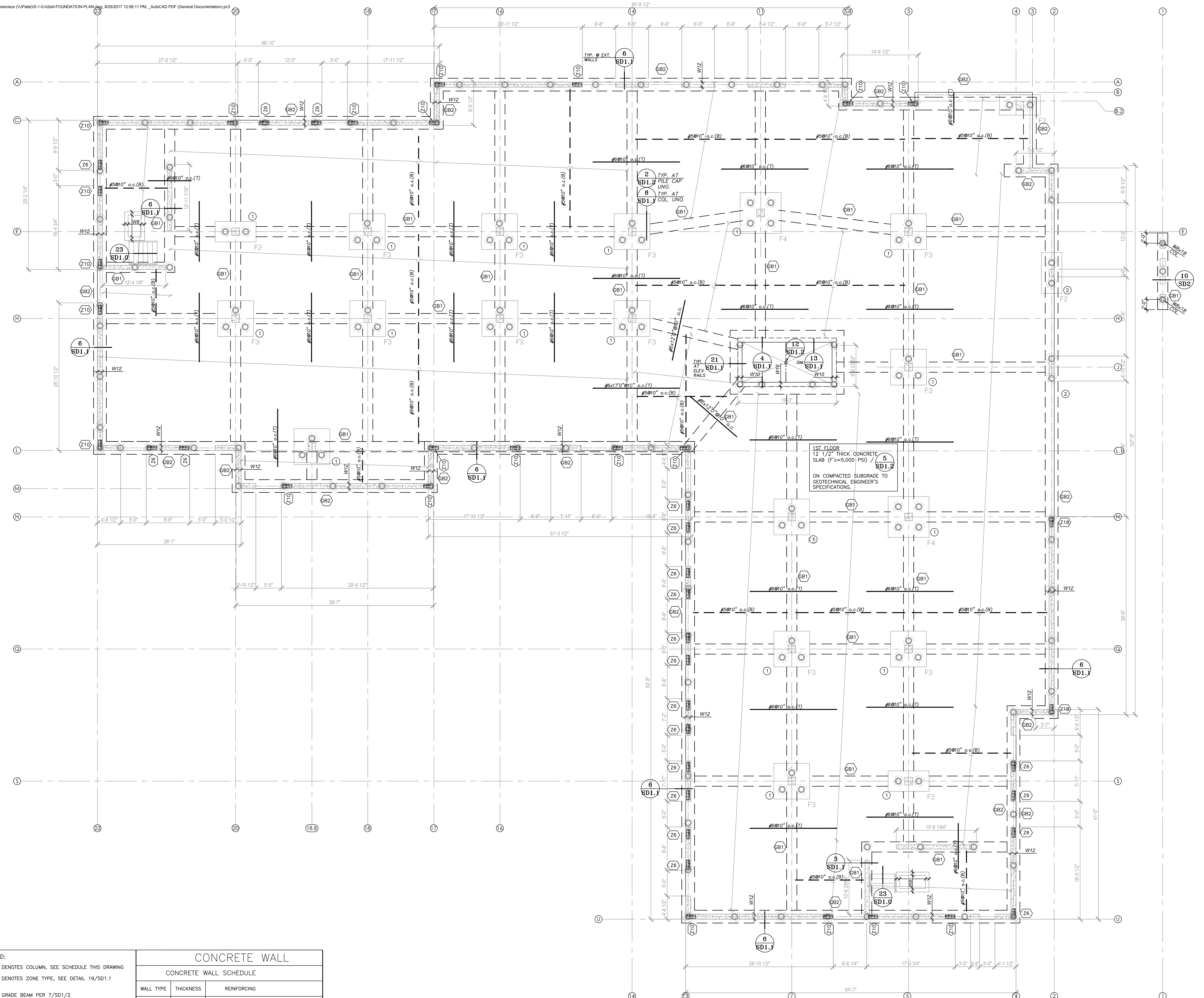
F:\HCP ENGINEERING\PROJECTS\2017-PROJECTS\2017-06 Home2-SouthSanFrancisco (VJPatel)\S-1-0-h2ssf-FOUNDATION-PLAN.dwg, 8/25/2017 12:58:11 PM, _AutoCAD PDF (General Documentation).pc3



1 FOUNDATION PLAN
SCALE 1/8"=1'-0"

NOTES:

1. THIS DRAWING TO BE READ IN CONJUNCTION WITH DRAWING SN-1 FOR GENERAL NOTES AND DRAWING SD-1.0, SD-1.1 & SD-1.2 FOR TYPICAL DETAILS.
2. REFER TO ARCHITECTURAL DRAWINGS FOR EXACT LOCATION OF SLAB EDGE, DETAILS OF FINISHED ELEVATIONS, TOPPING AND WATERPROOFING.
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5. REFER TO GENERAL NOTES FOR MINIMUM SLAB REINFORCEMENT REQUIRED UNLESS SHOWN ON PLAN.
6. MINIMUM REINFORCEMENT AS PER GENERAL NOTES
7. ADDITIONAL REINF. AROUND OPENING, SEE TYP. DETAIL ON DRAWING SD-1.0.
8. FOR CONSTRUCTION & CONTROL JOINT DETAILS AT 15'-0" ON CENTER MAXIMUM, SEE DETAIL ON DRAWING SD-1.0.

LEGEND:

- (1) DENOTES COLUMN, SEE SCHEDULE THIS DRAWING
- Z# DENOTES ZONE TYPE, SEE DETAIL 19/SD1.1
- GB# GRADE BEAM PER 7/SD1/2
- S——S STEPPED FOOTING – SEE DETAIL SHEET 19/SD-1.0
- STEP OR CHANGE AT BOTTOM OF SLAB, SEE SECTIONS
- PILES BY OTHERS
- Z# COLUMN CAPITAL
- SR# SHEAR REINFORCING
- HLDN HOLDOWN PER 6/SD1.2

CONCRETE WALL

CONCRETE WALL SCHEDULE

WALL TYPE	THICKNESS	REINFORCING
W12	12"	#5V@12"o.c & #5H@12" o.c EA. FACE, UNO.
W10	10"	#5V@12"o.c & #5H@12" o.c EA. FACE, UNO.
W8	8"	#5V@12"o.c & #5H@12" o.c EA. FACE, UNO.

REFER TO SD1.1 FOR ZONE REINFORCING AND TYPICAL U-BAR REINFORCING.

COLUMN SCHEDULE

TYPE	COLUMN SIZE	REINFORCING
1	18"x18"	12-#7 VERTS c/w #4 TIES @4"oc (8/SD1.1)
2	12" x 18"	12-#7 VERTS c/w #4 TIES @4"oc (8/SD1.1)

CP# COLUMN CAPITAL

TYPE	COL. CAPITAL SIZE	REINFORCING
CP48	48" SQ x 24" THK	#5 @ 12" o.c. E.W.
CP60	60" SQ x 24" THK	#5 @ 12" o.c. E.W.

