF FOOTINGS BASED ON SOIL REPORT BY GEOTECHNICAL ASSOCIATES NETWORK, LLC, DATED NOVEMBER 2012. ALLOWABLE SOIL BEARING PRESSURE 7,000 PSF USING AGGREGATE PIERS.

2. NO UNBALANCED BACKFILLING SHALL BE DONE AGAINST BASEMENT WALLS UNLESS WALLS ARE BRACED BY TEMPORARY BRACING OR BY PERMANENT CONSTRUCTION.

3. FOUNDATION WALLS WITH BACKFILL ON EACH SIDE SHALL BE BACKFILLED EVENLY ON EACH SIDE. THESE WALLS HAVE NOT BEEN DESIGNED FOR UNBALANCED SOIL LOADS. ALL BASEMENT WALLS (FOUNDATION WALLS WITH DOWELS INTO SLAB ON GRADE) SHALL BE SHORED UNTIL SLAB ON GRADE REACHES 75 PERCENT OF THE 28 DAY COMPRESSIVE STRENGTH. WATERPROOF BACKSIDE OF ALL FOUNDATION WALLS UNLESS NOTED OTHERWISE.

4. COORDINATE FOUNDATION WORK WITH EXISTING UTILITIES. FOUNDATIONS SHALL BE LOWERED WHERE REQUIRED TO AVOID UTILITIES. NOTIFY PROJECT ARCHITECT AND STRUCTURAL ENGINEER TO PROVIDE REINFORCED CONCRETE PIER FOR COLUMN FOOTINGS

5. UNLESS NOTED OTHERWISE COLUMN CENTERLINES SHALL BE CENTERLINES OF COLUMN FOOTINGS.

6. HEAVY GRADING EQUIPMENT SHALL NOT BE ALLOWED WITHIN THE HEIGHT OF THE WALL (HORIZONTALLY) OF BASEMENT OR CANTILEVER RETAINING WALLS.

SLAB ON GRADE:

1. CONTROL JOINTS FOR SLAB ON GRADE SHALL BE LOCATED AS SHOWN ON PLAN, WITH A MAXIMUM JOINT SPACING OF 2 1/2 TIMES THE SLAB THICKNESS IN FEET. JOINTS SHALL BE FORMED USING SAW CUTS 1/8" WIDE (MAXIMUM) BY T/4 (1 1/4" MIN.) DEEP. SAW CUT AS SOON AS PRACTICAL AND WITHIN 12 HOURS AFTER PLACING CONCRETE. JOINTS SHALL BE FILLED WITH SEMI-RIGID EPOXY JOINT FILLER (CONSPEC POLUREA JOINTFILL (OR EQUIVALENT).

2. SIDEWALKS AND OTHER EXTERIOR SLABS ARE NOT SHOWN ON THE STRUCTURAL DRAWINGS. SEE ARCHITECTURAL, SITE AND CIVIL DRAWINGS FOR LOCATIONS, DIMENSIONS AND ELEVATIONS.

3. SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATION OF DEPRESSED SLAB AREAS AND DRAINS. FLOOR SLABS SHALL SLOPE TO ALL FLOOR DRAINS. GENERAL CONTRACTOR SHALL COORDINATE WITH PLUMBING DRAWINGS - TYPICAL.

4. USE EVAPORATION RETARDER ONE OR MORE TIMES AFTER THE STRIKEOFF WHEN HIGH TEMPERATURES, LOW HUMIDITY AND/OR WIND WILL CAUSE CRUSTING AND PLASTIC CRACKING.

5. EXPOSED FLOOR SLAB AREAS ARE TO RECEIVE 2 - COATS OF 25% MIN. SOLIDS ACRYLIC HARDENER AND SEAL (SPEC CHEM'S CURE AND SEAL WB 25 OR SPEC CHEM'S CURE AND SEAL 25UV, OR EQUIVALENT). APPLICATION IS TO CONFORM TO MANUFACTURER'S SPECIFICATIONS. FIRST COAT IS FOR CURING, SECOND COAT IS FOR SEALING AND DUST PROOFING AFTER BUILDING CONSTRUCTION COMPLETION. FIRST COAT ONLY REQUIRED WHERE SLAB IS RECEIVING FLOOR COVERING.

6. SEE GEOTECHNICAL REPORT/GEOTECHNICAL ENGINEER FOR VAPOR RETARDER AND UNDERSLAB DRAINAGE FILL REQUIREMENTS. VAPOR RETARDER SHALL BE 10 MIL AND MEET ASTM E 1745 CLASS A INSTALLED PER ASTM E 1643 WITH LAPPED JOINTS NOT LESS THAN 6 INCHES.

CONCRETE:

1. CONCRETE SHALL BE PROPORTIONED TO MEET THE REQUIREMENTS OF THE FOLLOWING:

ELEMENT	28-DAY STRENGTH (PSI)	SLUMP RANGE (IN.)	UNIT WEIGHT (PCF)
COLUMN FOOTINGS	3000	3-5	150
WALL FOOTINGS	3000	3-5	150
STRUCTURAL SLAB ON GRADE	4000	3-4	150
RETAINING WALLS	3000	3-5	150
GRADE BEAMS	3000	3-5	150

2. PORTLAND CEMENT SHALL BE ASTM C 150, TYPE I. FLY ASH SHALL CONFORM TO ASTM C 618, CLASS F AND SHALL NOT EXCEED 25% OF CEMENT CONTENT BY WEIGHT. SLAG SHALL CONFORM TO ASTM C 989.

3. NORMAL WEIGHT AGGREGATE SHALL CONFORM TO ASTM C 33. CONCRETE AGGREGATE GRADATION SHALL BE IN ACCORDANCE WITH ASTM C33 SPECIFICATION. "SPECIFICATION FOR CONCRETE AGGREGATE". FINE AGGREGATE SHALL CONSIST OF NATURAL SAND OR A COMBINATION THEREOF, WITH A FINENESS MODULUS BETWEEN 2.3 AND 3.1. LARGER COURSE AGGREGATE MIXES UP TO #67 ARE ACCEPTABLE FOR FLOOR SLAB CONCRETE TO MINIMIZE SHRINKAGE CRACKING.

4. FLY ASH AND/OR SLAG SHALL NOT BE PERMITTED IN CONCRETE PLACED SUBJECT TO COLD WEATHER PLACEMENT PROCEDURES.

5. CONCRETE EXCEEDING THE SPECIFIED SLUMP RANGES SHALL BE RETURNED. DO NOT ADD WATER TO THE CONCRETE MIX AT THE JOB SITE WITHOUT THE WRITTEN PERMISSION FROM THE STRUCTURAL ENGINEER.

6. ALL REINFORCING STEEL SHALL BE ASTM A615 GRADE 60 UNLESS NOTED OTHERWISE. ALL WELDED WIRE FABRIC (W.W.F.) SHALL BE ASTM A82 AND A185 COLD DRAWN STEEL WIRE. W.W.F. SHALL BE DELIVERED TO THE JOB SITE IN FLAT SHEETS (NO ROLLS). PLACE SHEETS ON BOLSTERS AT 36" MAXIMUM TO LOCATE IN UPPER THIRD OF SLAB. LAP CONTINUOUS REINFORCING BARS 36 BAR DIAMETERS UNLESS NOTED OTHERWISE. PROVIDE CORNER BARS IN ALL WALLS AND FOOTINGS. BAR SUPPORTS, DESIGN, DETAILING, FABRICATION, AND PLACING OF REINFORCING BARS SHALL BE IN ACCORDANCE WITH THE ACI CODE AND DETAILING MANUAL AND CRSI'S "MANUAL OF STANDARD PRACTICE".

7. MINIMUM CONCRETE PROTECTION FOR REINFORCEMENT SHALL BE:

CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH..................3"
CONCRETE EXPOSED TO EARTH OR WEATHER:
No. 6 THROUGH No. 18 BARS...2"
No. 5 AND SMALLER...1 1/2"
CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
SLABS, WALLS AND JOISTS:
No. 14 AND No. 18 BARS..1 1/2"
No. 11 AND SMALLER..3/4"
BEAMS AND COLUMNS:
PRIMARY REINFORCEMENT, TIES, STIRRUPS AND SPIRALS......................1 1/2"

8. ANCHOR RODS FOR COLUMNS SHALL BE POSITIONED WITH A TEMPLATE PRIOR TO PLACING CONCRETE IN PIER OR FOOTING. NUTS SHALL BE TIGHTENED ON EACH SIDE OF THE TEMPLATE TO HOLD THE ANCHOR BOLTS IN PLACE.

9. CONCRETE DESIGN AND REINFORCEMENT SHALL BE IN ACCORDANCE WITH THE "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" (CODE REFERENCED ACI 318) AND WITH "DETAILS AND DETAILING OF CONCRETE REINFORCEMENT" (ACI 315-92). CONCRETE PLACED DURING HOT WEATHER AND COLD WEATHER SHALL MEET THE RECOMMENDATIONS OF ACI/PCA/TCA. CONCRETE SHALL BE SAMPLED AND TESTED BY AN INDEPENDENT TESTING AGENCY IN ACCORDANCE WITH ACI 318.

10. CONCRETE MIXES SHALL BE DESIGNED IN ACCORDANCE WITH ACI 301. WATER SHALL NOT BE ADDED TO THE CONCRETE MIX AT THE JOB SITE WITHOUT THE PRIOR WRITTEN PERMISSION OF THE STRUCTURAL ENGINEER.

11. UNLESS OTHERWISE SHOWN ON ARCHITECTURAL DRAWINGS, PROVIDE 3/4" CHAMFER AT ALL COLUMN, WALL SLAB AND BEAM EDGES THAT ARE EXPOSED TO VIEW IN THE FINAL STRUCTURE.

12. PROVIDE VERTICAL CONTROL OR CONTRACTION JOINTS AT 25' MAXIMUM IN ALL CONCRETE BASEMENT WALLS, RETAINING WALLS, OR SCREENWALLS. PROVIDE VERTICAL EXPANSION JOINTS AT 100' MAXIMUM IN THE LINEAR PLANE OF THE WALL. SEE ARCHITECTURAL DRAWINGS FOR LOCATIONS IN AESTHETIC WALLS.

13. FIBER REINFORCEMENT - POLYPROPYLENE FIBRILLATED FIBERS USE AT 1.5 POUNDS PER CUBIC YARD WITH A MINIMUM AVERAGE RESIDUAL STRENGTH OF 45 PSI IN ACCORDANCE WITH ASTM 1399 TESTING - FIBERMESH 300 OR EQUIVALENT.

14. SLAB ON GRADE SHALL HAVE AN OVERALL FLOOR FLATNESS (FF) OF 25 WITH A MINIMUM LOCAL VALUE OF 17 AND AN OVERALL FLOOR LEVELNESS (FL) OF 20 WITH A MINIMUM LOCAL VALUE OF 15. ELEVATED SLABS SHALL HAVE AN OVERALL FLOOR FLATNESS (FF) OF 25 WITH A MINIMUM LOCAL VALUE OF 17.

MASONRY:

1. MASONRY CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF THE "SPECIFICATION FOR MASONRY STRUCTURES (CODE REFERENCED ACI 530.1)" AND NCMA SPECIFICATION TEK NOTES AND BIA TECHNICAL NOTES ON BRICK CONSTRUCTION. CONTINUOUS INSPECTION SHALL BE PERFORMED BY AN INDEPENDENT TESTING AGENCY FOR GENERAL COMPLIANCE WITH THE CONTRACT DOCUMENTS.

2. ALL HOLLOW CONCRETE MASONRY UNITS SHALL CONFORM TO ASTM C-90, LIGHTWEIGHT. MINIMUM NET COMPRESSIVE STRENGTH (f'm) SHALL BE 2,000 PSI. FILL BLOCK CELLS WHERE REBAR OCCURS SOLID WITH GROUT. SUBMIT VERIFICATION OF ALL MATERIALS TO ARCHITECT FOR APPROVAL.

3. ALL BRICK UNIT MASONRY SHALL CONFORM TO ASTM C-216, GRADE SW, TYPE FBS. MINIMUM NET COMPRESSIVE STRENGTH (f'm) SHALL BE 3,000 PSI. VERIFICATION OF ALL MATERIALS TO ARCHITECT FOR APPROVAL.

4. MORTAR SHALL BE PORTLAND CEMENT-LIME MIX (PORTLAND CEMENT SHALL COMPLY WITH ASTM C 150, TYPE I OR III, AND HYDRATED LIME COMPLYING WITH ASTM C 270) OR MORTAR CEMENT ASTM C 1329 - TYPE S, THE USE OF MASONRY-CEMENT IS STRICTLY FORBIDDEN. AGGREGATE FOR MORTAR SHALL COMPLY WITH ASTM C 144.

5. GROUT FOR UNIT MASONRY SHALL COMPLY WITH ASTM C 476 (SLUMP 8 TO 11 INCHES). AGGREGATE FOR GROUT SHALL COMPLY WITH ASTM C404. COMPRESSIVE STRENGTH SHALL BE GREATER THAN OR EQUAL TO 2,000 PSI OR f'm, WHICHEVER IS GREATER. TESTING SHALL BE DONE IN AN ABSORBENT MOLD IN ACCORDANCE WITH ASTM C 1019.

6. MASONRY JOINT REINFORCEMENT SHALL COMPLY WITH ASTM A-951 AND SHALL BE HOT DIPPED GALVANIZED, CARBON STEEL. BRICK TIES SHALL CONFORM TO SEISMIC DESIGN CATEGORY REQUIREMENTS (SUBMIT FOR APPROVAL). BRICK TIES USED IN SEISMIC DESIGN CATEGORY D SHALL BE PLACED AT 16" ON CENTER VERTICALLY AND HORIZONTALLY. PROVIDE IN LENGTHS NOT LESS THAN 10 FEET IN LENGTH WITH PREFABRICATED CORNER AND TEE UNITS. FOR MULTIWYTHE MASONRY PROVIDE ADJUSTABLE 2-PIECE UNITS. PROVIDE CONTINUITY AT CORNERS AND WALL INTERSECTIONS BY USING PREFABRICATED "L" AND "T" SECTIONS. LAP REINFORCEMENT A MINIMUM OF 6". SPACE REINFORCEMENT NOT MORE THAN 16" O.C. PROVIDE REINFORCEMENT NOT MORE THAN 8" ABOVE OR BELOW WALL OPENINGS AND EXTENDING 24" BEYOND OPENINGS. CUT REINFORCEMENT AT CONTROL AND EXPANSION JOINTS UNLESS NOTED OTHERWISE.

7. ALL BOND BEAM REINFORCING AT FLOOR AND ROOF DIAPHRAGMS SHALL BE CONTINUOUS THROUGH MASONRY CONTROL JOINTS - UNLESS NOTED OTHERWISE.

8. PROVIDE VERTICAL CONTROL JOINTS AT 1.5 TIMES WALL HEIGHT OR 25' MAXIMUM (WHICHEVER IS LEAST). SEE ARCHITECTURAL DRAWINGS.

9. THE MASONRY CONTRACTOR SHALL PROVIDE ALL REQUIRED TEMPORARY WALL BRACING DURING CONSTRUCTION.

10. THE MINIMUM QUALITY ASSURANCE PROGRAM FOR NON-ESSENTIAL FACILITIES SHALL COMPLY WITH TABLE 1.14.2 OF ACI 530.

11. SEE MASONRY DETAILS ON SHEET S302.

STRUCTURAL STEEL:

1. ALL W-SHAPE STRUCTURAL STEEL SHALL BE ASTM A992, ALL OTHER STRUCTURAL SHAPES SHALL BE ASTM A-36, SQUARE OR RECTANGULAR HSS SHAPES SHALL CONFORM TO ASTM A-500, GRADE B, ROUND HSS SHAPES SHALL CONFORM TO ASTM A-500, GRADE B, STRUCTURAL STEEL PIPE COLUMNS SHALL CONFORM TO ASTM A-501 OR ASTM A-53, TYPE E OR S, GRADE B. DESIGN, DETAILING, FABRICATION AND ERECTION SHALL BE IN ACCORDANCE WITH THE AISC CODE AND DETAILING MANUAL. NO STRUCTURAL MEMBERS SHALL BE SPLICED EXCEPT AS SHOWN ON APPROVED SHOP DRAWINGS.

2. FABRICATOR IS SOLELY RESPONSIBLE FOR THE DESIGN OF THE CONNECTIONS SHOWN ON THE STRUCTURAL DRAWINGS. REVIEW OF STRUCTURAL STEEL CONNECTIONS BY WGPM, INC. IS FOR GENERAL DESIGN INTENT ONLY. FOR THE PURPOSE OF CONNECTION DESIGN, THE FABRICATOR SHALL RETAIN A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT. THE ENGINEER SHALL SEAL, SIGN AND SUBMIT DESIGN CALCULATIONS FOR ALL NON-STANDARD AND LATERAL RESISTING CONNECTION DESIGNS. A NOTE SHOULD ACCOMPANY THE SEAL STATING THAT THE SEAL IS FOR "CONNECTION DESIGN ONLY" AND DOES NOT INCLUDE RESPONSIBILITY FOR MEMBER OR BUILDING DESIGN, DIMENSIONS, FITUP, ERECTION AND ETC. GENERALLY CONNECTIONS SHOWN ON THE STRUCTURAL DRAWINGS ARE SCHEMATIC AND ARE INTENDED TO SHOW THE RELATIONSHIP OF THE MEMBERS. CONNECTIONS SHALL BE DESIGNED FOR REACTIONS SHOWN ON CONTRACT STRUCTURAL DRAWINGS, IF REACTIONS ARE NOT SHOWN ON CONTRACT STRUCTURAL DRAWINGS, DESIGN FOR ONE HALF (1/2) THE ALLOWABLE LOAD ON THE MEMBER, USING THE AISC "ALLOWABLE UNIFORM LOAD TABLES" WITH GIVEN BEAM SPAN, OR A MINIMUM OF 10 KIPS, WHICHEVER IS GREATEST. MEMBER FORCES AND REACTIONS HAVE BEEN REDUCED IN CONFORMANCE TO CODE PROVISIONS RELATED TO COMBINATIONS OF LOADINGS THAT INCLUDE WIND AND SEISMIC FORCES. NO FURTHER REDUCTIONS IN FORCES OR INCREASED IN ALLOWABLE STRESSES IS PERMITTED. CONNECTIONS MAY BE BOLTED OR WELDED UNLESS NOTED OTHERWISE.

3. FABRICATOR SHALL BE CATEGORY I CERTIFIED (CONVENTION STEEL STRUCTURES), OR A COMPANY SPECIALIZING IN PROJECTS OF THIS NATURE WITH A MINIMUM OF 5 YEARS OF EXPERIENCE.

4. ALL SHOP AND FIELD WELDING SHALL BE BY A CERTIFIED WELDER AND SHALL CONFORM TO AWS STANDARDS (LATEST EDITION). FIELD FILLET WELDS GREATER THAN 1/4" THICKNESS SHALL BE INSPECTED BY AN INDEPENDENT TESTING AGENCY.