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STATE OF CALIFORNIA
8/25/17

CLIENT
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PROJECT
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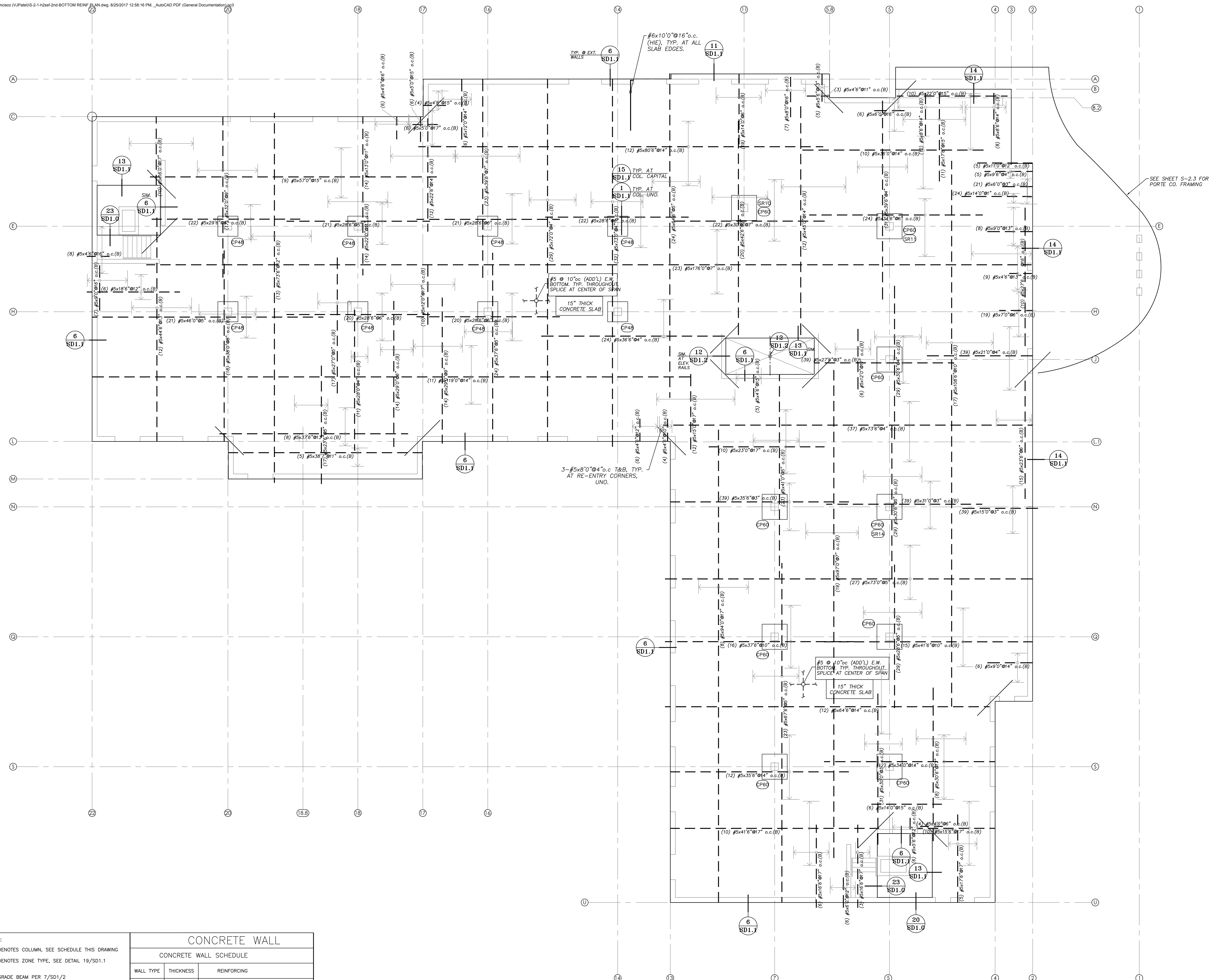
550 GATEWAY BOULEVARD
SOUTH SAN FRANCISCO, CA

FOUNDATION PLAN

#	REVISION	DATE

PROJECT
2017-06
DATE
March 9, 2017
SHEET
S-1.0
PLOT DATE: 8/25/17



1 2nd FLOOR – BOTTOM REINFORCING
SCALE 1/8"=1'-0"

NOTES:

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5. REFER TO GENERAL NOTES FOR MINIMUM SLAB REINFORCEMENT REQUIRED UNLESS SHOWN ON PLAN.
6. MINIMUM REINFORCEMENT AS PER GENERAL NOTES
7. ADDITIONAL REINF. AROUND OPENING, SEE TYP. DETAIL ON DRAWING SD–1.0.
8. FOR CONSTRUCTION & CONTROL JOINT DETAILS AT 15'–0" ON CENTER MAXIMUM, SEE DETAIL ON DRAWING SD–1.0.

LEGEND:

- (1) DENOTES COLUMN, SEE SCHEDULE THIS DRAWING
- (Z#) DENOTES ZONE TYPE, SEE DETAIL 19/SD1.1
- (GB#) GRADE BEAM PER 7/SD1/2
- S——S STEPPED FOOTING – SEE DETAIL SHEET 19/SD–1.0
- STEP OR CHANGE AT BOTTOM OF SLAB, SEE SECTIONS
- PILES BY OTHERS
- (Z#) COLUMN CAPITAL
- (SR#) SHEAR REINFORCING
- HLDN HOLDOWN PER 6/SD1.2

CONCRETE WALL

CONCRETE WALL SCHEDULE

WALL TYPE	THICKNESS	REINFORCING
W12	12"	#5V@12"o.c & #5H@12" o.c EA. FACE, UNO.
W10	10"	#5V@12"o.c & #5H@12" o.c EA. FACE, UNO.
W8	8"	#5V@12"o.c & #5H@12" o.c EA. FACE, UNO.

REFER TO SD1.1 FOR ZONE REINFORCING AND TYPICAL U–BAR REINFORCING.