5. WHERE CAMBER IS NOT PRESENT ERECT MILL CAMBER UP.

6. SEE ARCHITECTURAL DRAWINGS FOR MISCELLANEOUS STEEL NOT SHOWN ON STRUCTURAL DRAWINGS.

7. GALVANIZE OR PAINT ALL EXTERIOR EXPOSED STRUCTURAL STEEL, SEE ARCHITECTURAL DRAWINGS.

I JOISTS:

1. A STRUCTURAL ENGINEER LICENSED TO PRACTICE IN THE STATE OF THE PROJECT SHALL DESIGN ALL I JOISTS. DESIGN FOR ALL CODE REQUIRED LIVE, SNOW AND WIND LOADS. DESIGNS SHALL BE SEALED AND SIGNED BY HIM/HER AND SHALL BE SUBMITTED TO THE ARCHITECT FOR APPROVAL. SUBMIT SHOP DRAWINGS SHOWING LAYOUT OF I JOISTS AND STRUCTURAL FRAMING INCLUDING ARRANGEMENT, DIMENSIONS, MATERIALS, GRADES, STRESS VALUES, CONNECTORS, ANCHORAGE, AND RELATION TO ADJACENT WORK TO ARCHITECT FOR APPROVAL. GENERAL CONTRACTOR SHALL PROVIDE I JOIST SUPPLIER WITH SPRINKLER LAYOUT PLAN WITH HANGER LOCATIONS AND WEIGHTS. GENERAL CONTRACTOR SHALL PROVIDE I JOIST SUPPLIER WITH ALL OTHER HVAC/ELECTRICAL HANGING LOADS.

FLOOR I JOISTS:

DEAD LOAD	22.0 PSF
LIVE LOAD	40.0 PSF

2. I JOIST SUPPLIER SHALL PROVIDE ALL CONNECTIONS NOT DETAILED ON STRUCTURAL DRAWINGS. WEB STIFFENERS AND BLOCKING PANELS SHALL BE PROVIDED AS REQUIRED FOR DESIGN LOADS AND SPANS. JOIST SUPPLIER SHALL PROVIDE ALL BRIDGING/BRACING AS REQUIRED.

3. I JOIST MEMBERS AND COMPONENTS SHALL NOT BE CUT, NOTCHED, DRILLED OR ALTERED IN ANY OTHER MANNER WITHOUT THE WRITTEN APPROVAL OF THE I JOIST DESIGNER/SUPPLIER.

4. I JOISTS SHALL BE ERECTED IN ACCORDANCE WITH THE MANUFACTURERS RECOMMENDATIONS. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY AND PERMANENT BRACING AS REQUIRED FOR THE SAFE ERECTION AND PERFORMANCE OF THE I JOISTS.

5. PROVIDE DOUBLE I JOISTS UNDER ALL PARTITION WALLS RUNNING PARALLEL TO JOISTS UNLESS DESIGN SHOWS SINGLE I JOIST CAN SUPPORT PARTITION DEAD LOAD.

7. LOAD BEARING PARTITIONS, JACKS, BEAMS AND COLUMN SUPPORTS MUST BE SOLID BLOCKED THROUGH FLOOR. I-JOISTS AND PLYWOOD CANNOT SUPPORT CONCENTRATED POINT LOADS. I-JOIST MATERIAL SHOULD NOT BE USED AS BLOCKING UNDER CONCENTRATED POINT LOADS. ALL POINT LOADS MUST BE CARRIED TO FOUNDATIONS WITH ADEQUATE BLOCKING AND/OR BEAMS.

7. GENERAL CONTRACTOR SHALL COORDINATE LOCATION OF I JOISTS WITH OTHER TRADES - SHIFT I JOISTS A MAXIMUM OF 3 1/2" AS REQUIRED.

8. I JOIST SIZE AND SPACING SHOWN ON STRUCTURAL DRAWINGS IS FOR PRELIMINARY PRICING PURPOSES ONLY. THE OWNER, ARCHITECT AND STRUCTURAL ENGINEER WILL NOT ACCEPT ANY ADDITIONAL CHARGES FOR FINAL I JOIST DESIGN.

TIMBER/WOOD/PLYWOOD/OSB:

1. A STRUCTURAL ENGINEER LICENSED TO PRACTICE IN THE STATE OF THE PROJECT SHALL DESIGN WOOD TRUSSES. DESIGN FOR ALL CODE REQUIRED LIVE, SNOW AND WIND LOADS. DESIGNS SHALL BE SEALED AND SIGNED BY HIM/HER AND SHALL BE SUBMITTED TO THE ARCHITECT FOR APPROVAL. FABRICATION SHALL BE BY A MEMBER OF THE TRUSS PLATE INSTITUTE, INC. SUBMIT SHOP DRAWINGS SHOWING LAYOUT OF TRUSSES AND STRUCTURAL FRAMING INCLUDING ARRANGEMENT, DIMENSIONS, MATERIALS, GRADES, STRESS VALUES, CONNECTORS, ANCHORAGE, AND RELATION TO ADJACENT WORK TO ARCHITECT FOR APPROVAL. TRUSSES SHALL BE DESIGNED FOR THE FOLLOWING MINIMUM LOADS:

HIGH SLOPED ROOF TRUSSES:

TOP CHORD DEAD LOAD	8.0 PSF
BOTTOM CHORD DEAD LOAD	12.0 PSF
TOP CHORD LIVE LOAD	20.0 PSF
BOTTOM CHORD LIVE LOAD (WHERE CODE REQUIRED)	10.0 PSF

GENERAL CONTRACTOR SHALL PROVIDE TRUSS SUPPLIER WITH SPRINKLER LAYOUT PLAN WITH HANGER LOCATIONS AND WEIGHTS. GENERAL CONTRACTOR SHALL PROVIDE TRUSS SUPPLIER WITH ALL OTHER HVAC/ELECTRICAL HANGING LOADS.

2. ALL TEMPORARY TRUSS BRACING REQUIRED FOR ERECTION, AS PER THE GUIDELINES SET FORTH BY THE TRUSS PLATE INSTITUTE PUBLICATION "HIB-91", SHALL BE PERMANENTLY ATTACHED AND REMAIN IN PLACE TO SERVE AS PERMANENT TRUSS BRACING UNLESS NOTED OTHERWISE.

3. TRUSS MEMBERS AND COMPONENTS SHALL NOT BE CUT, NOTCHED, DRILLED OR ALTERED IN ANY OTHER MANNER WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER RESPONSIBLE FOR DESIGNING THE TRUSSES.

4. TRUSS LAYOUTS AND CONFIGURATIONS SHOWN ARE SCHEMATIC ONLY AND MAY BE ALTERED AS REQUIRED. COORDINATE TRUSS CONFIGURATIONS WITH ALL ARCHITECTURAL REQUIREMENTS AND OTHER TRADES.

5. WOOD TRUSSES SHALL BE ERECTED IN ACCORDANCE WITH THE MANUFACTURERS RECOMMENDATIONS. THE CONTRACTOR SHALL PROVIDE ALL TEMPORARY AND PERMANENT BRACING AS REQUIRED FOR THE SAFE ERECTION AND PERFORMANCE OF THE TRUSSES. PLYWOOD/OSB ROOF SHEATHING SHALL RUN CONTINUOUS UNDER ALL VALLEY/OVERBUILD TRUSSES - TYPICAL.

6. PROVIDE MINIMUM OF TWO (2) STUDS UNDER 2-PLY TRUSSES, THREE (3) STUDS UNDER 3-PLY TRUSSES AND FOUR (4) STUDS UNDER 4-PLY TRUSSES UNLESS NOTED OTHERWISE.

7. MICRO=LAM (LVL) TIMBER SHALL HAVE THE FOLLOWING MINIMUM ALLOWABLE DESIGN STRESSES: BENDING STRESS, Fb = 2,600 PSI, HORIZONTAL SHEAR STRESS, Fv = 285 PSI, AND MODULUS OF ELASTICITY, E = 1,900,000 PSI - CONNECT MULTIPLE MEMBERS TOGETHER AS PER MANUFACTURERS RECOMMENDATIONS.

8. ALL WOOD CONNECTORS, ANCHORS, FASTENERS, TIES, STRAPS, BASES, CAPS, ETC. SHALL BE SIMPSON "STRONG-TIE" (OR EQUIVALENT). CONNECTORS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURERS REQUIREMENTS. ALL CONNECTORS IN CONTACT WITH PRESSURE TREATED LUMBER SHALL MEET THE REQUIREMENTS OF ASTM A653 (CLASS G185) OR ASTM A153.

9. ALL FRAMED LUMBER SHALL BE SOUTHERN PINE NO. 2 (SURFACED AT 19% MOISTURE CONTENT) OR BETTER - UNLESS NOTED OTHERWISE.

10. ALL LOAD BEARING TIMBER WALL STUDS SHALL BE SPRUCE-PINE-FIR(SOUTH) (SURFACE AT 19% MOISTURE CONTENT). TYPICAL UNLESS NOTED OTHERWISE.

11. ALL TIMBER/WOOD/PLYWOOD/OSB IN CONTACT WITH CONCRETE OR MASONRY OR EXPOSED TO THE EXTERIOR SHALL BE PRESSURE TREATED.

12. ALL WOOD CONNECTIONS SHALL NOT BE LESS THAN THOSE SPECIFIED IN TABLE 2304.9.1 OF THE CURRENT NORTH CAROLINA BUILDING CODE/IBC UNLESS NOTED OTHERWISE. LEAD HOLES FOR LAG SCREWS SHALL BE IN ACCORDANCE WITH NDS REQUIREMENTS.

13. GENERAL CONTRACTOR SHALL COORDINATE LOCATION OF TRUSSES WITH OTHER TRADES - SHIFT TRUSSES A MAXIMUM OF 3 1/2" AS REQUIRED.

14. ALL NON TONGUE AND GROOVE PLYWOOD/OSB PANELS SHALL HAVE 1/8" GAP AT ALL PANEL EDGES. PROVIDE SIMPSON PSCL (OR EQUIVALENT) PLYWOOD CLIPS @ 24" AT PANEL EDGES OF ALL ROOF PLYWOOD/OSB SHEATHING. WHERE SHEATHING IS APPLIED TO BOTH SIDES OF A SHEAR WALL PROVIDE DOUBLE STUDS OR STAGGER SHEATHING JOINTS.

15. WALL SHEATHING SHALL LAP AND CONNECT TO FOUNDATION SILL PLATE AND LAP PAST WALL PLATES TO CONNECT TO UPPER STORY FLOOR PLATE - PROVIDE EDGE PATTERN NAILING. PROVIDE 2x BLOCKING AT ALL EDGES.

16. ALL TIMBER/WOOD POSTS GREATER THAN 5" IN SIZE SHALL BE SOUTHERN PINE, NO. 2 DENSE SR OR BETTER. TYPICAL UNLESS NOTED OTHERWISE.

SIMPSON ANCHOR TIEDOWN SYSTEMS (ATS):

1. ATS IS AN ASSEMBLAGE OF STEEL COMPONENTS, WHICH INCLUDE RODS, PLATES, COUPLER NUTS, TAKE-UP DEVICES AND NUTS. STUDS, POSTS AND BLOCKING BY ENGINEER OF RECORD.

2. SIMPSON STRONG-TUE IS PROVIDING THE ANCHOR TIEDWON SYSTEM TO MEET THE DESIGN FORCES PROVIDED BY THE ENGINEER OF RECORD. THE EOR IS RESPONSIBLE FOR EVALUATING THE EFFECTS OF LUMBER SHRINKAGE AND ATS ELONGATION ON SHEARWALL DRIFT.

3. GENERAL CONTRACTOR OF INSTALLER OF ATS SHALL CUT RODS TO LENGTH AS REQUIRED.

4. DO NOT WELD PRODUCTS UNLESS DRAWINGS SPECIFICALLY IDENTIFY A PRODUCT AS ACCEPTABLE FOR WELDING, OR UNLESS SPECIFIC APPROVAL FOR WELDING IS PROVIDED BY SIMPSON STRONG-TIE.

5. FULLY ENGAGE EACH ROD INTO THE SPECIFIED COUPLING NUT OR UNTIL EACH ROD CAN BE SEEN FULLY IN THE WITNESS HOLES.

6. INSTALL NUTS AND ISOLATOR NUTS SNUG TIGHT, PLUS AN ADDITIONAL 1/2 TURN.

7. IN THE EVENT OF A DISCREPANCY BETWEEN THE STRUCTURAL DRAWINGS AND SIMPSON INSTALLATION DRAWINGS, THE STRUCTURAL DRAWINGS SHALL GOVERN.

NOTE!

GEOTECHNICAL ENGINEER SHALL VERIFY REQUIREMENTS FOR VOID SPACE WITH CARDBOARD FORMS UNDER STRUCTURAL FIRST FLOOR SLAB. GENERAL CONTRACTOR SHALL COORDINATE.

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REVISIONS		
No.	Date	Description

KEY PLAN

Shiva Southaven Inc.

Holiday Inn Express & Suites

Lot 16 (Rev Lot 3) Southcrest Pkwy.
Southcrest Subdivision
Southaven, MS 38671

Drawing Title
General Notes

Phase
Construction Documents

Project No.	14-081	Sheet No.
Prepared by	AEB	S001
Checked by	HLW	
Date	Feb. 27, 2015	

Review

FOLDING PARTITION:

1. FOLDING PARTITION IS ASSUMED TO BE 10'-0" HIGH WITH A WEIGHT OF 12 PSF. MAXIMUM SUPPORT LIVE LOAD DEFLECTION SHALL BE LIMITED TO 1 1/2". GENERAL CONTRACTOR SHALL VERIFY WITH PARTITION SUPPLIER.

POST-INSTALLED ANCHORS:

1. POST-INSTALLED ANCHORS SHALL ONLY BE USED WHERE SPECIFIED ON THE CONTRACT DOCUMENTS. CONTRACTOR SHALL OBTAIN APPROVAL FROM STRUCTURAL ENGINEER OF RECORD PRIOR TO USING POST-INSTALLED ANCHORS FOR MISSING OR MISPLACED CAST-IN-PLACE ANCHORS. CARE SHALL BE GIVEN TO AVOID CONFLICTS WITH EXISTING STEEL REINFORCING. HOLES SHALL BE DRILLED AND CLEANED AS PER MANUFACTURERS RECOMMENDATIONS. ANCHORS SHALL BE INSTALLED AS PER MANUFACTURERS RECOMMENDATIONS. CONTRACTOR INSTALLING ADHESIVE ANCHORS SHALL BE TRAINED BY THE MANUFACTURERS REPRESENTATIVE. THIS TRAINING SHALL INCLUDE HOLE DRILLING, CLEANING AND INSTALLATION METHODS FOR CONSTRUCTION CONDITIONS ON THIS PROJECT.

*CONCRETE ANCHORS SEISMIC DESIGN CATEGORY A, B, C, D, E, AND F:
1) EXPANSION ANCHORS - "STRONG-BOLT 2" OR "STRONG-BOLT" BY SIMPSON STRONG-TIE, "KWIK BOLT TZ" BY HILTI OR EQUIVALENT, "POWER STUD+ SD1" BY POWERS FASTENERS - UNLESS NOTED OTHERWISE.
2) CONCRETE ADHESIVE ANCHORS - "SET-XP EPOXY-TIE" BY SIMPSON STRONG-TIE, "HIT-RE 500-SD" OR "HIT-HY 150 MAX-SD" BY HILTI OR EQUIVALENT - UNLESS NOTED OTHERWISE.
3) SCREW ANCHORS - "TITEN HD" BY SIMPSON STRONG-TI, "KWIK HUS-EZ" BY HILTI OR EQUIVALENT.
4) SLEEVE ANCHORS - "HSL-3" BY HILTI OR EQUIVALENT.

*MASONRY ANCHORS:
1) EXPANSION ANCHORS - "WEDGE-ALL" BY SIMPSON STRONG-TIE, "KWIK BOLT 3" BY HILTI OR EQUIVALENT - UNLESS NOTED OTHERWISE.
2) ADHESIVE ANCHORS (GROUT FILLED) - "SET EPOXY-TIE" BY SIMPSON STRONG-TIE, "HIT-HY 150 MAX" BY HILTI OR EQUIVALENT - UNLESS NOTED OTHERWISE.
3) ADHESIVE ANCHORS (HOLLOW CMU OR BRICK) - "SET EPOXY-TIE" BY SIMPSON STRONG-TIE, "HIT-HY 70" BY HILTI OR EQUIVALENT - UNLESS NOTED OTHERWISE.
4) SCREW ANCHORS - "TITEN HD" BY SIMPSON STRONG-TIE, "HUS-H" BY HILTI OR EQUIVALENT.
5) SLEEVE ANCHORS - "SLEEVE-ALL" BY SIMPSON STRONG-TIE, "HLC" BY HILTI OR EQUIVALENT.

STAIR DESIGN:

1. STAIRS, LANDINGS AND HANDRAILS SHALL BE DESIGNED BY A STRUCTURAL ENGINEER LICENSED TO PRACTICE IN THE STATE OF OF THE PROJECT. DESIGNS SHALL BE SEALED AND SIGNED BY HIM/HER AND SHALL BE SUBMITTED TO THE ARCHITECT FOR APPROVAL. SUBMIT SHOP DRAWINGS SHOWING LAYOUT OF STAIRS AND STRUCTURAL FRAMING, INCLUDING ARRANGEMENT, DIMENSIONS, CONNECTIONS AND RELATION TO ADJACENT WORK TO ARCHITECT FOR APPROVAL.

ELEVATOR:

1. GENERAL CONTRACTOR SHALL COORDINATE/VERIFY ALL STRUCTURAL STEEL REQUIRED FOR GUIDE RAIL SUPPORT AND HOIST BEAM. PROVIDE W8x10 HOIST BEAM AND HSS 6x4x5/16 GUIDE RAIL UNLESS NOTED OTHERWISE. VERIFY/COORDINATE ALL STEEL LOCATIONS, DIMENSIONS AND ELEVATIONS - TYPICAL.

PRE-MANUFACTURED CANOPIES AND AWNINGS:

1. THE DESIGN, CONNECTION AND ALL ATTACHMENTS OF ALL CANOPIES AND AWNINGS SHALL BE THE RESPONSIBILITY OF THE CANOPY/AWNING SUPPLIER. THE GENERAL CONTRACTOR SHALL COORDINATE ALL ATTACHMENT REQUIREMENTS AND PROVIDE ADDITIONAL STUDS, BLOCKING ETC. AS REQUIRED.

WALL STUD SCHEDULE.

FLOOR	SPECIES	SIZE	SPACING	LOCATION	NOTES
4TH	SPFS STUD	2x6	@ 16" O.C.	ALL	
3RD	SPFS STUD	2x6	@ 16" O.C.	ALL	
2ND	SPFS STUD	2x6	@ 16" O.C.	ALL	
1ST	SPFS STUD	(2) 2x6	@ 16" O.C.	ALL	

SPFS STUD INDICATES SPRUCE-PINE-FIR (SOUTH) STUD GRADE

SHEAR WALLS SCHEDULE

FLOOR	PLYWOOD/OSB	FASTENER	SPACING	TENSION TIE
1ST	15/32" (1) SIDE OF WALL	8d NAIL	@ 4" O.C.	SEE SHEET S602
2ND	15/32" (1) SIDE OF WALL	8d NAIL	@ 4" O.C.	SEE SHEET S602
3RD	15/32" (1) SIDE OF WALL	8d NAIL	@ 6" O.C.	SEE SHEET S602
4TH	15/32" (1) SIDE OF WALL	8d NAIL	@ 6" O.C.	SEE SHEET S602

* 8d @ 12" O.C. INTERMEDIATE

* ALL TENSION TIES ARE SIMPSON OR EQUIVALENT

NOTE! – PLYWOOD/OSB SHEAR WALLS

MAIN WINDFORCE-RESITING SYSTEM SUBJECT TO SPECIAL INSPECTIONS 1705.4.1 INCLUDES PLYWOOD/OSB SHEATHING AND ATTACHMENT, BOTTOM PL ANCHORS, TENSION TIES, HURRICANE TIES, STRAPS, BLOCKING PANELS, BOLTS, THREADED RODS, AT ALL EXTERIOR WALLS AND INTERIOR SHEAR WALLS (SW), PLYWOOD/OSB FLOOR AND ROOF SHEATHING AND ATTACHMENT.

NOTE! – PLYWOOD/OSB SHEAR WALLS

ALL EXTERIOR WALLS AND INTERIOR SHEAR WALLS (SW) SHALL HAVE 15/32" EXTERIOR GRADE PLYWOOD/OSB SHEATHING. ATTACH SHEATHING TO SUPPORTING MEMBERS USING 8d NAILS. ALL EDGES TO BE BLOCKED WITH 2" NOMINAL FRAMING MEMBERS. NAILS SHALL BE PLACED NOT LESS THAN 3/8" FROM THE PANEL EDGE – SEE SCHEDUE (S002) FOR PANEL EDGE NAIL SPACING – NAIL @ 12" O.C. AT INTERMEDIATE FRAMING MEMBERS. LAYOUT PANELS STAGGERED AND PERPENDICULAR TO WALL STUDS.

NOTE!

ATTACH DOUBLE 2x6 WALL STUDS TOGETHER USING 2 ROWS OF 16d NAILS @ 24" O.C. – TYPICAL

NOTE!

PROVIDE SOLID WOOD BLOCKING FROM ALL STUDS AND MULTIPLE (GANG) STUDS TO BEAM/WALL, STUDS OR FOUNDATION BELOW – TYPICAL

NOTE!

ATTACH GANG STUDS (3 OR MORE STUDS) TOGETHER USING SIMPSON SDS¼x6 SCREWS @ 12" O.C. STAGGERED EACH FACE – TYPICAL WHERE USING 3 OR MORE STUDS TOGETHER

NOTE!

GEOTECHNICAL ENGINEER SHALL VERIFY REQUIREMENTS FOR VOID SPACE WITH CARDBOARD FORMS UNDER STRUCTURAL FIRST FLOOR SLAB. GENERAL CONTRACTOR SHALL COORDINATE.

STRUCTURAL DESIGN CRITERIA:

DESIGN:

1. STRUCTURAL DESIGN CONFORMS TO THE REQUIREMENTS OF THE INTERNATIONAL BUILDING CODE, 2012 EDITION AND ASCE 7-10.

2. BUILDING CATEGORY (T1604.5) II

3. FLOOR LIVE LOADS USED IN DESIGN (POUNDS PER SQUARE FOOT):

RESIDENCE AND CORRIDORS SERVING THEM	40 PSF
PUBLIC ROOMS AND CORRIDORS SERVING THEM	100 PSF
STAIRS	100 PSF

4. BUILDING CODE REQUIRED ROOF LIVE AND SNOW LOAD USED IN DESIGN (POUNDS PER SQUARE FOOT):

LIVE		20 PSF
SNOW - Pg		10 PSF
SNOW - Pf		12 PSF
SNOW EXPOSURE FACTOR, Ce	1.0	
SNOW LOAD IMPORTANCE FACTOR, Is	1.0	
SNOW THERMAL FACTOR, Ct	1.0	
SNOW ROOF SLOPE FACTOR, Cs	1.0 (SLIDING SNOW)	

5. DEAD LOADS USED IN DESIGN (POUNDS PER SQUARE FOOT):

2ND-4TH FLOOR DEAD LOADS:	
FLOOR FINISH	1 PSF
3/4" GYPCRETE TOPPING	8 PSF
3/4" PLYWOOD	3 PSF
HANGING	4 PSF
MECHANICAL	3 PSF
SPRINKLERS	3 PSF
I-JOISTS	3 PSF
ROOF DEAD LOADS:	
ROOFING (SINGLE PLY MECHANICALLY FASTENED)	3 PSF
INSULATION	2 PSF
3/4" PLYWOOD	3 PSF
HANGING	3 PSF
MECHANICAL	3 PSF
SPRINKLERS	3 PSF
TRUSSES	3 PSF

6. WIND LOAD DATA:

ULTIMATE WIND SPEED, Vult	115 MPH
WIND IMPORTANCE FACTOR, Iw	1.0
WIND EXPOSURE	C
INTERNAL PRESSURE COEFFICIENT (ASCE 7-10 T26.11-1)	±0.18

COMPONENT AND CLADDING WIND PRESSURE - ULTIMATE:

	10sf	100sf	500sf
ZONE 4	33.4 PSF	28.5 PSF	25.1 PSF
	-36.2 PSF	-31.3 PSF	-27.8 PSF
ZONE 5	33.4 PSF	28.5 PSF	25.1 PSF
	-44.5 PSF	-28.5 PSF	-27.8 PSF

CALCULATED WIND BASE SHEARS (FOR MWFRS) Vx = 79.2K Vy = 402.2K

7. SEISMIC LOAD DATA:

COMPLIANCE WITH ASCE 7-05 SECTION 11.7 ONLY? NO

SEISMIC DESIGN CATEGORY B, C & D

SEISMIC IMPORTANCE FACTOR, Ie	1.0
SOIL SITE CLASS	E
SPECTRAL RESPONSE ACCELERATION - SHORT PERIOD, SDS	0.619g
SPECTRAL RESPONSE ACCELERATION - 1.0 SECOND, SD1	0.547g
SEISMIC DESIGN CATEGORY	D
BASIC SEISMIC-FORCE RESISTING SYSTEM	
BEARING WALL SYSTEM/LIGHT FRAMED WALL W/ WOOD SHEAR WALLS	
RESPONSE MODIFICATION COEFFICIENT, R	6 1/2
DEFLECTION AMPLIFICATION FACTOR, Cd	4
BUILDING HEIGHT LIMIT, FEET	H = NL
EQUIVALENT LATERAL-FORCE PROCEDURE	

SEISMIC BASE SHEAR Vx = 138.6K Vy = 138.6K

ARCHITECTURAL, MECHANICAL, COMPONENTS ANCHORED? SEE CHAPTER 13 OF ASCE 7-10

LATERAL DESIGN CONTROLLED BY: X-SEISMIC Y-WIND

SOIL BEARING CAPACITIES:
FIELD TEST (PROVIDED COPY OF TEST REPORT) STONE AGGREGATE PIERS 7,000 PSF

PRESUMPTIVE BEARING CAPACITY	NA
PILE SIZE, TYPE AND CAPACITY	NA

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02-27-15

Project No.	14-081	Sheet No.	S002
Prepared by	AEB		
Checked by	HLW		
Date	Feb. 27, 2015		

Review

Holiday Inn Express & Suites

THIS STATEMENT OF SPECIAL INSPECTIONS IS SUBMITTED AS A CONDITION FOR PERMIT ISSUANCE IN ACCORDANCE WITH THE SPECIAL INSPECTIONS REQUIREMENTS OF THE BUILDING CODE. IT INCLUDES A SCHEDULE OF SPECIAL INSPECTION SERVICES APPLICABLE TO THIS PROJECT.

THE SPECIAL INSPECTOR SHALL KEEP RECORDS OF ALL INSPECTIONS AND SHALL FURNISH INSPECTION REPORTS TO THE BUILDING OFFICIAL, STRUCTURAL ENGINEER AND ARCHITECT OF RECORD. DISCOVERED DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. IF SUCH DISCREPANCIES ARE NOT CORRECTED, THE DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL, STRUCTURAL ENGINEER AND ARCHITECT OF RECORD. THE SPECIAL INSPECTION PROGRAM DOES NOT RELIEVE THE CONTRACTOR OF HIS OR HER RESPONSIBILITIES.

INTERIM REPORTS SHALL BE SUBMITTED TO THE BUILDING OFFICIAL, OWNER, STRUCTURAL ENGINEER AND ARCHITECT OF RECORD.

A FINAL REPORT OF SPECIAL INSPECTIONS DOCUMENTING COMPLETION OF ALL REQUIRED SPECIAL INSPECTIONS AND CORRECTION OF ANY DISCREPANCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED PRIOR TO ISSUANCE OF A CERTIFICATE OF USE AND OCCUPANCY.

JOB SITE SAFETY AND MEANS AND METHODS OF CONSTRUCTION ARE SOLELY THE RESPONSIBILITY OF THE CONTRACTOR.

STATEMENT OF SPECIAL INSPECTIONS (INTERNATIONAL BUILDING CODE, 2012 EDITION, CHAPTER 17):

REQUIRED VERIFICATION AND INSPECTION OF STEEL CONSTRUCTION

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD	IBC REFERENCE
1. MATERIAL VERIFICATION OF HIGH-STRENGTH BOLTS, NUTS AND WASHERS:				
A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	X	APPLICABLE ASTM MATERIAL SPECIFICATIONS: AISC 360, SECTION A3.3	-
B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	-	X	-	-
2. INSPECTION OF HIGH-STRENGTH BOLTING:				
A. BEARING-TYPE CONNECTIONS.	-	X	AISC 360, SECTION M2.5	1704.3.3
B. SLIP-CRITICAL CONNECTIONS.	X	X		
3. MATERIAL VERIFICATION OF STRUCTURAL STEEL:				
A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	-	ASTM A 6 OR ASTM A 568	1708.4
B. MANUFACTURER'S CERTIFIED MILL TEST REPORTS.	-	-	ASTM A 6 OR ASTM A 568	
4. MATERIAL VERIFICATION OF WELD FILLER MATERIALS:				
A. IDENTIFICATION MARKINGS TO CONFORM TO ASW STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS.	-	-	AISC 360, SECTION A3.5	-
B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE REQUIRED.	-	-	-	-
5. INSPECTION OF WELDING: A. STRUCTURAL STEEL:	-	-		
1. COMPLETE AND PARTIAL PENETRATION GROOVE WELDS.	X	-	AWS D1.1	1704.3.1
2. MULTIPASS FILET WELDS.	X	-		
3. SINGLE-PASS FILET WELDS > 5/16"	X	-		
4. SINGLE-PASS FILET WELDS < 5/16"	-	X		
5. FLOOR AND DECK WELDS	-	X	AWS D1.3	-
B. REINFORCING STEEL	-	-	AWS D1.4 ACI 318: 3.5.2	-
1. VERIFICATION OF WELDABILITY OF REINFORCING STEEL OTHER THAT ASTM A 706	-	X		
2. REINFORCING STEEL-RESISTING FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, AND BOUNDARY ELEMENTS OF SPECIAL REINFORCED CONCRETE SHEAR WALLS AND SHEAR REINFORCEMENT.	X	-		
3. SHEAR REINFORCEMENT.	X	-		
4. OTHER REINFORCING STEEL.	-	X		
6. INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE WITH APPROVED CONSTRUCTION DOCUMENT:	-	X	-	1704.3.2
A. DETAILS SUCH AS BRACING AND STIFFENING.	-	-		
B. MEMBER LOCATIONS.	-	-		
C. APPLICATION OF JOINT DETAILS AT EACH CONNECTION.	-	-		

REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONSTRUCTION

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCED STANDARD	IBC REFERENCE
1. INSPECTION OF REINFORCING STEEL, INCLUDING PRESTRESSING TENDONS, AND PLACEMENT.	-	X	ACI 318: 3.5, 7.1-7.7	1913.4
2. INSPECTION OF REINFORCING STEEL WELDING IN ACCORDANCE WITH TABLE 1704.3 ITEM 5B.	-	-	AWS D1.4 ACI 318: 3.5.2	-
3. INSPECT BOLTS TO BE INSTALLED IN CONCRETE PRIOR TO AND DURING PLACEMENT OF CONCRETE WHERE ALLOWABLE LOADS HAVE BEEN INCREASED.	X	-	-	1911.5
4. VERIFYING USE OF REQUIRED DESIGN MIX.	-	X	ACI 318: CH. 4, 5.2-5.4	1904.2.2, 1913.2, 1913.3
5. AT THE TIME FRESH CONCRETE IS SAMPLED TO FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE.	X	-	ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.8	1913.10
6. INSPECTION OF CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	X	-	ACI 318: 5.9, 5.10	1913.6, 1913.7 1913.8
7. INSPECTION FOR MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	-	X	ACI 318: 5.11-5.13	1913.9
11. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	-	X	ACI 318: 6.1.1	-

REQUIRED VERIFICATION AND INSPECTION OF MASONRY CONSTRUCTION: LEVEL 1 INSPECTION

INSPECTION TASK	FREQUENCY OF INSPECTION		REFERENCE FOR CRITERIA		
	CONTINUOUS DURING TASK LISTED	PERIODICALLY DURING TASK LISTED	IBC SECTION	ACI 530/ASCE 5/TMS 402[a]	ACI 530/ASCE 6/TMS 602[a]
1. AS MASONRY CONSTRUCTION BEGINS, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:					
A. PROPORTIONS OF SITE-PREPARED MORTAR.	-	X	-	-	ART. 2.6A
B. CONSTRUCTION OF MORTAR JOINTS.		X			ART. 3.3B
C. LOCATION OF REINFORCEMENT AND CONNECTORS, PRESTRESSING TENDONS AND ANCHORAGES.		X			ART. 3.4,3.6A
D. PRESTRESSING TECHNIQUE.		X			ART. 3.6B
E. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES.		X			ART. 2.4B, 2.4H
2. THE INSPECTION PROGRAM SHALL VERIFY:					
A. SIZE AND LOCATION OF STRUCTURAL ELEMENTS.	-	X	-	-	ART. 3.3G
B. TYPE, SIZE AND LOCATION OF ANCHORS, INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES OR OTHER CONSTRUCTION.	-	X	-	SEC. 1.2.2(e), 2.1.4, 3.1.6	-
C. SPECIFIED SIZE, GRADE AND TYPE OF REINFORCEMENT.	-	X	-	SEC. 1.13	ART. 2.4, 3.4
E. PROTECTION OF MASONRY DURING COLD WEATHER (TEMPERATURE BELOW 40°F) OR HOT WEATHER (TEMPERATURE ABOVE 90°F).	-	X	SEC. 2104.3, 2104.4	-	ART. 1.8C, 1.8D
F. APPLICATION AND MEASUREMENT OF PRESTRESSING FORCE.	-	X	-	-	ART. 3.3B
3. PRIOR TO GROUTING, THE FOLLOWING SHALL BE VERIFIED TO ENSURE COMPLIANCE:					
A. GROUT SPACE IS CLEAN.	-	X	-	-	ART. 3.2D
B. PLACEMENT OF REINFORCEMENT AND CONNECTORS.		X		SEC. 1.13	ART. 3.4
C. PROPORTION OF SITE-PREPARED GROUT AND PRESTRESSING GROUT FOR BONDDED TENDONS.		X		-	ART. 2.6B
D. CONSTRUCTION OF MORTAR JOINTS.		X		-	ART 3.3B
4. GROUT PLACEMENT SHALL BE VERIFIED TO ENSURE COMPLIANCE WITH CODE AND CONSTRUCTION DOCUMENT PROVISIONS.	X	-	-	-	ART. 3.5
5. PREPARATION OF ANY REQUIRED GROUT SPECIMENS, MORTAR SPECIMENS AND/OR PRISMS SHALL BE OBSERVED.	X	-	SEC. 2105.2.2, 2105.3	-	ART. 1.4
6. COMPLIANCE WITH REQUIRED INSPECTION PROVISIONS OF THE CONSTRUCTION DOCUMENTS AND THE APPROVED SUBMITTALS SHALL BE VERIFIED.	-	X	-	-	ART. 1.5

WALL PANELS AND VENEERS

Verification/Inspection	Agent No. / MQIA	Inspections Cont.	Inspections Periodic	Referenced Standard	IBC Reference	Notes	Design
1. Architectural wall panels a. Interior b. Exterior	1 1	– –	X 50% X 50%		1704.10		A
2. Masonry veneer (see 2.1–2.6)					1704.5		A
3. Exterior insulations and finish systems (EIFS) See Note #1 below			X		1704.12		A
4. Special cases: Special Inspections are required for work, that is in the opinion of the Building Official, unusual in its nature such as, but not limited to: a. Construction materials and systems that are alternatives to materials and systems prescribed by the code. b. Unusual design applications of materials described in the code. c. Materials and systems required to be installed with addtional manufacturers instructions that prescribe requirements not contained in the code or referenced standards.		as required	X		1704.13		A

NOTE #1: Special Inspections for EIFS are not required when installed over a water resistive barrier, with a means of draining moisture to the exterior; and not required where installed over concrete or masonry walls.