COLUMN SCHEDULE

TYPE	COLUMN SIZE	REINFORCING
1	18"x18"	12–#7 VERTS c/w #4 TIES @4"oc — 8/SD1.1
2	12" x 18"	12–#7 VERTS c/w #4 TIES @4"oc — 8/SD1.1

(CP#) COLUMN CAPITAL

TYPE	COL. CAPITAL SIZE	REINFORCING
CP48	48" SQ x 24" THK	#5 @ 12" o.c. E.W.
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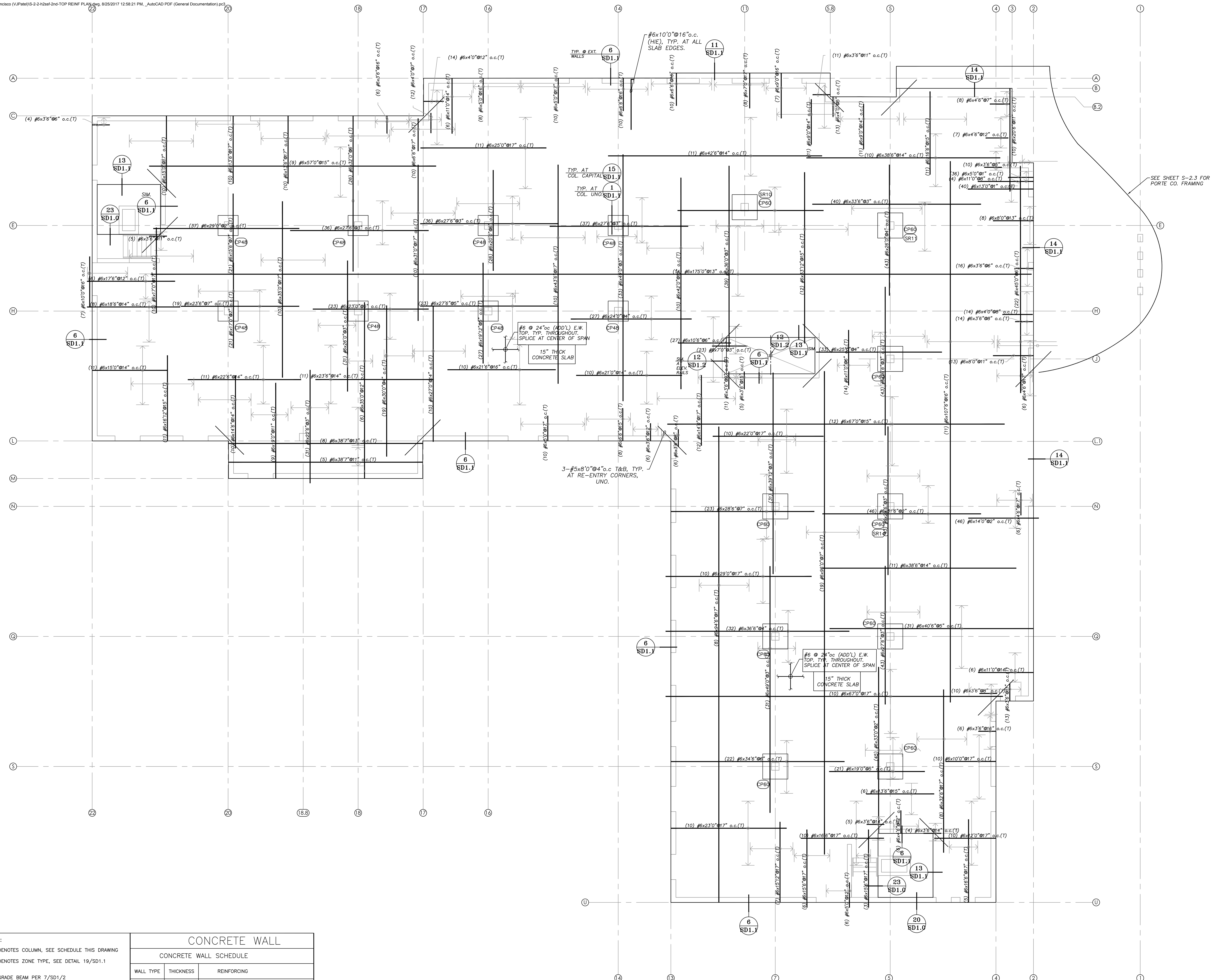
SOUTH SAN FRANCISCO, CA

BOTTOM REINFORCING PLAN

#	REVISION	DATE

PROJECT 2017-06
DATE March 9, 2017
SHEET **S-2.1**



1 — 2nd FLOOR – TOP REINFORCING
SCALE 1/8"=1'-0"

NOTES:

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8. FOR CONSTRUCTION & CONTROL JOINT DETAILS AT 15'-0" ON CENTER MAXIMUM, SEE DETAIL ON DRAWING SD-1.0.