SEISMIC RESISTANCE

Verification/Inspection	Agent No. / MQIA	Inspections Cont.	Inspections Periodic	Referenced Standard	IBC Reference	Notes	Design
1. Structural Wood: a. Periodic special inspection is required for nailing, bolting, anchoring and framing components within the seismic–force–resisting system, including wood shear walls, wood diaphragms, drag struts, braces, shear panels and hold downs	1	–	X		1707.3		S

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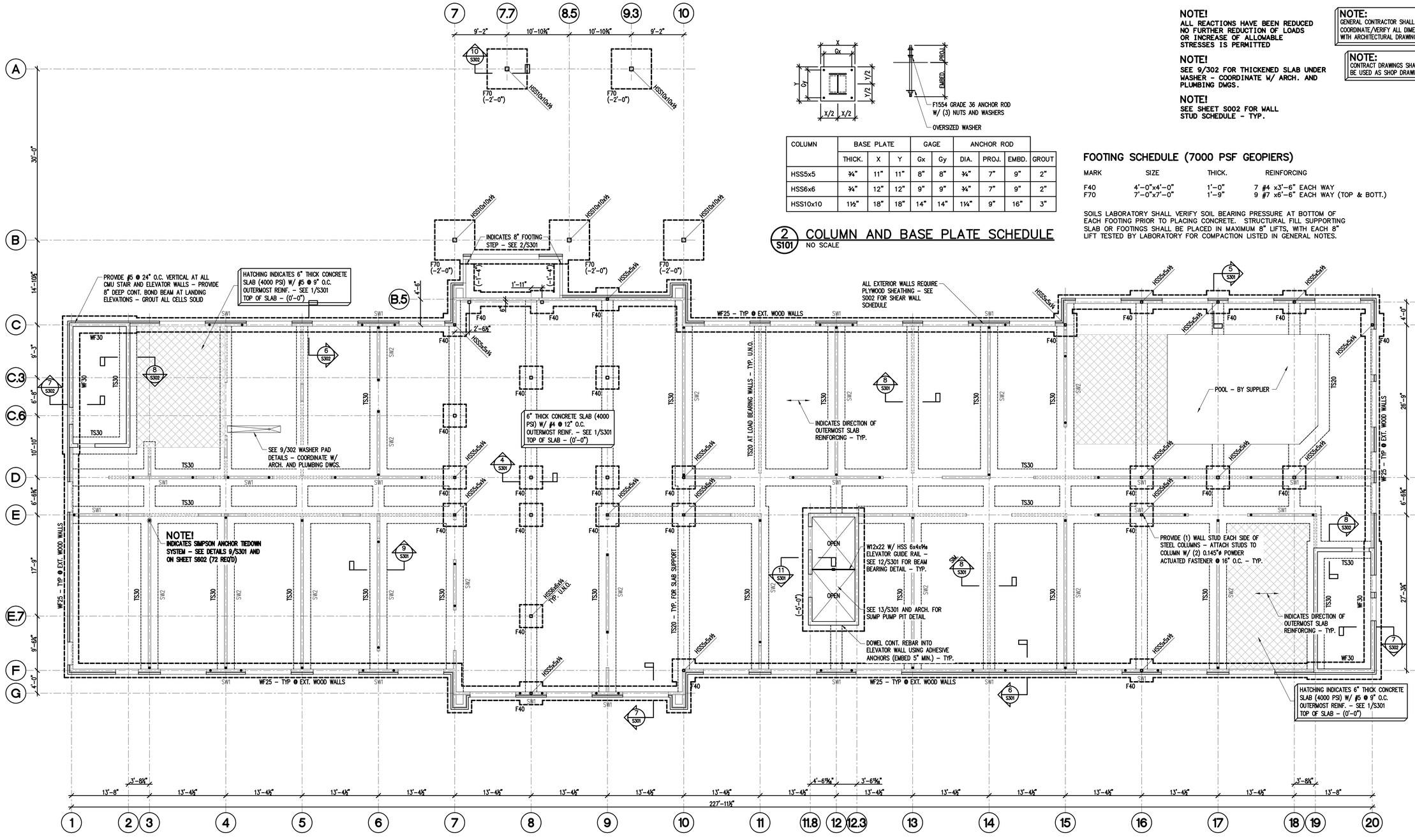
JOB NUMBER: 128-14

WESLEY T. PATTON REGISTERED PROFESSIONAL ENGINEER 12766 STATE OF MISSISSIPPI
02-27-15

Project No.	14-081
Prepared by	AEB
Checked by	HLW
Date	Feb. 27, 2015
Sheet No.	S003

Review

Holiday Inn Express & Suites



COLUMN	BASE PLATE			GAGE		ANCHOR ROD			
	THICK.	X	Y	Gx	Gy	DIA.	PROJ.	EMBD.	GROUT
HSS5x5	¾"	11"	11"	8"	8"	¾"	7"	9"	2"
HSS6x6	¾"	12"	12"	9"	9"	¾"	7"	9"	2"
HSS10x10	1½"	18"	18"	14"	14"	1¼"	9"	16"	3"

F1554 GRADE 36 ANCHOR ROD W/ (3) NUTS AND WASHERS

OVERSIZED WASHER

2/S101 COLUMN AND BASE PLATE SCHEDULE
NO SCALE

NOTE!
ALL REACTIONS HAVE BEEN REDUCED NO FURTHER REDUCTION OF LOADS OR INCREASE OF ALLOWABLE STRESSES IS PERMITTED

NOTE!
SEE 9/302 FOR THICKENED SLAB UNDER WASHER - COORDINATE W/ ARCH. AND PLUMBING DWGS.

NOTE!
SEE SHEET S002 FOR WALL STUD SCHEDULE - TYP.

NOTE:
GENERAL CONTRACTOR SHALL COORDINATE/VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS

NOTE:
CONTRACT DRAWINGS SHALL NOT BE USED AS SHOP DRAWINGS

FOOTING SCHEDULE (7000 PSF GEOPIERS)

MARK	SIZE	THICK.	REINFORCING
F40	4'-0"x4'-0"	1'-0"	7 #4 x3'-6" EACH WAY
F70	7'-0"x7'-0"	1'-9"	9 #7 x6'-6" EACH WAY (TOP & BOTT.)

SOILS LABORATORY SHALL VERIFY SOIL BEARING PRESSURE AT BOTTOM OF EACH FOOTING PRIOR TO PLACING CONCRETE. STRUCTURAL FILL SUPPORTING SLAB OR FOOTINGS SHALL BE PLACED IN MAXIMUM 8" LIFTS, WITH EACH 8" LIFT TESTED BY LABORATORY FOR COMPACTION LISTED IN GENERAL NOTES.

NOTE!
ALL OUTERMOST SLAB REINFORCING AT END SPANS SHALL HAVE 90° STD. HOOK

NOTE!
GEOTECHNICAL ENGINEER SHALL VERIFY REQUIREMENTS FOR VOID SPACE WITH CARDBOARD FORMS UNDER STRUCTURAL FIRST FLOOR SLAB. GENERAL CONTRACTOR SHALL COORDINATE.

WALL FOOTING SCHEDULE 7,000 PSF SOIL BEARING (GEOPIERS)

MARK	WIDTH	THICK.	LONG. REINF.	TRANSVERSE REINF.	COMMENTS
TS20	2'-0"	1'-0"	(3) #4 BOTTOM	#4x1'-6" @ 24" O.C. BOTTOM	SEE 8/S301
TS30	3'-0"	1'-3"	(4) #5 BOTTOM	#4x2'-6" @ 14" O.C. BOTTOM	SEE 9/S301 SEE 8/S302
WF25	2'-6"	1'-0"	(3) #5 BOTTOM	#4x2'-0" @ 18" O.C. BOTTOM	SEE 6/S301
WF30	3'-0"	1'-3"	(4) #3 TOP (4) #5 BOTTOM	#4x2'-6" @ 14" O.C. TOP #4x2'-6" @ 14" O.C. BOTTOM	SEE 7/S302

SOILS LABORATORY SHALL VERIFY SOIL BEARING PRESSURE AT BOTTOM OF EACH FOOTING PRIOR TO PLACING CONCRETE. STRUCTURAL FILL SUPPORTING SLAB OR FOOTINGS SHALL BE PLACED IN MAXIMUM 8" LIFTS, WITH EACH 8" LIFT TESTED BY LABORATORY FOR COMPACTION LISTED IN GENERAL NOTES.

1/S101 FOUNDATION AND FLOOR SLAB PLAN
1/8" = 1'-0"

NOTES:

1. ALL ELEVATIONS REFERENCED ( ) FROM FINISH FLOOR ELEVATION 325.33' (0-0).
2. INTERIOR TOP OF FOOTING - T.O.F. (-1'-4") TYPICAL UNLESS NOTED OTHERWISE.
3. EXTERIOR TOP OF FOOTING - T.O.F. (-1'-4") TYPICAL UNLESS NOTED OTHERWISE.
4. CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS.
5. SEE SHEET S001 AND S002 FOR GENERAL NOTES, WALL STUDS AND WALL SHEATHING NOTES.
6. ALL FOOTING STEPS 16" TYPICAL UNLESS NOTED OTHERWISE - SEE 2/S301.
7. SW1 INDICATES SHEAR WALL 1 - SEE SHEAR WALL SCHEDULE ON SHEET S002 TYPICAL.

TYPICAL WOOD FRAMING NOTES

1. ALL WALLS LABELED SW1 AND SW2 ARE SHEAR WALLS. SEE SHEET S002 FOR SHEAR WALL SCHEDULE.

2. SEE SHEET S002 FOR WALL STUDS AND PLYWOOD/OSB WALL SHEATHING - TYP.

3. SEE 10/S401 FOR HOLE IN I-JOIST - GENERAL CONTRACTOR TO VERIFY/COORDINATE WITH I-JOIST SUPPLIER - TYP.

4. WOOD I-JOISTS SHALL BE DESIGNED FOR ALL ADDITIONAL LOADS SHOWN ON FRAMING PLANS AND PROVIDE ADDITIONAL I-JOISTS IF REQUIRED.

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JOB NUMBER: 128-14

WESLEY T. PATTON REGISTERED PROFESSIONAL ENGINEER 12768 STATE OF MISSISSIPPI
02-27-15

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REVISIONS		
No.	Date	Description

KEY PLAN

Shiva Southaven Inc.

Holiday Inn Express & Suites

Lot 16 (Rev Lot 3) Southcrest Pkwy.
Southcrest Subdivision
Southaven, MS 38671

Drawing Title
Foundation and Floor Slab Plan

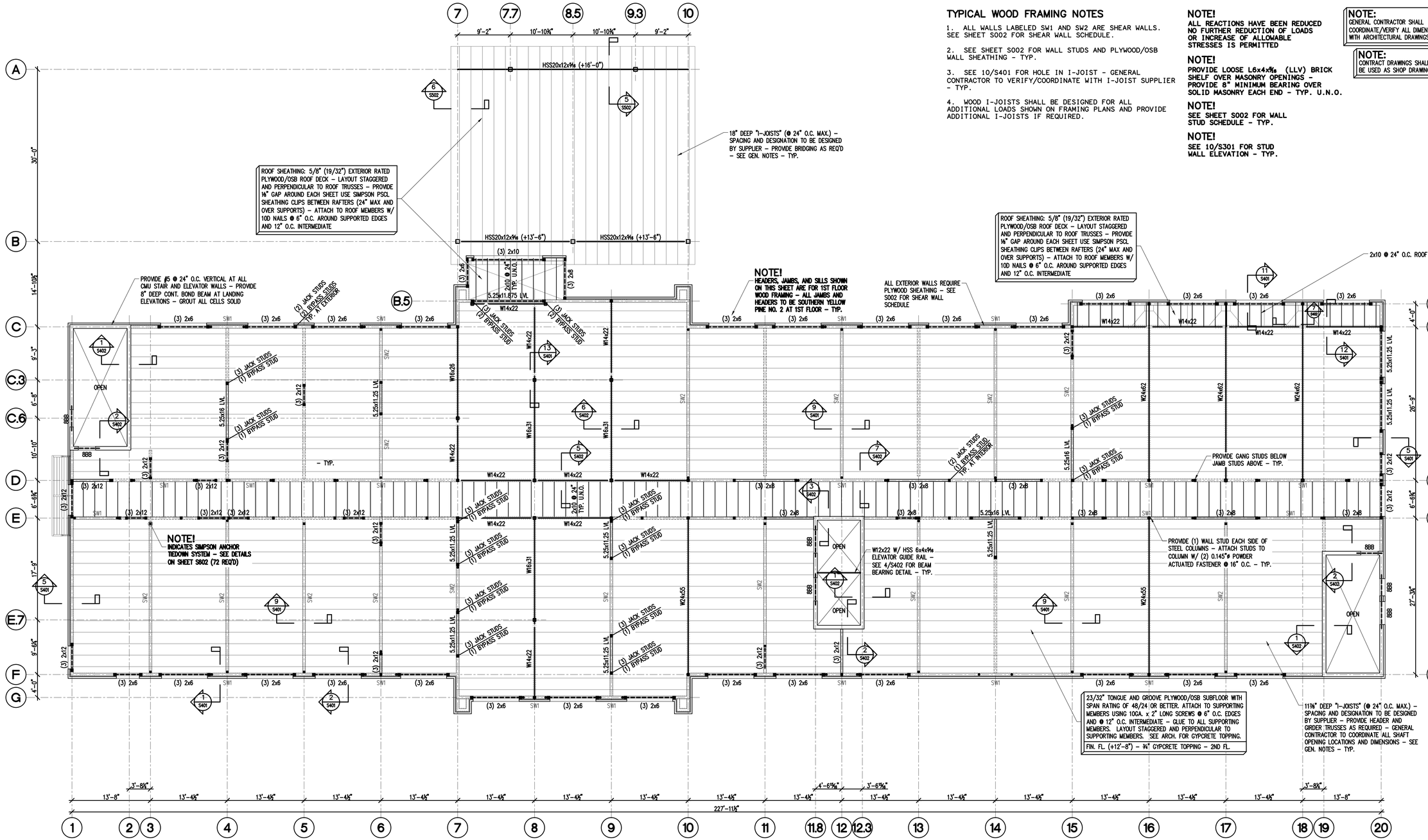
Phase
Construction Documents

Project No.	14-081
Prepared by	AEB
Checked by	HLW
Date	Feb. 27, 2015

Sheet No. S101

Review

Holiday Inn Express & Suites



TYPICAL WOOD FRAMING NOTES

1. ALL WALLS LABELED SW1 AND SW2 ARE SHEAR WALLS. SEE SHEET S002 FOR SHEAR WALL SCHEDULE.
2. SEE SHEET S002 FOR WALL STUDS AND PLYWOOD/OSB WALL SHEATHING - TYP.
3. SEE 10/S401 FOR HOLE IN I-JOIST - GENERAL CONTRACTOR TO VERIFY/COORDINATE WITH I-JOIST SUPPLIER - TYP.
4. WOOD I-JOISTS SHALL BE DESIGNED FOR ALL ADDITIONAL LOADS SHOWN ON FRAMING PLANS AND PROVIDE ADDITIONAL I-JOISTS IF REQUIRED.

NOTE!
ALL REACTIONS HAVE BEEN REDUCED NO FURTHER REDUCTION OF LOADS OR INCREASE OF ALLOWABLE STRESSES IS PERMITTED

NOTE!
PROVIDE LOOSE L6x4x⅝ (LLV) BRICK SHELF OVER MASONRY OPENINGS – PROVIDE 8" MINIMUM BEARING OVER SOLID MASONRY EACH END – TYP. U.N.O.

NOTE!
SEE SHEET S002 FOR WALL STUD SCHEDULE – TYP.

NOTE!
SEE 10/S301 FOR STUD WALL ELEVATION – TYP.

NOTE:
GENERAL CONTRACTOR SHALL COORDINATE/VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS

NOTE:
CONTRACT DRAWINGS SHALL NOT BE USED AS SHOP DRAWINGS

1 S201 2ND FLOOR FRAMING PLAN

1/8" = 1'-0"

NOTES:

1. ALL ELEVATIONS REFERENCED FROM FINISH FLOOR ELEVATION 325.33' (0-0).
2. "I-JOIST BEARING" - J.B. (+11'-6⅝") SECOND FLOOR TYPICAL UNLESS NOTED OTHERWISE.
3. TOP OF STEEL - T.O.S. (+12'-5") TYPICAL UNLESS NOTED OTHERWISE.
4. CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS.
5. SEE SHEET S001 AND S002 FOR GENERAL NOTES, WALL STUDS AND WALL SHEATHING NOTES.
6. LVL INDICATES MICRO=LAM LVL BY I-LEVEL OR EQUIVALENT.
7. BB INDICATES 8" DEEP BOND BEAM - SEE 5/S302.
8. SEE SHEAR WALL SCHEDULE ON SHEET S002 TYPICAL.

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REVISIONS		
No.	Date	Description

KEY PLAN

Shiva Southaven Inc.

Holiday Inn Express & Suites

Lot 16 (Rev Lot 3) Southcrest Pkwy.
Southcrest Subdivision
Southaven, MS 38671

Drawing Title
2nd Floor Framing Plan

Phase
Construction Documents

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JOB NUMBER: 128-14

WESLEY T. PATTON REGISTERED PROFESSIONAL ENGINEER 12748 STATE OF MISSISSIPPI
02-27-15

Project No.	14-081	Sheet No.	S201
Prepared by	AEB		
Checked by	HLW		
Date	Feb. 27, 2015		

Review

Holiday Inn Express & Suites

TYPICAL WOOD FRAMING NOTES

1. ALL WALLS LABELED SW1 AND SW2 ARE SHEAR WALLS. SEE SHEET S002 FOR SHEAR WALL SCHEDULE.

2. SEE SHEET S002 FOR WALL STUDS AND PLYWOOD/OSB WALL SHEATHING - TYP.

3. SEE 10/S401 FOR HOLE IN I-JOIST - GENERAL CONTRACTOR TO VERIFY/COORDINATE WITH I-JOIST SUPPLIER - TYP.

4. WOOD I-JOISTS SHALL BE DESIGNED FOR ALL ADDITIONAL LOADS SHOWN ON FRAMING PLANS AND PROVIDE ADDITIONAL I-JOISTS IF REQUIRED.

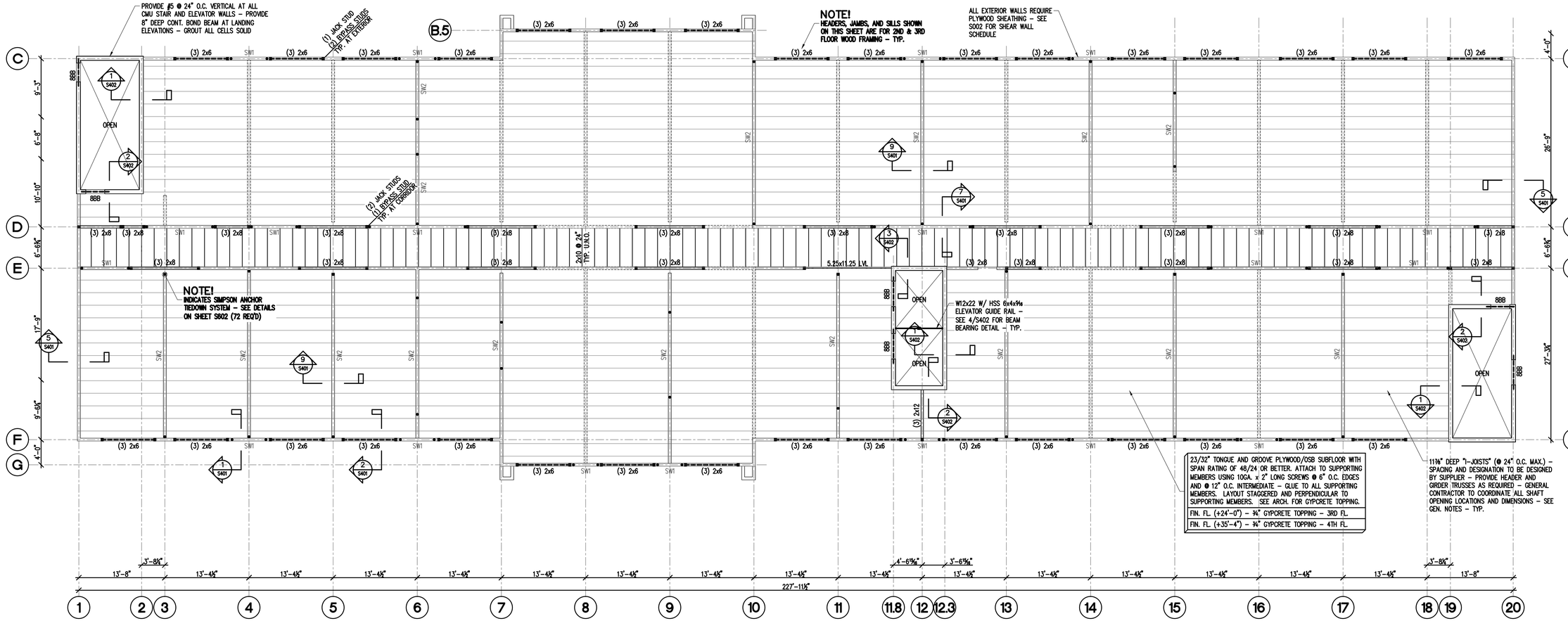
NOTE!
ALL REACTIONS HAVE BEEN REDUCED NO FURTHER REDUCTION OF LOADS OR INCREASE OF ALLOWABLE STRESSES IS PERMITTED

NOTE!
SEE SHEET S002 FOR WALL STUD SCHEDULE - TYP.

NOTE!
SEE 10/S301 FOR STUD WALL ELEVATION - TYP.

NOTE:
GENERAL CONTRACTOR SHALL COORDINATE/VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS

NOTE:
CONTRACT DRAWINGS SHALL NOT BE USED AS SHOP DRAWINGS



1/S202 3RD AND 4TH FLOOR FRAMING PLAN

1/8" = 1'-0"

NOTES:

1. ALL ELEVATIONS REFERENCED FROM FINISH FLOOR ELEVATION 325.33' (0-0).
2. "I-JOIST BEARING" - J.B. (+22'-10 5/8") THIRD FLOOR TYPICAL UNLESS NOTED OTHERWISE.
   J.B. (+34'-2 5/8") FOURTH FLOOR TYPICAL UNLESS NOTED OTHERWISE.
3. CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS.
4. SEE SHEET S001 AND S002 FOR GENERAL NOTES, WALL STUDS AND WALL SHEATHING NOTES.
5. LVL INDICATES MICRO=LAM LVL BY I-LEVEL OR EQUIVALENT.
6. BB INDICATES 8" DEEP BOND BEAM - SEE 5/S302.
7. SEE SHEAR WALL SCHEDULE ON SHEET S002 TYPICAL.

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REVISIONS		
No.	Date	Description

KEY PLAN

Shiva Southaven Inc.

Holiday Inn Express & Suites

Lot 16 (Rev Lot 3) Southcrest Pkwy.
Southcrest Subdivision
Southaven, MS 38671

Drawing Title
3rd and 4th Floor Framing Plan

Phase
Construction Documents

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JOB NUMBER: 128-14

WESLEY T. PATTON REGISTERED PROFESSIONAL ENGINEER 12768 STATE OF MISSISSIPPI
02-27-15

Project No.	14-081	Sheet No.	S202
Prepared by	AEB		
Checked by	HLW		
Date	Feb. 27, 2015		

Review

Holiday Inn Express & Suites

TYPICAL WOOD FRAMING NOTES

1. ALL WALLS LABELED SW1 AND SW2 ARE SHEAR WALLS. SEE SHEET S002 FOR SHEAR WALL SCHEDULE.

2. SEE SHEET S002 FOR WALL STUDS AND PLYWOOD/OSB WALL SHEATHING - TYP.

3. SEE 10/S401 FOR HOLE IN I-JOIST - GENERAL CONTRACTOR TO VERIFY/COORDINATE WITH I-JOIST SUPPLIER - TYP.

4. WOOD I-JOISTS SHALL BE DESIGNED FOR ALL ADDITIONAL LOADS SHOWN ON FRAMING PLANS AND PROVIDE ADDITIONAL I-JOISTS IF REQUIRED.

NOTE!
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NOTE!
SEE SHEET S002 FOR WALL STUD SCHEDULE - TYP.

NOTE!
SEE 10/S301 FOR STUD WALL ELEVATION - TYP.

NOTE:
GENERAL CONTRACTOR SHALL COORDINATE/VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS

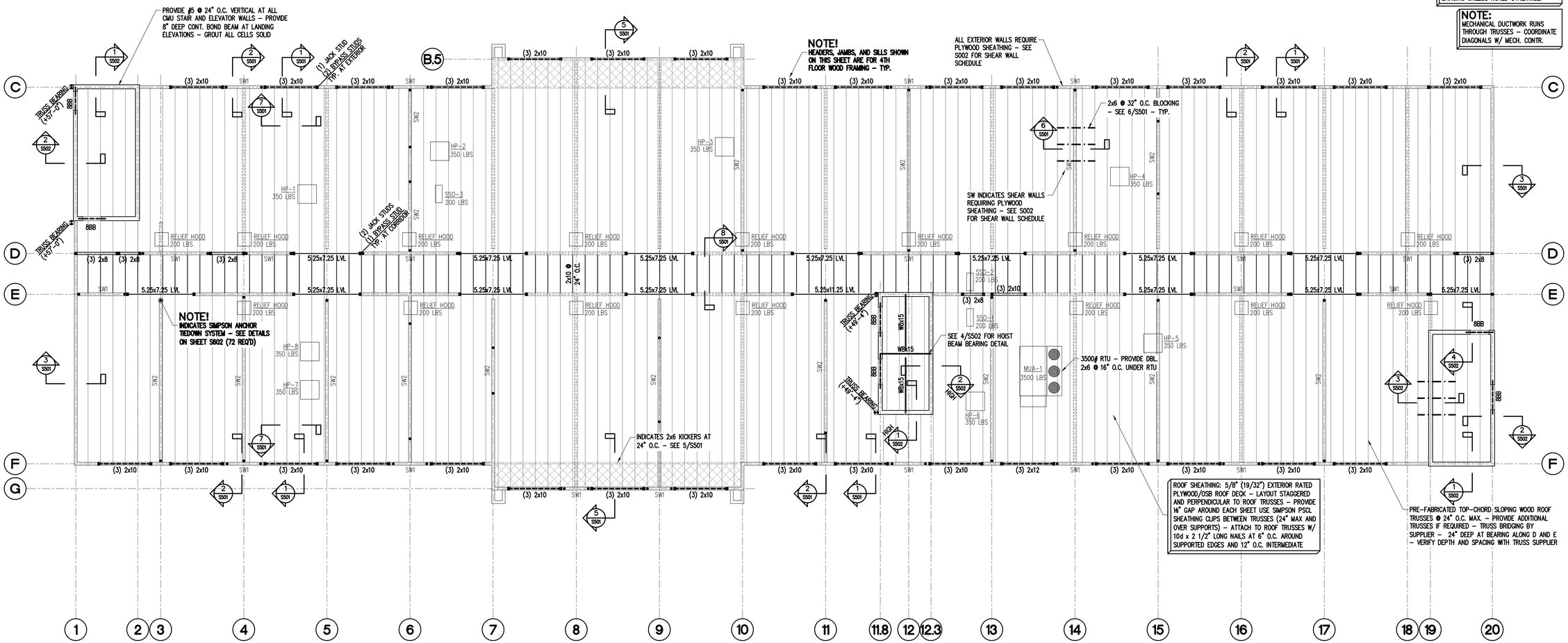
NOTE:
CONTRACT DRAWINGS SHALL NOT BE USED AS SHOP DRAWINGS

NOTE:
MECHANICAL CONTR. SHALL VERIFY SIZE, WEIGHT AND LOCATION OF ALL MECHANICAL UNITS AND OPENINGS PRIOR TO WOOD JOIST FABRICATION. MECH. CONTRACTOR SHALL VERIFY MAGNITUDE AND LOCATION OF POINT LOADS PRIOR TO TRUSS FABRICATION. ALL INFORMATION REGARDING MECHANICAL UNITS ARE BASED ON PROPOSED UNITS. PROVIDE DBL. 2x6 FRAME UNDER MECHANICAL UNIT CURBS BELOW ROOF DECK AND 2x6 FRAME AROUND ROOF OPENINGS - SEE 9/S501 - TYP.

NOTE:
ATTACHMENT OF ALL MECHANICAL UNITS TO SUPPORTING STRUCTURE SHALL BE DESIGNED AND FURNISHED BY MECH. UNIT SUPPLIER TO RESIST LOCAL SEISMIC AND WIND LOADS.

NOTE:
ALL TEMPORARY TRUSS BRACING REQUIRED FOR ERECTION SHALL BE PERMANENTLY ATTACHED AND REMAIN IN PLACE TO SERVE AS PERMANENT TRUSS BRACING UNLESS NOTED OTHERWISE.

NOTE:
MECHANICAL DUCTWORK RUNS THROUGH TRUSSES - COORDINATE DIAGONALS W/ MECH. CONTR.



1/S203 ROOF FRAMING PLAN

1/8" = 1'-0"

NOTES:

1. ALL ELEVATIONS REFERENCED FROM FINISH FLOOR ELEVATION 325.33' (0-0).
2. TRUSS BEARING - T.B. (+46'-0") TYPICAL UNLESS NOTED OTHERWISE.
3. CONTRACT DRAWINGS SHALL NOT BE USED FOR SHOP DRAWINGS.
4. SEE SHEET S001 AND S002 FOR GENERAL NOTES, WALL STUDS AND WALL SHEATHING NOTES.
5. LVL INDICATES MICRO=LAM LVL BY I-LEVEL OR EQUIVALENT.
6. BB INDICATES 8" DEEP BOND BEAM - SEE 5/SJ02.
7. SEE SHEAR WALL SCHEDULE ON SHEET S002 TYPICAL.

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REVISIONS		
No.	Date	Description

KEY PLAN

Shiva Southaven Inc.

Holiday Inn Express & Suites

Lot 16 (Rev Lot 3) Southcrest Pkwy.
Southcrest Subdivision
Southaven, MS 38671

Drawing Title
Roof Framing Plan

Phase
Construction Documents

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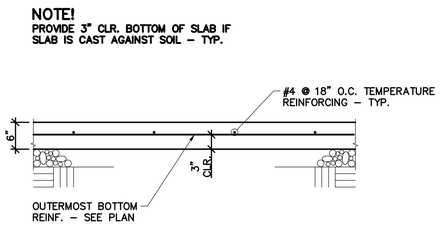
WESLEY T. PATTON REGISTERED PROFESSIONAL ENGINEER 12766 STATE OF MISSISSIPPI
02-27-15

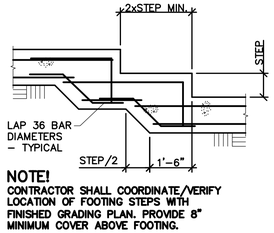
Project No.	14-081	Sheet No.
Prepared by	AEB	S203
Checked by	HLW	
Date	Feb. 27, 2015	

Review

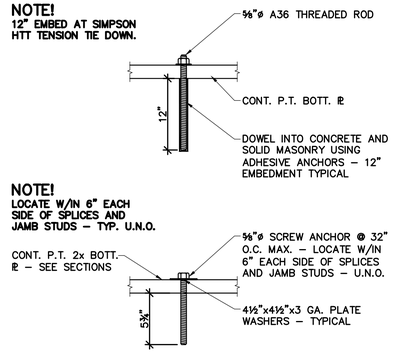
Holiday Inn Express & Suites



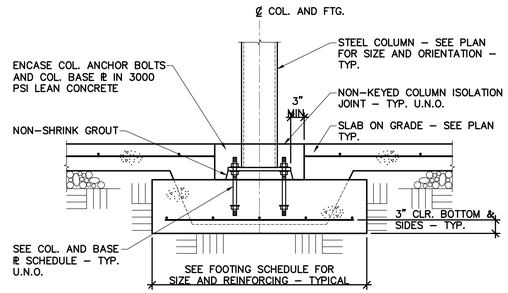
1 S301 TYPICAL 6" SLAB REINFORCING
3/4" = 1'–0"



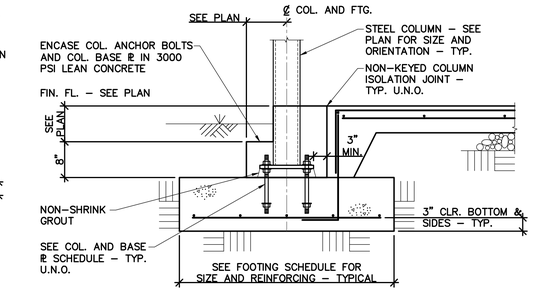
2 S301 TYP. FOOTING STEP
3/4" = 1'–0"



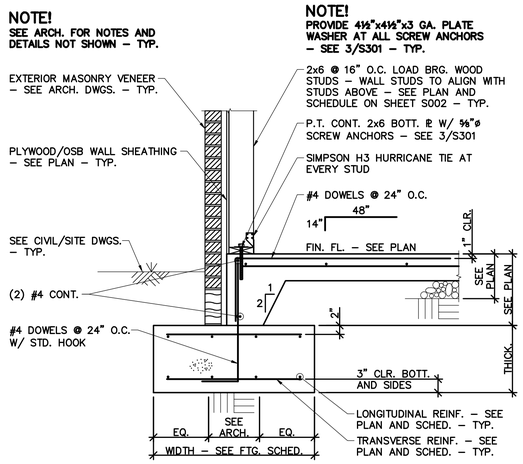
3 S301 SILL PLATE DETAIL
1 1/2" = 1'–0"



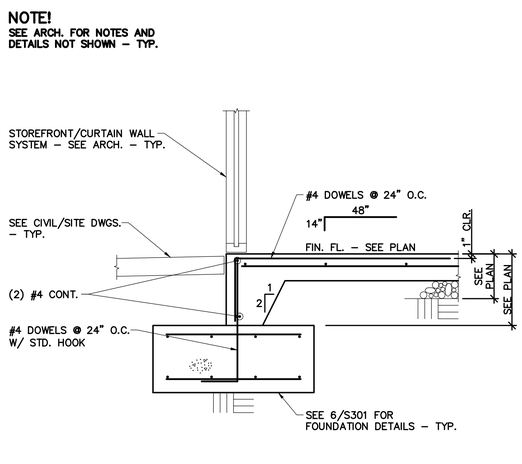
4 S301 SECTION INTERIOR COLUMN FOOTING
3/4" = 1'–0"



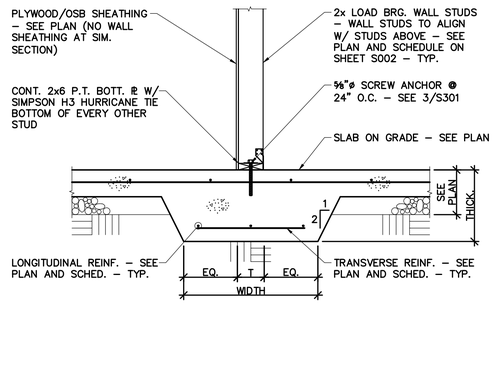
5 S301 SECTION EXTERIOR COLUMN FOOTING
3/4" = 1'–0"



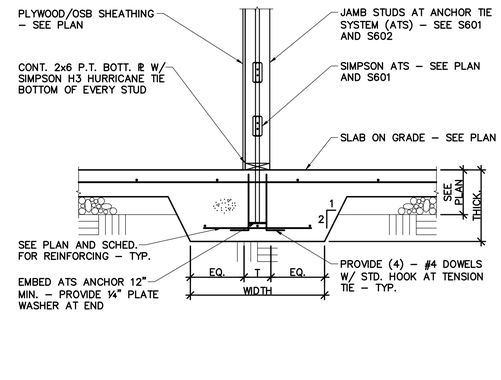
6 S301 EXTERIOR WALL FOOTING
3/4" = 1'–0"



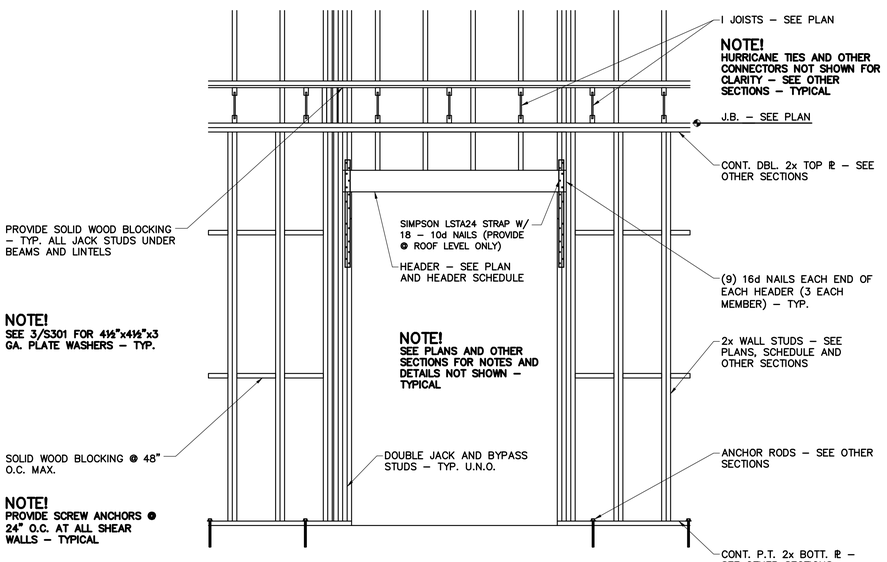
7 S301 EXTERIOR WALL FOOTING
3/4" = 1'–0"



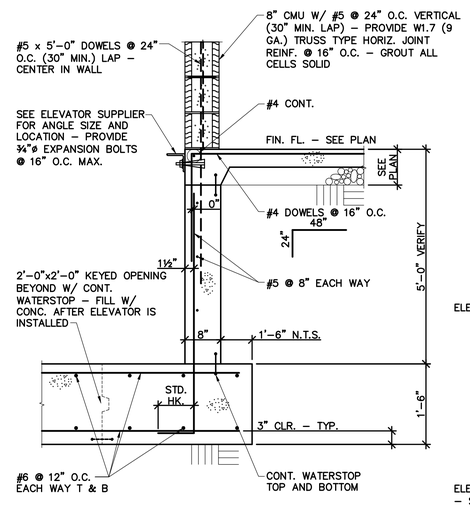
8 S301 THICKENED SLAB AT SHEAR WALL
3/4" = 1'–0"



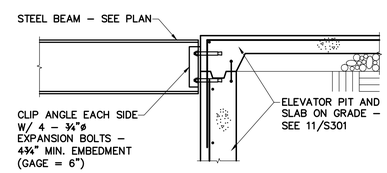
9 S301 THICKENED SLAB AT SHEAR WALL
3/4" = 1'–0"



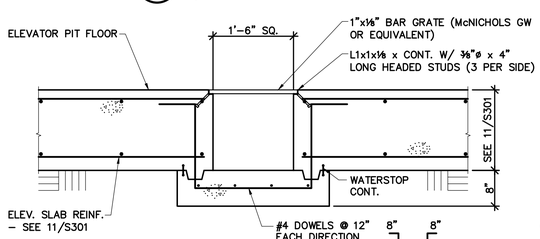
10 S301 ELEVATION LOAD BEARING WOOD STUD WALL
NO SCALE



11 S301 ELEVATOR PIT WALL
3/4" = 1'–0"



12 S301 W12 AT ELEVATOR PIT
3/4" = 1'–0"



13 S301 SUMP PUMP PIT DETAIL
3/4" = 1'–0"

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KEY PLAN

Shiva Southaven Inc.