LEGEND:

Symbol	Description
(1)	DENOTES COLUMN, SEE SCHEDULE THIS DRAWING
Z#	DENOTES ZONE TYPE, SEE DETAIL 19/SD1.1
GB#	GRADE BEAM PER 7/SD1/2
S——S	STEPPED FOOTING – SEE DETAIL SHEET 19/SD-1.0
	STEP OR CHANGE AT BOTTOM OF SLAB, SEE SECTIONS
●	PILES BY OTHERS
Z#	COLUMN CAPITAL
SR#	SHEAR REINFORCING
HLDN	HOLDOWN PER 6/SD1.2

CONCRETE WALL

CONCRETE WALL SCHEDULE

WALL TYPE	THICKNESS	REINFORCING
W12	12"	#5V@12"o.c & #5H@12" o.c EA. FACE, UNO.
W10	10"	#5V@12"o.c & #5H@12" o.c EA. FACE, UNO.
W8	8"	#5V@12"o.c & #5H@12" o.c EA. FACE, UNO.

REFER TO SD1.1 FOR ZONE REINFORCING AND TYPICAL U-BAR REINFORCING.

COLUMN SCHEDULE

TYPE	COLUMN SIZE	REINFORCING
1	18"x18"	12-#7 VERTS c/w #4 TIES @4"oc (8/SD1.1)
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TYPE	COL. CAPITAL SIZE	REINFORCING
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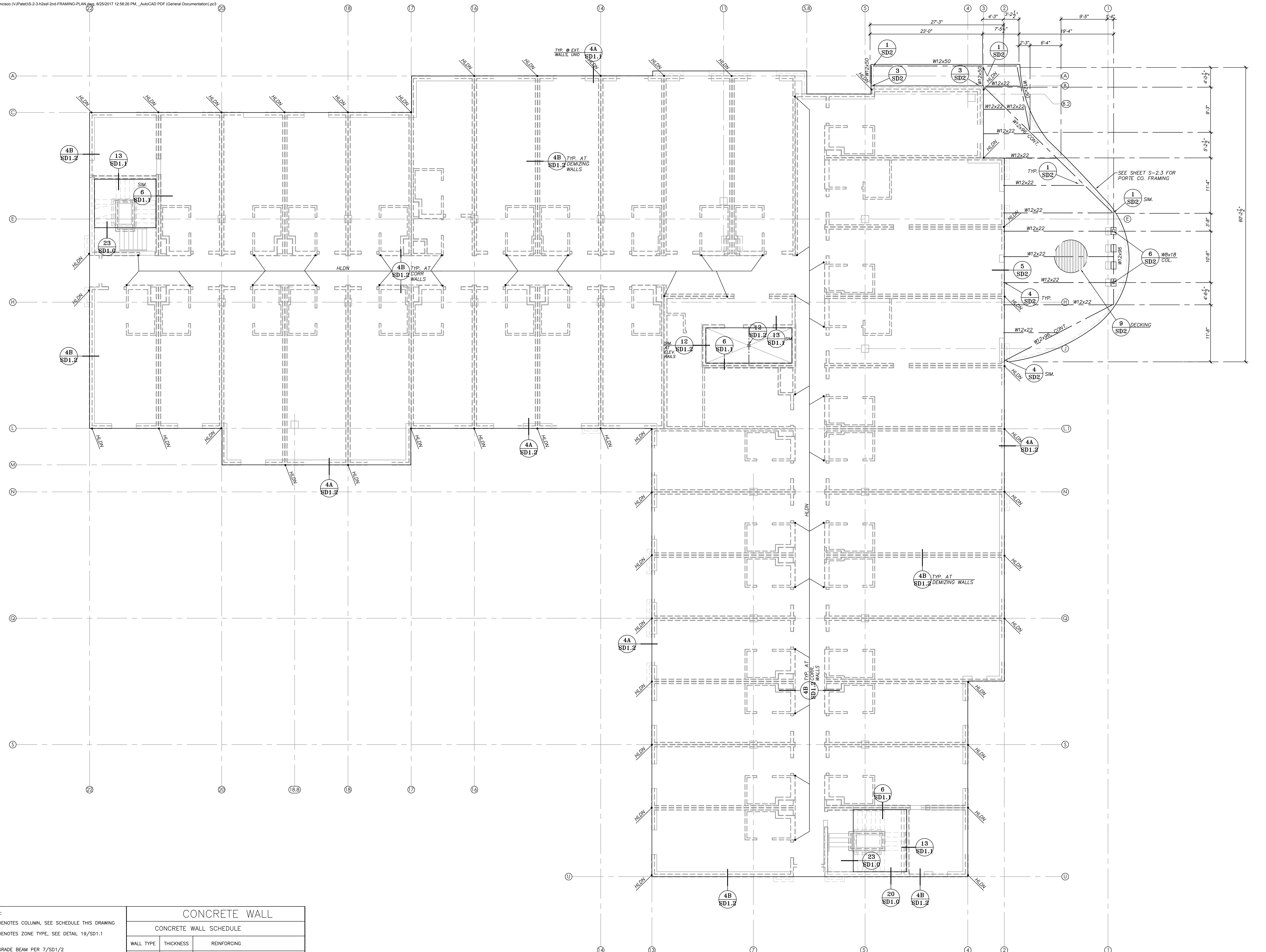
TOP REINFORCING PLAN