Holiday Inn Express & Suites

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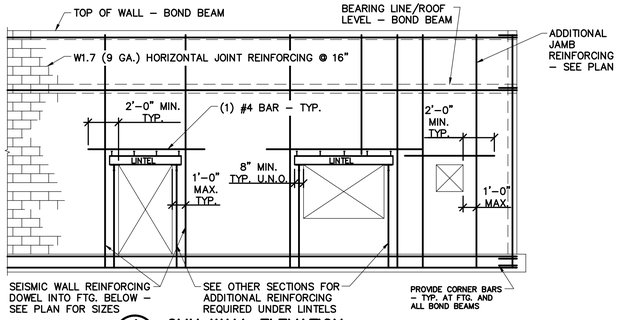
WESLEY T. PATTON REGISTERED PROFESSIONAL ENGINEER 12766 STATE OF MISSISSIPPI
02-27-15

Project No.	14-081
Prepared by	AEB
Checked by	HLW
Date	Feb. 27, 2015
Sheet No.	S301

Review

Holiday Inn Express & Suites

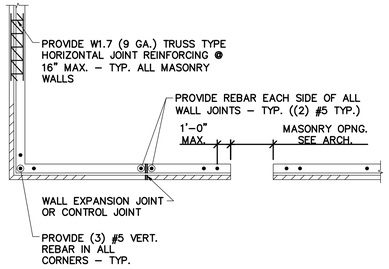


1 S302 CMU WALL ELEVATION

NO SCALE – INTERMEDIATE REINF. CMU WALLS

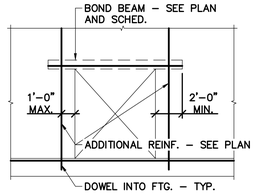
INTERIOR & EXTERIOR SEISMIC MASONRY WALL REQUIREMENTS – (U.N.O.)

1. VERTICAL REBAR (SEE PLAN) DOWELED INTO FOOTING.
2. HORIZONTAL REBAR AT TOP OF WALL, AT BEARING LINES, AND AT TOP OF FOOTING.
3. PROVIDE W1.7 (9 GA.) TRUSS TYPE HORIZONTAL JOINT REINFORCEMENT @ 16" O.C.
4. PROVIDE REBAR (OR LINTEL) AROUND OPENINGS.
5. VERTICAL REBAR REQ'D EA. SIDE OF ALL VERT. WALL JOINTS (CONTROL & EXPANSION JOINTS).
6. LAP VERTICAL REBAR 48 BARø (MIN. LAP 24").
7. GROUT ALL CELLS SOLID WHERE REBAR OCCURS.
8. ALL VERTICAL REBAR TO BE POSITIONED IN BLOCK CELL BY PREFABRICATED WIRE POSITIONER – SUBMIT FOR APPROVAL.
9. SEE BEAM OR LINTEL BEARING DETAILS FOR ADDITIONAL WALL REINFORCING REQUIRED UNDER LINTELS.
10. ALL BRICK TIES SHALL CONFORM WITH SEISMIC DESIGN CATEGORY REQUIREMENTS – SUBMIT FOR APPROVAL.
11. CONSTRUCT ALL WALLS IN 5'–0" LIFTS.
12. SEE OTHER SECTIONS FOR NOTES AND DETAILS NOT SHOWN.



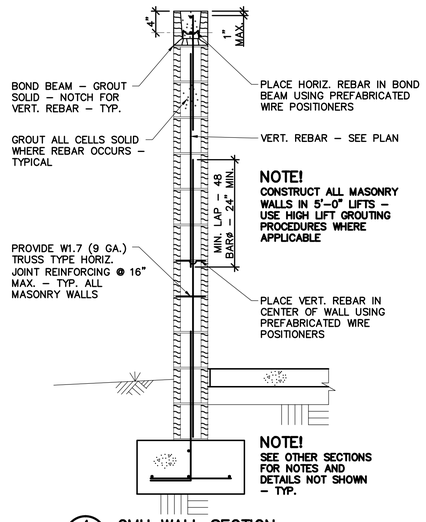
2 S302 CMU WALL REINFORCING

NO SCALE



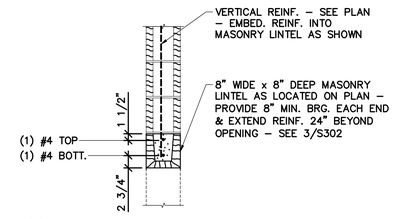
3 S302 CMU BOND BEAM ELEVATION

NO SCALE



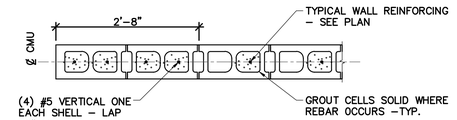
4 S302 CMU WALL SECTION

NO SCALE



5 S302 8" DEEP MASONRY LINTEL – 8BB

3/4" = 1'–0"



6 S302 8" CMU SHEAR WALL JAMB REINFORCING

3/4" = 1'–0"

NOTE!
SEE ARCH. DWGS. FOR NOTES AND DETAILS NOT SHOWN – TYPICAL

7 S302 EXTERIOR WALL FOOTING

3/4" = 1'–0"

8 S302 THICKENED SLAB AT INTERIOR WALL

3/4" = 1'–0"

9 S302 SECTION AT WASHING MACHINE PAD

3/4" = 1'–0"

10 S302 CANOPY COLUMN FOOTING

NO SCALE

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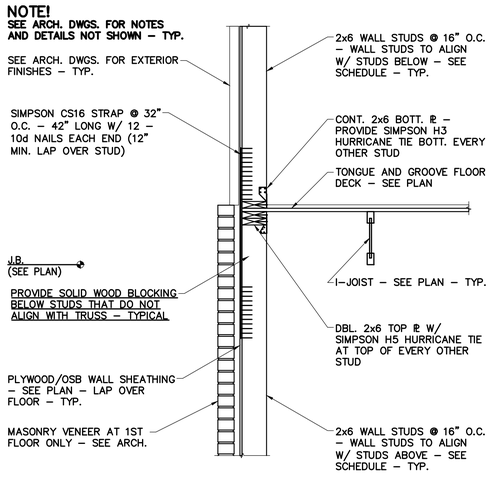
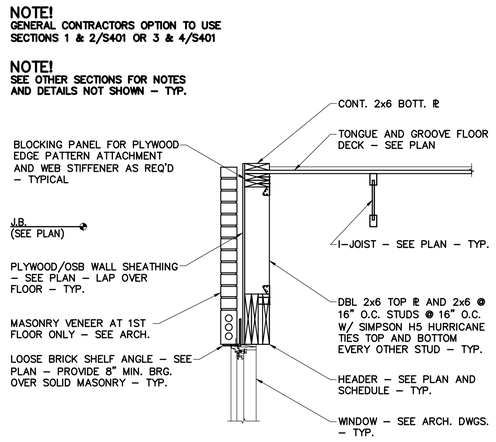
Holiday Inn Express & Suites



1 S401 SECTION AT FLOOR
3/4" = 1'–0"



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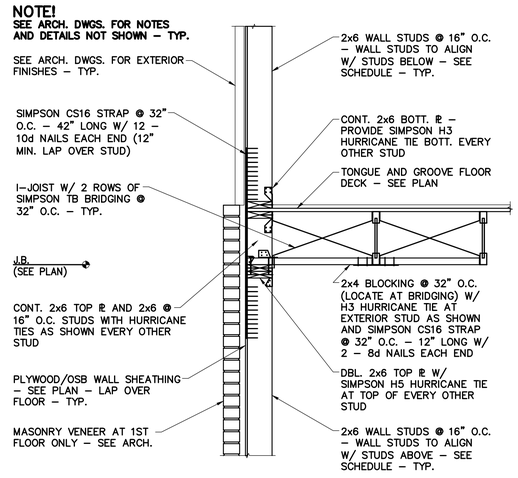

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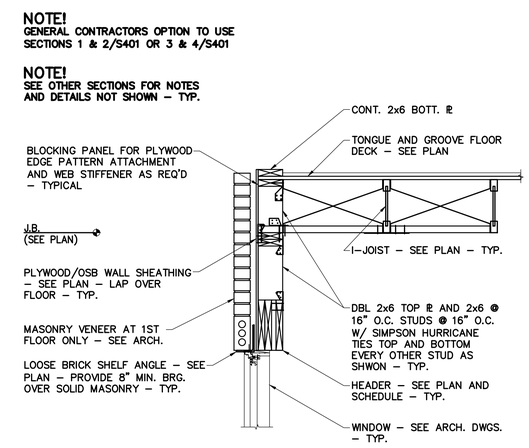

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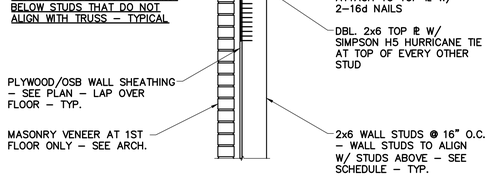

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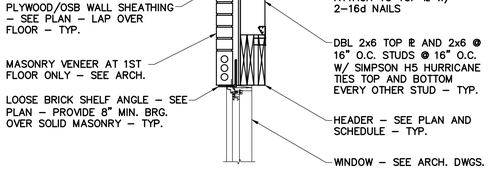
3/4" = 1'–0"



6 S401 SECTION AT FLOOR
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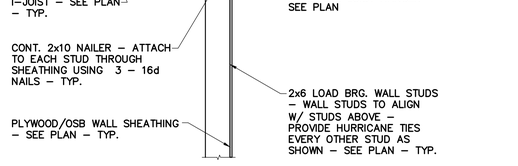
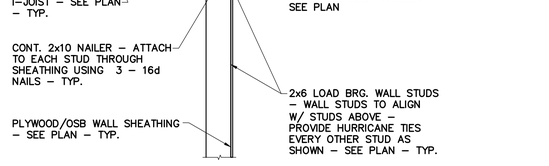
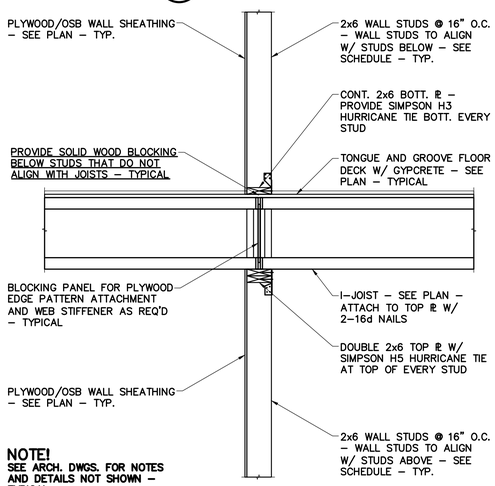


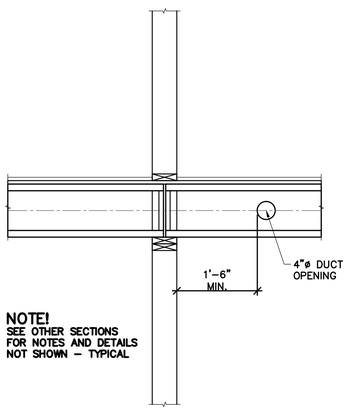
7 S401 SECTION AT CORRIDOR
3/4" = 1'–0"



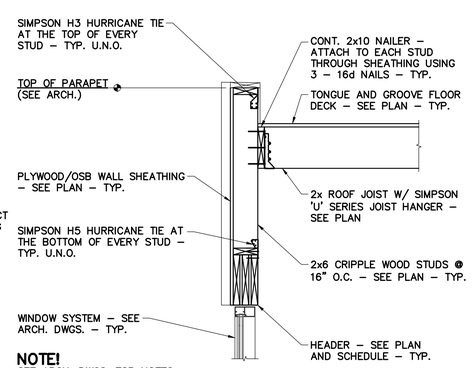
8 S401 SECTION AT CORRIDOR
3/4" = 1'–0"



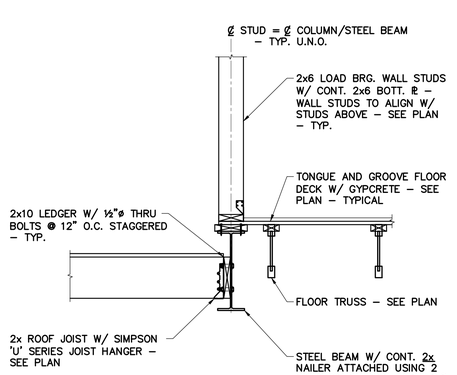
9 S401 INTERIOR LOAD BEARING SHEAR WALL
3/4" = 1'–0"



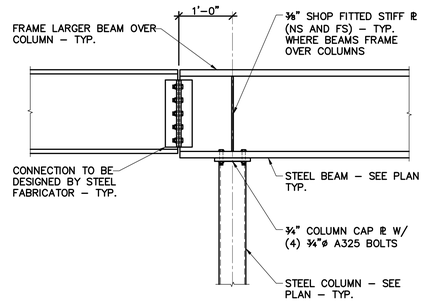
10 S401 HOLE IN I–JOIST
3/4" = 1'–0"



11 S401 LOW ROOF FRAMING
3/4" = 1'–0"



12 S401 SECTION AT STEEL BEAM
3/4" = 1'–0"



13 S401 BEAM CANTILEVERING OVER COLUMN
3/4" = 1'–0"

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Project No. 14-081	Sheet No. S401
Prepared by AEB	
Checked by HLW	
Date Feb. 27, 2015	

Review

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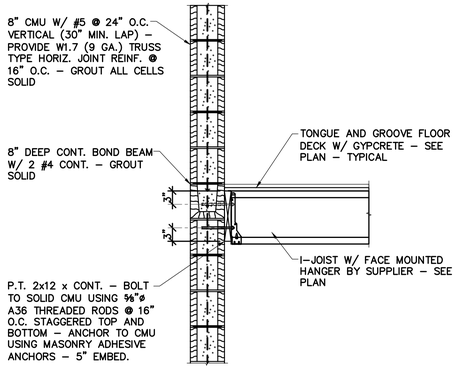
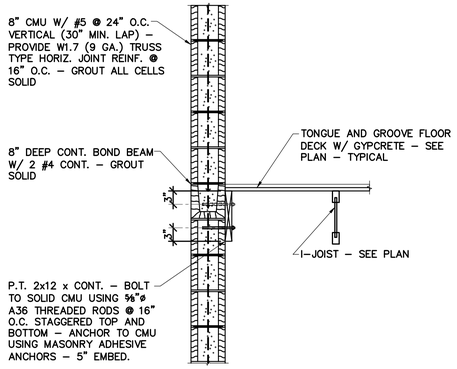
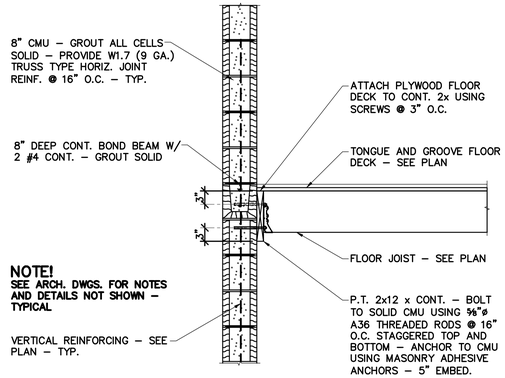


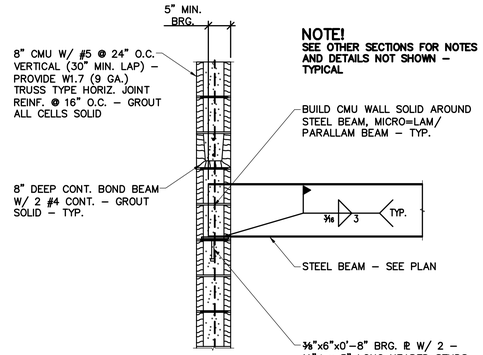
1 S402 **SECTION AT ELEVATOR/STAIR**
3/4" = 1'–0"



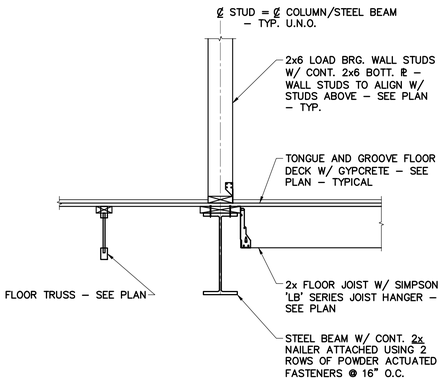
2 S402 **SECTION AT ELEVATOR/STAIR**
3/4" = 1'–0"



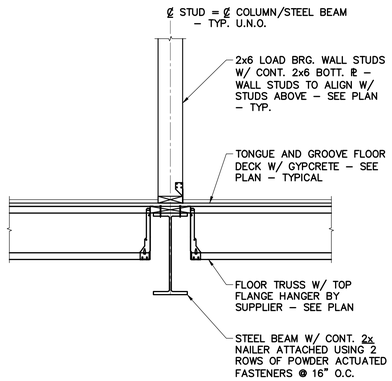
3 S402 **SECTION AT ELEVATOR/STAIR**
3/4" = 1'–0"



4 S402 **STEEL BEAM AT CMU WALL**
3/4" = 1'–0"



5 S402 **SECTION AT STEEL BEAM**
3/4" = 1'–0"



6 S402 **SECTION AT STEEL BEAM**
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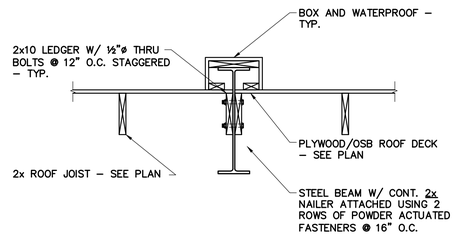

7 S402 **SECTION AT STEEL BEAM**
3/4" = 1'–0"

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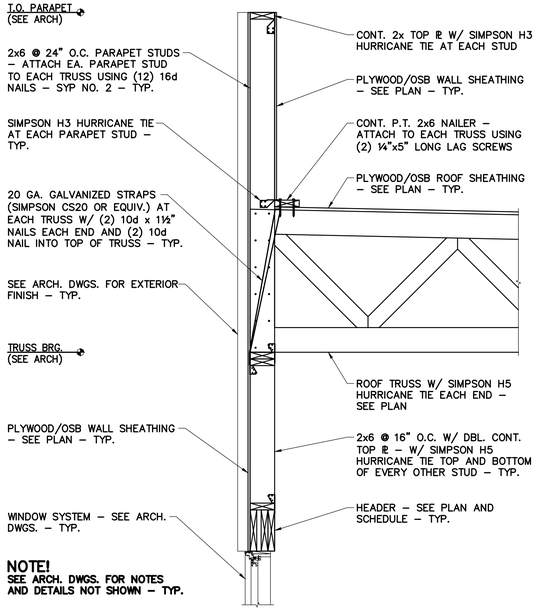
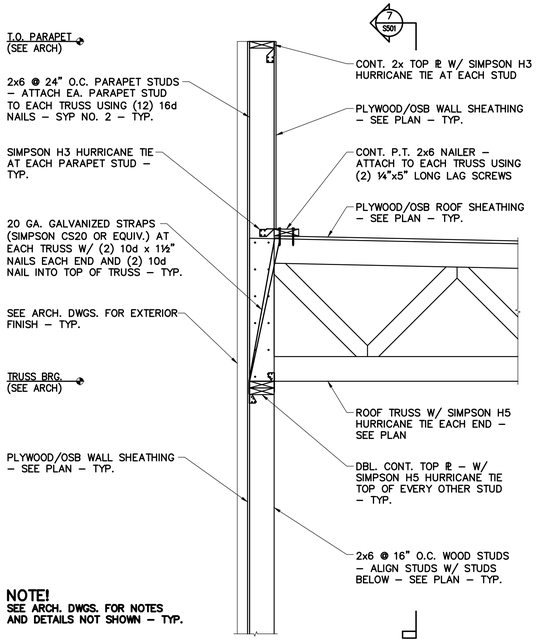
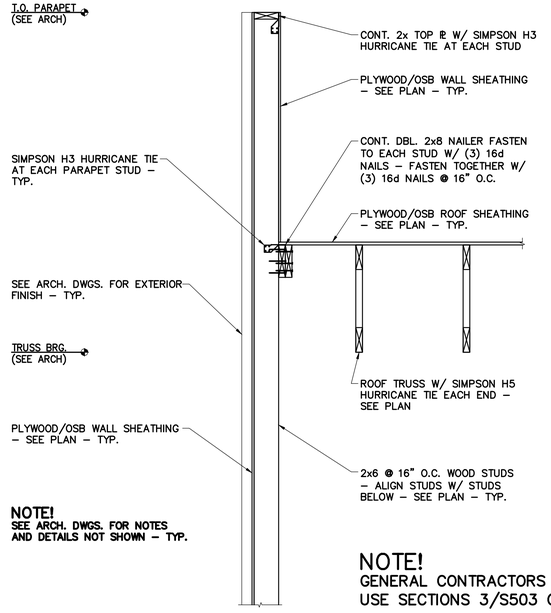


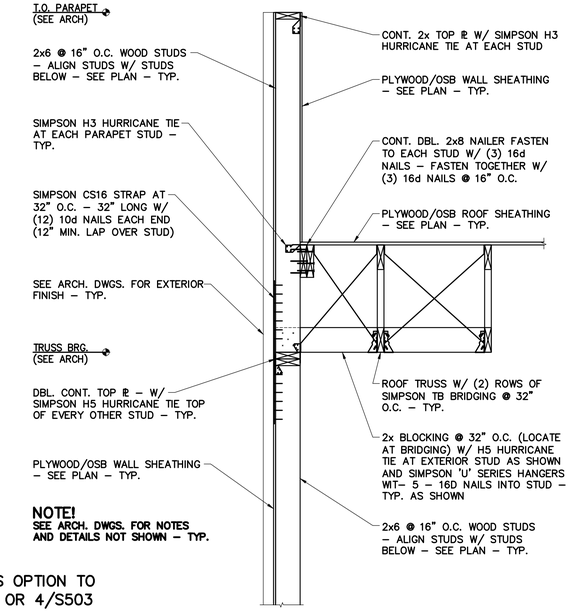
1 S501 SECTION AT LOW PARAPET
3/4" = 1'-0"



2 S501 SECTION AT LOW PARAPET
3/4" = 1'-0"

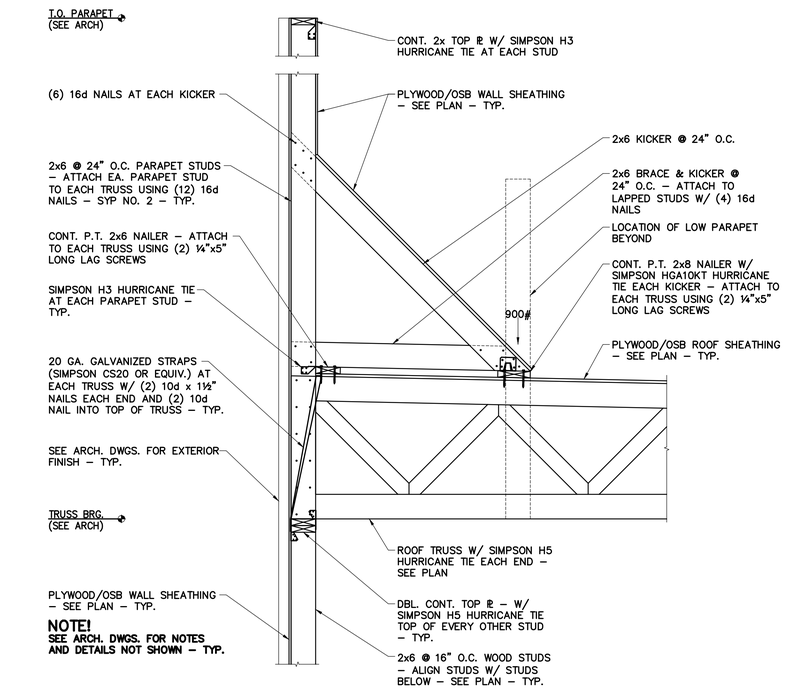


3 S501 SECTION AT LOW PARAPET
3/4" = 1'-0"

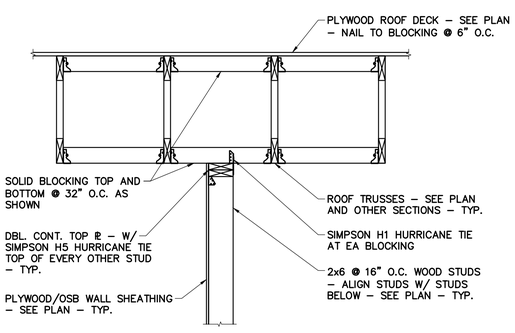


NOTE!
GENERAL CONTRACTORS OPTION TO USE SECTIONS 3/S503 OR 4/S503

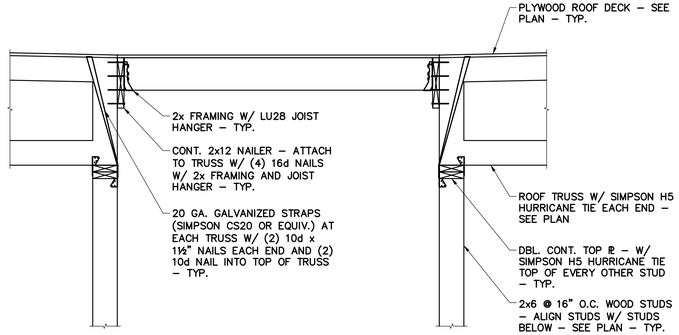
4 S501 SECTION AT LOW PARAPET
3/4" = 1'-0"



5 S501 SECTION AT HIGH PARAPET AND KICKER
3/4" = 1'-0"



6 S501 SECTION AT SHEAR WALL
3/4" = 1'-0"



8 S501 SECTION AT CORRIDOR WALL
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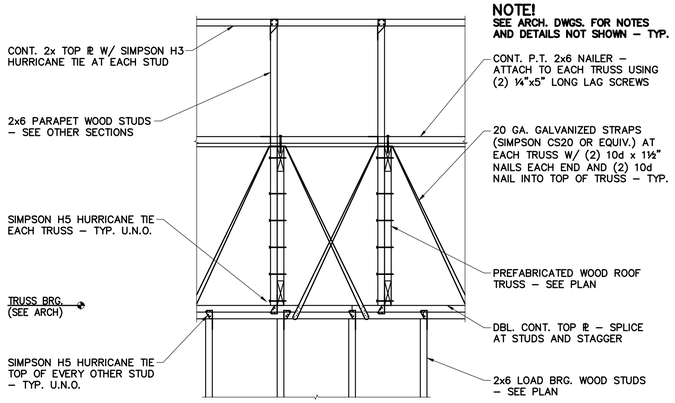

7 S501 ELEVATION STUDS AND TRUSSES
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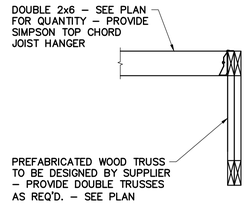

9 S501 SECTION AT RTU
3/4" = 1'-0"

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MEP:
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REVISIONS		
No.	Date	Description

KEY PLAN

Shiva Southaven Inc.

Holiday Inn Express & Suites

Lot 16 (Rev Lot 3) Southcrest Pkwy.
Southcrest Subdivision
Southaven, MS 38671

Drawing Title
Roof Framing Sections and Details

Phase
Construction Documents

WGPM, Inc.
Wright - Gibson - Patton
STRUCTURAL ENGINEERING
11220 Elm Lane, Suite 201
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704-542-7199 Fax: 704-542-7195
www.wgpminc.com
JOB NUMBER: 128-14

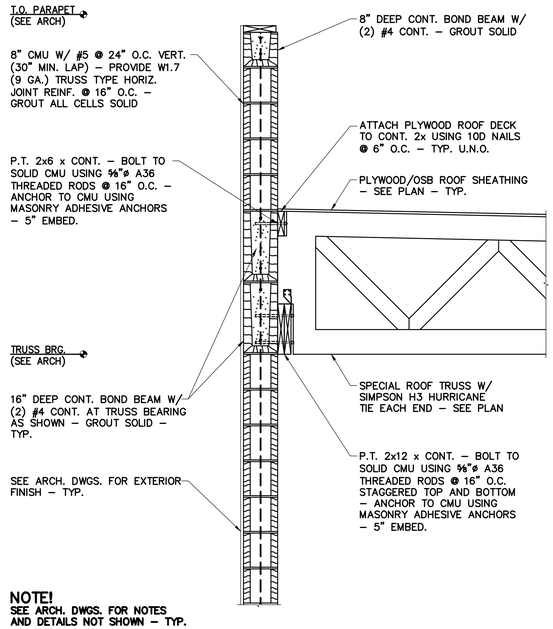
WESLEY T. PATTON REGISTERED PROFESSIONAL ENGINEER 12766 STATE OF MISSISSIPPI
02-27-15

Project No.	14-081
Prepared by	AEB
Checked by	HLW
Date	Feb. 27, 2015

Sheet No. S501

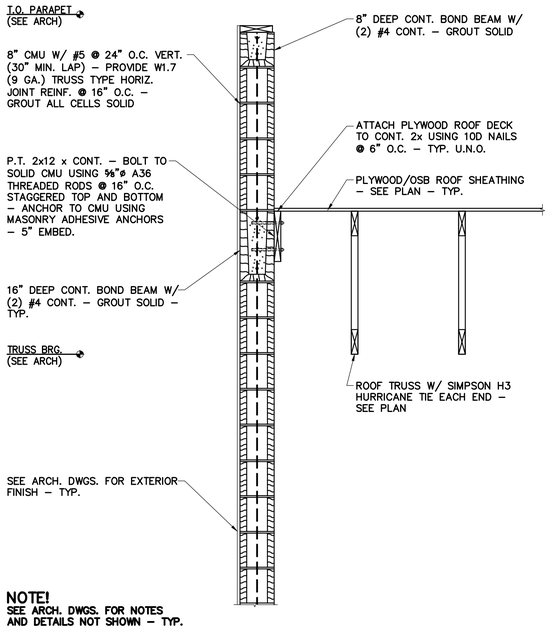
Review

Holiday Inn Express & Suites



NOTE!
SEE ARCH. DWGS. FOR NOTES AND DETAILS NOT SHOWN – TYP.

1 S502 SECTION AT LOW PARAPET
3/4" = 1'–0"



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2 S502 SECTION AT LOW PARAPET
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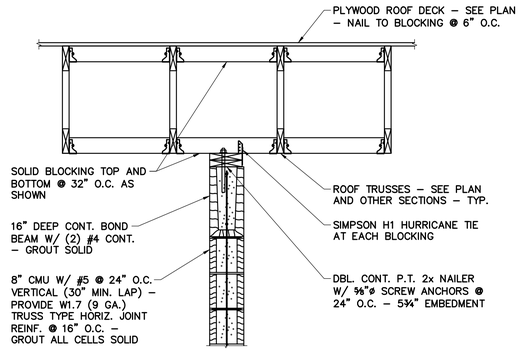
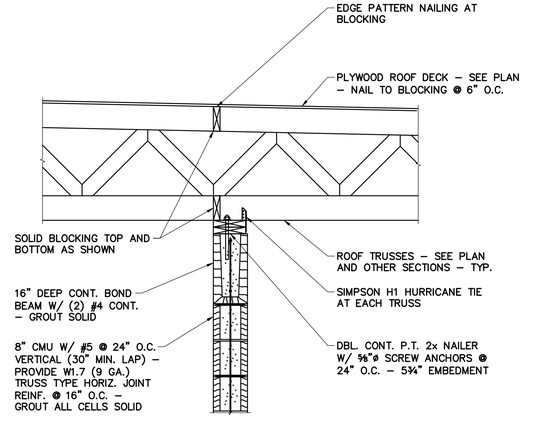
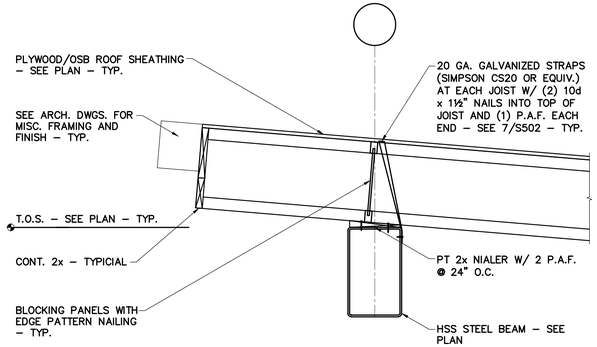


3 S502 SECTION AT SHEAR WALL
3/4" = 1'–0"



4 S502 SECTION AT SHEAR WALL
3/4" = 1'–0"



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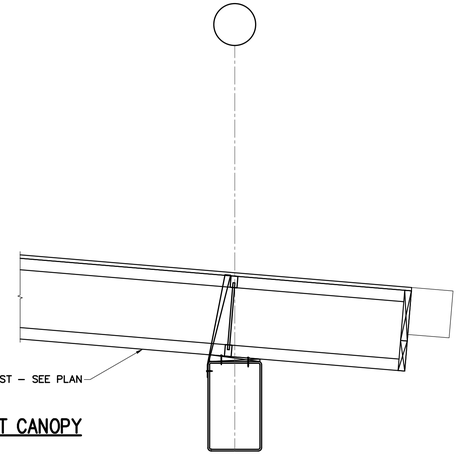

5 S502 SECTION AT CANOPY
3/4" = 1'–0"

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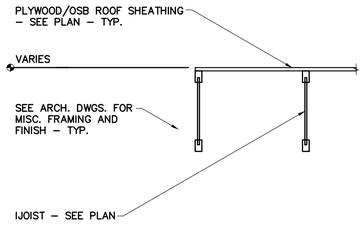
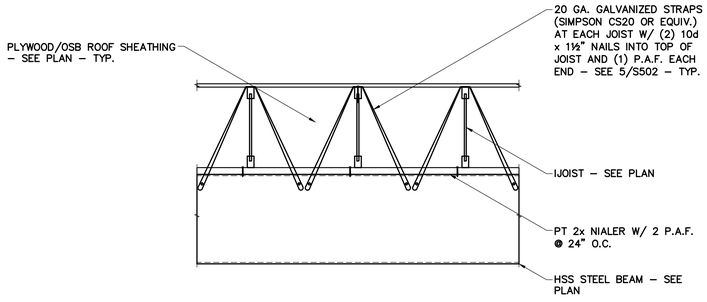


6 S502 SECTION AT CANOPY
3/4" = 1'–0"



7 S502 SECTION AT CANOPY
3/4" = 1'–0"

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Project No.	14-081	Sheet No.
Prepared by	AEB	S502
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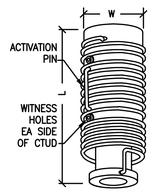
Date	Feb. 27, 2015	

Review

Holiday Inn Express & Suites

SEE S0.1 FOR NOTES

1 GENERAL NOTES AND CONDITIONS OF USE



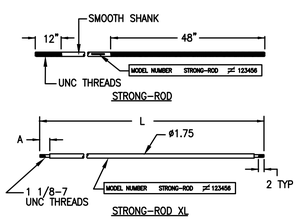
CTUD

MODEL NO.	THREADED ROD SIZE (IN)	W (IN)	L (IN)	COMPENSATION (IN)
ATS-CTUD55	5/8-5/8	1 7/8	5	1
ATS-CTUD65	3/4-5/8	2	5 1/2	
ATS-CTUD66	3/4-3/4	2	5 1/2	
ATS-CTUD75	7/8-5/8	2	5 1/2	
ATS-CTUD76	7/8-3/4	2	5 1/2	
ATS-CTUD77	7/8-7/8	2	5 1/2	
ATS-CTUD87	1-7/8	2 1/2	6 1/8	
ATS-CTUD88	1-1	2 1/2	6 1/8	
ATS-CTUD97	1 1/8-7/8	2 1/2	6 1/8	
ATS-CTUD98	1 1/8-1	2 1/2	6 1/8	
ATS-CTUD99	1 1/8-1 1/8	2 1/2	6 1/8	

INSTALLATION
1. THREAD THE CTUD ONTO THE THREADED ROD BELOW, POSITIONING IT WITH ACTIVATION PIN FACING OUT.
2. THREAD THE ROD ABOVE INTO THE CTUD. RODS ARE INSTALLED CORRECTLY WHEN VISIBLE THROUGH THE WITNESS HOLES. DO NOT REMOVE ACTIVATION PIN UNTIL THE ISOLATOR NUT (IN) IS INSTALLED AT LEVEL ABOVE.
3. SECURE ROD AT LEVEL ABOVE WITH BEARING PLATE AND ISOLATOR NUT. ATTACH ISOLATOR NUT TO SOLE PLATE WITH PROVIDED SCREW.
4. REMOVE ACTIVATION PIN AFTER LEVEL ABOVE IS SECURED WITH ISOLATOR NUT.
5. THE CTUD MAY BE INSTALLED IN EITHER DIRECTION.

2 COUPLING TAKE UP DEVICE (CTUD)

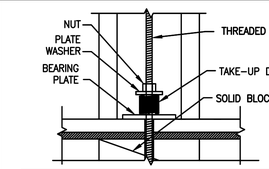
ROD DIA. IN.	FULL THREAD MODEL NO.	STRONG-ROD MODEL NO.
1/2	R4/ATR½	–
5/8	R5/ATR⅝	–
3/4	R6/ATR¾	–
7/8	R7/ATR⅞	–
1	R8/ATR1	–
1 1/8	R9/ATR1 ⅛	–
1 1/4	R10/ATR1 ¼	–
1 3/8	R11	–
1 1/2	R12	–
1 3/4	R14	–
2	R16	–
5/8	HSR5	SR5H
3/4	HSR6	SR6H
7/8	HSR7	SR7H
1	HSR8	SR8H
1 1/8	HSR9	SR9H
1 1/4	HSR10	SR10H
1 3/8	HSR11	–
1 1/2	HSR12	–
1 3/4	HSR14	–
2	HSR16	–
1 1/8	HSSR9H150	SR9H150
1 1/4	HSSR10H150	SR10H150



STRONG-ROD XL MODEL NO.	ROD DIA. IN.	A	L	COLOR CODE
SR9H/14-5	1 3/4-1 1/8	6"	6'-0"	ORANGE
SR9H/14-7	1 3/4-1 1/8	2 1/2"	7'-5"	
SR9H150/14-7	1 3/4-1 1/8	2 1/2"	7'-5"	

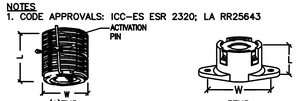
NOTES
1. FULLY THREADED AND STRONG-ROD IS UNCOATED OR PLAIN BLACK STEEL.
2. STRONG-ROD MODEL DESIGNATION: THE NUMBER FOLLOWING ATS-SR(#) REPRESENTS THE ROD DIAMETER IN 1/8" (i.e. ATS-SR5H IS A 5/8" DIAMETER HIGH STRENGTH ROD).
3. STRONG-ROD XL ARE 1 3/4" DIA WITH 1 1/8" THREADED ENDS AND ARE USED TO REDUCE ELONGATION.