#	REVISION	DATE

PROJECT: 2017-06
DATE: March 9, 2017
SHEET: S-2.2

F:\HCP ENGINEERING\PROJECTS\2017-PROJECTS\2017-06 Home2-SouthSanFrancisco (VJPatel)\S-2-3-h2ssf-2nd-FRAMING-PLAN.dwg, 8/25/2017 12:58:26 PM, _AutoCAD PDF (General Documentation).pc3



1 2nd FLOOR - FRAMING PLAN
SCALE 1/8"=1'-0"

NOTES:
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- GB# GRADE BEAM PER 7/SD1/2
- S——S STEPPED FOOTING – SEE DETAIL SHEET 19/SD-1.0
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- PILES BY OTHERS
- Z# COLUMN CAPITAL
- SR# SHEAR REINFORCING
- HLDN HOLDOWN PER 6/SD1.2

CONCRETE WALL

CONCRETE WALL SCHEDULE

WALL TYPE	THICKNESS	REINFORCING
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COLUMN SCHEDULE

TYPE	COLUMN SIZE	REINFORCING
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2	12" x 18"	12-#7 VERTS c/w #4 TIES @4"oc 8/SD1.1

CP# COLUMN CAPITAL

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F:\HCP ENGINEERING\PROJECTS\2017-PROJECTS\2017-06 Home2-SouthSanFrancisco (VJPatel)\S-2-3-h2ssf-2nd-FRAMING-PLAN.dwg, Model, Aug 25, 2017 12:58pm, HCP ENGINEERING

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2ND FLOOR FRAMING PLAN

#	REVISION	DATE

PROJECT
2017-06
DATE
March 9, 2017
SHEET
S-2.3

DESIGN DATA

ROOF DEAD LOADS:

Roofing (Built-up)	= 4.0	psf
Sheathing	= 2.5	psf
Framing	= 3.0	psf
Misc. (Incl. Fire Sprklr)	= 3.0	psf
Ceiling (2 layer ⅝" Gyp)	= 5.5	psf
Photovoltaic Solar Panel	= 12.0	psf
TOTAL DEAD LOAD	= 30.0	psf

ROOF LIVE LOADS:

TYP. ROOF	= 20.0	psf

FLOOR DEAD LOADS:

Flooring	= 2.0	psf
1 ½" Gypsum / Lt. Wt. Conc.	= 15.0	psf
Sheathing	= 2.5	psf
Framing	= 4.0	psf
Ceiling (2 layer ⅝" Gyp)	= 5.6	psf
Misc. (Incl. Fire Sprklr)	= 2.4	psf
TOTAL DEAD LOAD	= 31.5	psf

FLOOR LIVE LOADS:

TYP. FLOOR	= 40.0	psf
STORAGE (LIGHT)	= 125.0	psf
CORRIDOR– PRIVATE AREAS	= 40.0	psf
CORRIDOR– PUBLIC AREAS	= 100.0	psf
BALCONY – PRIVATE	= 60.0	psf
BALCONY– PUBLIC AREAS	= 100.0	psf

OTHER LOADS:

EXTERIOR WALLS	= 15.0	psf
INTERIOR PARTIONS	= 8.0	psf
STUCCO CEILING	= 8.0	psf
VENEER (1" THK MAX.)	= 10.0	psf

WIND:

SPEED, V	= 110.0	mph
IMPORTANCE, I	= 1.0	psf
BUILDING CLASSIFICATION	= II	
EXPOSURE	= C	
± GCpi	= ± 0.18	
C & C PRESSURE	= 18.5	psf

SEISMIC:

SPECTRAL ACCELERATION	S_s	= 1.506
	S_1	= 0.600
	S_{DS}	= 1.004
	S_{D1}	= 0.600
SITE CLASS		= D

UPPER LEVELS :

IMPORTANCE	= 1.0
OCCUPANCY	= II
SEISMIC DESIGN CATEGORY	= E
SEISMIC FORCE RESISTING SYSTEM:	= LIGHT FRAMED WOOD SHEAR PANEL
SEISMIC RESPONSE COEFF, Cs	= 0.155
RESPONSE MODIFICATION FACTOR, R	= 6.5
FUNDAMENTAL PERIOD, T	= 0.330
OVERSTRENGHT FACTOR, Ω_o	= 3
DEFLECTION AMPLIFICATION, Cd	= 4
ANAYSIS PROCEEDURE	= EQUIV. LATERAL FORCE METHOD

CONCRETE PODIUM

IMPORTANCE	= 1.0
OCCUPANCY	= II
SEISMIC DESIGN CATEGORY	= D
SEISMIC FORCE RESISTING SYSTEM:	= CONCRETE SHEAR WALLS
SEISMIC RESPONSE COEFF, Cs	= 0.201
RESPONSE MODIFICATION FACTOR, R	= 5
FUNDAMENTAL PERIOD, T	= 0.112
OVERSTRENGHT FACTOR, Ω_o	= 2.5
DEFLECTION AMPLIFICATION, Cd	= 5
ANAYSIS PROCEEDURE	= EQUIV. LATERAL FORCE METHOD

TESTS, INSPECTIONS AND OBSERVATIONS:

1. Continuous inspection by a Registered Deputy Inspector is required for all field welding, high strength bolting, post tensioning, concrete with a strength greater than 2,500 psi, and masonry where noted on the plans and details. The extent of such inspection shall conform to Chapter 17 of the Building Code. An affidavit shall be issued to the Architect/Engineer and the Building Department at the completion of each type of work stating whether the work was in conformance with the approved plans and specifications.

2. In addition to inspections by the Building Department, the following checked items will require Special Inspection in accordance with Chapter 17 of the Building Code.

SPECIAL INSPECTION	REQUIED IF CHECKED	REMARKS
FOUNDATION		
SOIL COMPLIANCE PRIOR TO FOUNDATION INSPECTION BY GEOTECHNICAL ENGINEER	X	OBSERVATION OF SOIL EXCAVATION AND FOUNDATION CONSTRUCTION OPERATION BY GEOTECHNICAL ENGINEER.
CONCRETE:		
HIGH STRENGTH CONCRETE (2,500 PSI OR GREATER)	X	CONCRETE SPECIMEN SAMPLES REBAR & CONTINOUS CONCRETE PLACEMENT
MOMENT RESISTING CONCRETE FRAMES		
PRETENSIONED OR POST-TENSIONED CONCRETE	X	
STEEL:		
SHOP WELDING	X	
FIELD WELDING	X	
HIGH STRENGTH BOLTS	X	
MASONRY:		
PRISM TEST – HIGH STRENTH MASONRY		
REINFORCING AND MASONRY UNIT PLACEMENT	X	PERIODIC SPECIAL INSPECTION
PRIOR TO GROUTING AND GROUTING OPERATIONS	X	
WOOD (Seismic Design Category C, D, E, & F):		PERIODIC SPECIAL INSPECTION REQUIRED FOR NAILING, BOLTING, ANCHORING AND OTHER FASTENING OF COMPONENTS WITHIN SEISMIC FORCE RESISTING SYSTEM, INCLUDING WOOD SHEAR WALLS, DIAPHRAGMS, DRAG STRUTS, BRACES, SHEAR PANELS, AND HOLDOWNS. (SPECIAL INSPECTION NOT REQUIRED WHERE FASTENER SPACING IS MORE THAN 4" o.c.)
SHEAR WALLS & SEISMIC FORCE RESISTING SYSTEM	X	
HOLDOWNS	X	
OTHER:		
SPECIAL GRADING, EXCAVATION & BACKFILL OPERATIONS:		
SHOTCRETE / GUNITE WORK.	X	
PILING, DRILLED PIERS & CASSIONS	X	
EXPANSION ANCHORS / EPOXIED ANCHORS	X	PER CBC SECTION 1705
MECHANICAL AND ELECTRICAL COMPONENTS	X	PERIODIC SPECIAL INSPECTION
SPRAYED FIRE-RESISTANT MATERIAL ON STRUCT. STEEL		
ARCHITECTURAL COMPONENTS: PERIODIC SPECIAL INSPECTION IS REQUIRED DURING THE ERECTION AND FASTENING OF EXTERIOR CLADDING, INTERIOR AND EXTERIOR NONBEARING WALLS AND INTERIOR AND EXTERIOR VENEER IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY D, E or F.	X	EXCEPTIONS: 1. SPECIAL INSPECTION IS NOT REQUIRED ARCHITECTURAL COMPONENTS IN STRUCTURES 30 FEET OR LESS IN HEIGHT. 2. SPECIAL INSPECTION IS NOT REQUIRED FOR CLADDING AND VENEER WEIGHING 5 PSF OR LESS. 3. SPECIAL INSPECTION IS NOT REQUIRED FOR INTERIOR NONBEARING WALLS WEIGHING 15 PSF OR LESS.