3 ROD



MODEL NO.	MAXIMUM THREADED ROD SIZE (IN)	W (IN)	L (IN)	COMPENSATION (IN)
ATUD5	5/8	1 3/8	1 7/8	3/4
ATUD6-2	3/4	1 3/4	3 1/8	2
ATUD9	1 1/8	2 1/8	2 1/4	1
TUD9	1 1/8	2 1/8	2 1/4	1
ATUD9-2	1 1/8	2 1/8	3 7/8	2
TUD10	1 1/4	2 3/8	2 1/4	1
ATUD14	1 3/4	2 7/8	2 1/4	3/4
RTUD3	3/8	1 7/8	15/16	NO LIMIT
RTUD4	1/2	1 7/8	15/16	NO LIMIT

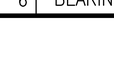
NOTES
1. CODE APPROVALS: ICC-ES ESR 2320; LA RR25643



INSTALLATION (TUD OR ATUD)
1. PLACE TAKE-UP DEVICE OVER ROD FROM LEVEL BELOW ONTO APPROPRIATE BEARING PLATE.
2. INSTALL PLATE WASHER ON TOP OF TUD OR ATUD, AND SECURE THE NUT ON TOP BY FINGER TIGHTENING PLUS AN ADDITIONAL 1/2 TO 1/3 TURN WITH WRENCH.
3. REMOVE ACTIVATION PIN.

INSTALLATION (RTUD)
1. RTUD MUST BE INSTALLED ON TOP OF BPRTUD BEARING PLATE AND FASTENED THROUGH THE BEARING PLATE TO WOOD PLATES WITH (2)8dX 1 ½ MIN NAILS OR (2) #9X1 ½ STRONG-DRIVE® SD SCREWS

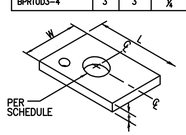
4 TAKE UP DEVICE (TUD)


ISOLATOR NUT

MODEL NO.	ROD DIA. (IN)	WASHER O.D. (IN)
ATS-IN5KT	5/8	3 1/8
ATS-IN6KT	3/4	
ATS-IN7KT	7/8	
ATS-IN8KT	1	
ATS-IN9KT	1 1/8	

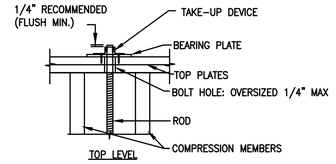
5 ISOLATOR NUT DETAIL

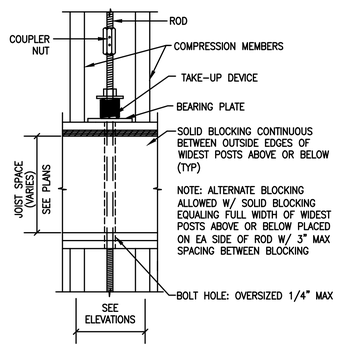
MODEL	ATS PLATE DIMENSIONS W (IN)	L (IN)	t (IN)	D hole (IN)	COMPATIBLE TAKE-UP DEVICE SERIES
ATS-BP5-3x3.5	3	3 ½	⅜	1⅛	CTUD
ATS-BP5-3x5.5	3	5 ½	⅜	1⅛	
ATS-BP5-5x5.5	5	5 ½	¾	1⅛	
ATS-BP6-3x5.5	3	5 ½	⅜	1⅜	
ATS-BP6-5x5.5	5	5 ½	⅜	1⅜	
ATS-BP7-3x5.5	3	5 ½	⅜	1⅝	
ATS-BP7-5x5.5	5	5 ½	⅜	1⅝	
ATS-BP8-3x5.5	3	5 ½	½	1 ⅛	
ATS-BP8-5x5.5	5	5 ½	½	1 ⅛	
ATS-BP9-3x5.5	3	5 ½	⅜	1 ⅜	
ATS-BP9-5x5.5	5	5 ½	⅜	1 ⅜	
ATS-BP10-3x5.5	3	5 ½	½	1 ⅝	
ATS-BP10-5x5.5	5	5 ½	½	1 ⅝	
ATS-BP10-3x9	3	9	1	1 ⅝	
ATS-BP10-3x12	3	12	1 ½	1 ⅝	
ATS-BP10-3x15	3	15	1 ⅜	1 ⅝	
ATS-BP10-5x9	5	9	1	1 ⅝	
ATS-BP10-5x12	5	12	1 ½	1 ⅝	
BPRTUD3-4	3	3	¼	⅝	RTUD

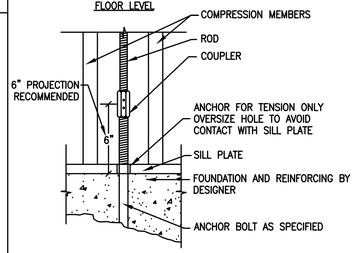


6 BEARING PLATES FOR CTUD

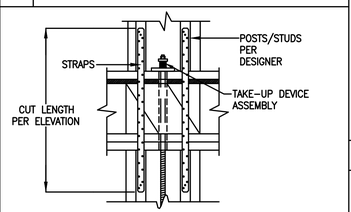
NOTE: NO SPLICE IN TOP PLATE IS PERMITTED WITHIN 8" OF ATS ROD







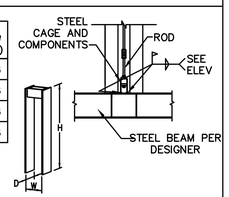
7 INSTALLATION DETAILS



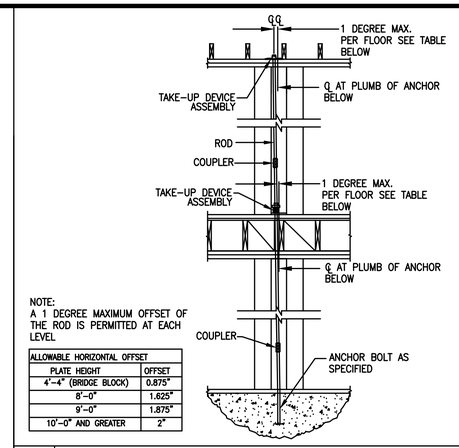
1. VERTICAL STRAPS ARE SOLD SEPARATELY, UNO.
2. WHEN STRAPS OCCUR, REFER TO THE STRUCTURAL DRAWINGS AND INSTALLATION DETAILS.
3. REFER TO THE APPLICABLE CODE FOR MINIMUM NAIL EDGE AND END DISTANCES.

8 ALT. STRAP TRANSITION

MODEL	D (IN)	H (IN)	W (IN)
ATS-1/2-2B	2	16	3
ATS-1/2-2C	2	16	3
ATS-1/2-2D	3	16	3
ATS-1/2-2E	3	16	3



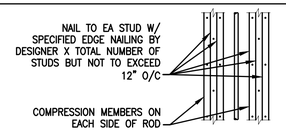
9 WELDABLE CAGES



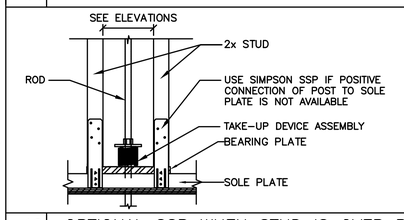
NOTE:
A 1 DEGREE MAXIMUM OFFSET OF THE ROD IS PERMITTED AT EACH LEVEL

ALLOWABLE HORIZONTAL OFFSET	
PLATE HEIGHT	OFFSET
4'-4" (BRIDGE BLOCK)	0.875"
8'-0"	1.625"
9'-0"	1.875"
10'-0" AND GREATER	2"

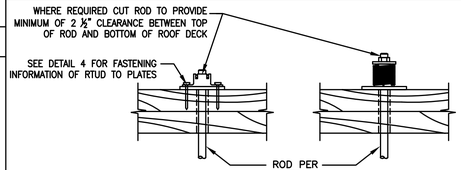
10 ALLOWABLE ROD OFFSET



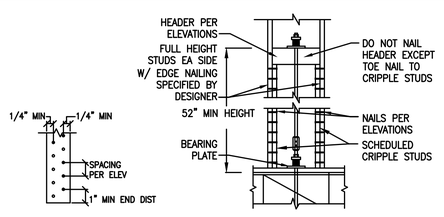
11 SHEARWALL EDGE NAILING @ BUILT UP POSTS



12 OPTIONAL SSP WHEN STUD IS OVER PLATE



13 TAKE-UP DEVICE INSTALLATION AT TOP LEVEL



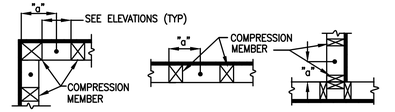
BRIDGE BLOCK INSTALLATION DETAILS

BRIDGE BLOCK NOTES
1. CRIPPLE STUDS SHALL BE 2X MEMBERS ONLY AND BE FIELD NAILED.
2. CRIPPLE STUDS SHALL BE NAILED TO FULL HEIGHT STUDS PER ELEVATION DRAWINGS AND DETAILS ABOVE. IF (2) CRIPPLE STUDS ARE REQUIRED ON EACH SIDE OF ROD, NAIL CRIPPLE ADJACENT TO FULL HEIGHT STUD WITH SCHEDULED NAILS AND NAIL INNER CRIPPLE WITH ½ OF THE SCHEDULED NAILS.
3. CRIPPLE STUDS MAY BE NAILED TO FULL HEIGHT STUDS FROM EITHER SIDE.
4. IF 6" HEADER IS SOLID SAWN, IT MUST BE A MINIMUM #1 GRADE. SCL IS ALSO ACCEPTABLE.
5. FOR SDS SCREW OPTIONS, CONTACT SIMPSON STRONG-TIE.

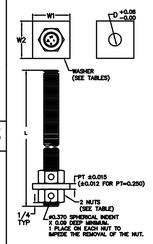
14 ALT. BRIDGE BLOCK DETAIL

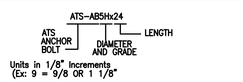
ANCHOR BOLT LAYOUT		
POST SIZE	"a" W/ 6" SPACING BETWEEN STUDS	"a" W/ 9" SPACING BETWEEN STUDS
2x4 or 2x6	4 1/2"	6"
3x4 or 3x6	5 1/2"	7"
2-2x4 or 2-2x6	6"	7 1/2"
2-3x4 or 2-3x6	8"	9 1/2"
4x6 or 6x6	8 1/2"	10"
4x8	10 1/4"	11 3/4"
6x8	10 1/2"	12"
4x10	12 1/4"	13 3/4"
6x10	12 1/2"	14"

1. COMPRESSION MEMBERS DO NOT INCLUDE TRIMMERS.
2. LOCATE ANCHOR BOLT W/ NARROWEST COMPRESSION MEMBER.
3. WHEN SHEARWALLS MEET IN A CORNER AND SHARE A COMMON CORNER POST, THE POST SHALL BE THE LARGER OF THAT SPECIFIED FOR EACH INDIVIDUAL SHEARWALL.



15 ANCHOR BOLT LAYOUT

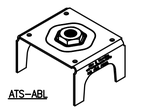




MODEL	ROD DIA (IN)	NUTS	WASHER SIZE OR COMPONENT PT	W1	W2	D
ATS-AB4 PAB4	1/2	ATS-N4	0.250	1.25	1.25	0.562
ATS-AB5 PAB5	5/8	ATS-N5	0.375	1.50	1.50	0.688
ATS-AB6 PAB6	3/4	ATS-N6	0.375	2.00	2.00	0.812
ATS-AB7 PAB7	7/8	ATS-N7	0.375	2.25	2.25	0.938
ATS-AB8 PAB8	1	ATS-N8	0.375	2.50	2.50	1.062
ATS-AB9 PAB9	1 1/8	ATS-N9	0.375	2.75	2.75	1.188
ATS-AB10 PAB10	1 1/4	ATS-N10	0.500	3.00	3.00	1.312

ATS-AB NOTES
1. ATS ANCHOR BOLTS AVAILABLE IN 5/8" THRU 1 1/4" DIA IN STANDARD AND HIGH STRENGTH.
2. ANCHOR BOLT SIZE AND LENGTH BY DESIGNER.
3. LENGTHS AVAILABLE IN 18", 24", 30", AND 36".

16 ANCHOR BOLTS

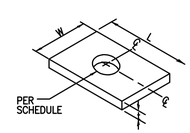

ATS-ABL

MODEL NO.	ANCHOR BOLT DIAMETER
ABL4-1	1/2"
ABL5-1	5/8"
ABL6-1	3/4"
ABL7-1	7/8"
ABL8-1	1"
ABL9-1	1 1/8"
ABL10-1	1 1/4"

ATS-ABL NOTES AND INSTALLATION
1. MODELS SHOWN PROVIDE 1" STANDOFF.
2. MODELS WITH OST NUTS AVAILABLE FOR USE WITH HDG RODS.
3. ATTACH ABL TO FRAMEWORK WITH (2) MIN NAILS OR SCREWS.
4. ATTACH THE UPPER NUT AND PLATE WASHER TO THREADED ROD.
5. THREAD THE ROD INTO THE ABL UNTIL IT BOTTOMS OUT ON THE PLATE WASHER AND IS FULLY ENGAGED WITH BOTTOM NUT.

17 ANCHOR BOLT LOCATOR

MODEL	ATS PLATE DIMENSIONS W (IN)	L (IN)	t (IN)	D hole (IN)	COMPATIBLE TAKE-UP DEVICE SERIES
PLS-3X3.5	3	3 ½	⅜	1⅛	ATUD/TUD
PLS-3X5.5	3	5 ½	⅜	1⅛	
PLS-3X5.5	3	3 ½	⅜	1⅜	
PLS-3X5.5	3	5 ½	⅜	1⅜	
PLS-3X5.5	3	5 ½	½	1 ⅛	
PLS-3X8.5	3	8 ½	½	1 ⅜	
PL14-3X8.5	3	8 ½	½	1 ⅜	
PLS-3X12	3	12	1 ½	1 ⅜	
PL14-3X12	3	12	1 ½	1 ⅜	
PLS-3X15	3	15	1 ½	1 ⅜	
PL10-3X15	3	15	1 ½	1 ⅜	
PL14-3X15	5	15	1 ½	1 ⅜	
PL10-5X5.5	5	5 ½	½	1 ⅜	
PL14-5X5.5	5	5 ½	½	1 ⅜	
PL10-5X8.5	5	8 ½	½	1 ⅜	
PL14-5X8.5	5	8 ½	½	1 ⅜	
PLS-5X12	5	12	1 ½	1 ⅜	
PL10-5X12	5	12	1 ½	1 ⅜	
BPRTUD3-4	3	3	¼	⅝	RTUD



18 BEARING PLATES FOR ATUD/TUD AND RTUD

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REVISIONS		
No.	Date	Description

KEY PLAN

Shiva Southaven Inc.

Holiday Inn Express & Suites

Lot 16 (Rev Lot 3) Southcrest Pkwy.
Southcrest Subdivision
Southaven, MS 38671

Drawing Title
Simpson ATS Details

Phase
Construction Documents

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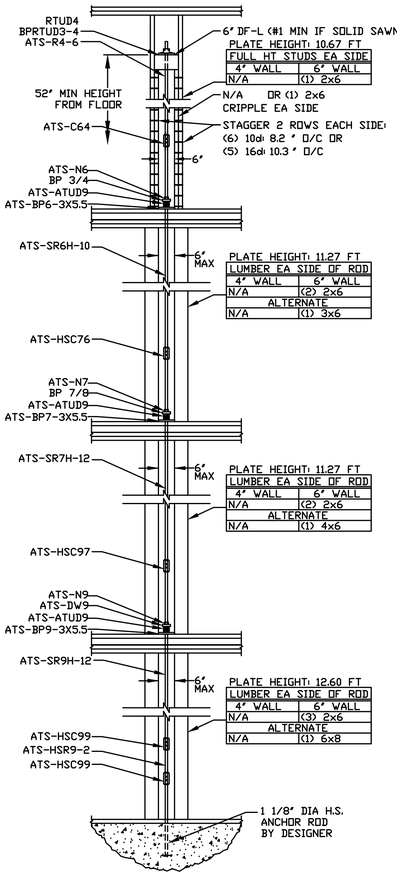
JOB NUMBER: 128-14

WESLEY T. PATTON REGISTERED PROFESSIONAL ENGINEER 12766 STATE OF MISSISSIPPI
02-27-15

Project No.	14-081	Sheet No.
Prepared by	AEB	S601
Checked by	HLW	
Date	Feb. 27, 2015	

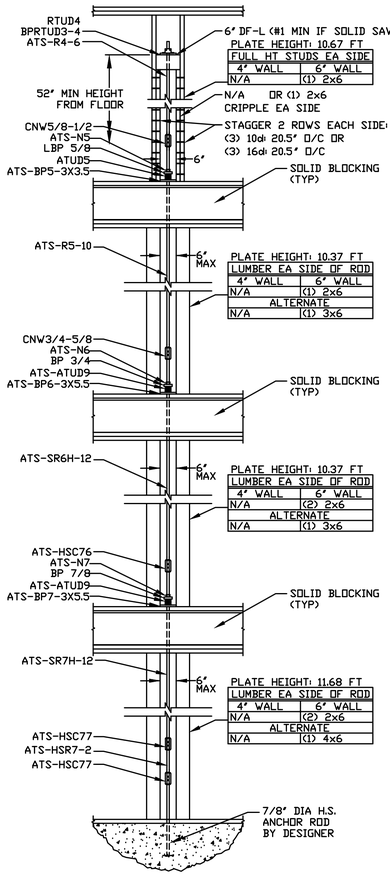
Review

Holiday Inn Express & Suites



SEE ALTERNATE BRIDGE BLOCK DETAIL
ON S601 FOR ADDITIONAL INFORMATION

1 ATS ELEVATION – SW1



SEE ALTERNATE BRIDGE BLOCK DETAIL
ON S601 FOR ADDITIONAL INFORMATION

2 ATS ELEVATION – SW2

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REGISTERED PROFESSIONAL ENGINEER
12746
STATE OF MISSISSIPPI
02-27-15

Project No.	14-081	Sheet No.
Prepared by	AEB	S602
Checked by	HLW	
Date	Feb. 27, 2015	

Review

Holiday Inn Express & Suites