CBC TABLE 1705.3
REQUIRED VERIFICATION AND INSPECTION OF CONCRETE CONTRUCTION

VERIFICATION AND INSPECTION	CONTINUOUS	PERIODIC	REFERENCE STANDARD [a]	IBC REFERENCE
1. Inspection of reinforcing steel, including prestressing tendons, and placement.	–	X	ACI 318: 3.5, 7.1–7.7	1910.4
2. Inspection of reinforcing steel welding in accordance with Table 1705.2.2, Item 2b.	–	–	AWS D1.4 ACI 318: 3.5.2	–
3. Inspection of anchors cast in concrete where allowable loads have been increased or where strenght design is used.	–	X	ACI 318: 8.1.3, 21.2.8	1908.5 1909.1
4. Inspection of anchors post-installed in hardened concrete members [b]	–	X	ACI 318: 3.8.5, 8.1.3, 21.2.8	1909.1
5. Verifying use of required design mix.	–	X	ACI 318: Ch 4,5.2–5.4	1904.2., 1910.2, 1910.3
6. At the time fresh concrete is sampled to fabricate specimens for strength tests, perform slump and air content test, and determine the temperature of the concrete.	X	–	ASTM C 172 ASTM C 31 ACI 318: 5.6, 5.10	1910.10
7. Inspection of concrete and shotcrete placement for proper application techniques.	X	–	ACI 318: 5.9, 5.10	1910.6, 1910.7, 1910.8
8. Inspection for maintenance of specified curing temperature and technique.	–	X	ACI 318: 5.11–5.13	1910.9
9. Inspection of prestressed concrete: a. Application of prestressing forces. b. Grouting of bonded prestressing tendons in the seismic-force-resisting system.	X X	–	ACI 318: 18.20 ACI 318: 18.18.4	–
10. Erection of precast concrete members.	–	X	ACI 318: Ch. 16	–
11. Verification of in-situ concrete, prior to stressing of tendons in posttensioned concrete and prior to removal of shores and forms from beams and structural slabs.	–	X	ACI 318: 6.2	–
12. Inspection formwork for shape, location and dimensions of the concrete member being formed.	–	X	ACI 318: 6.1.1	–

For SI: 1 inch = 25.4 mm.
a. Where applicable, see also Section 1705.11, Special inspection for seismic resistance.
b. Specific requirements for special inspection shall be included in the research report of the anchor issued by an approved source in accordance with ACI 355.2 or other qualification procedures. Where specific requirements are not provided, special inspection shall be specified by the registered design professional and shall be approved by the building official prior to the commencement of the work.

3. Special inspection agencies and inspectors must possess an ICC certification for Prestress Concrete/Reinforcing Concrete and be pre-approved by The Building Official prior to any inspections. The special inspector must provide to The Building Official with a copy of his/her ICC current certificate and photo identification card. All documents of special inspections, field reports, surveys must be addressed to the The Building Official with job address and premit number for inclusion in city records. CBC 108.4.3.1

STRUCTURAL OBSERVATIONS:

1. Where required by the provisions of Section 1709.2 or 1709.3 the owner shall employ the Engineer or Architect Responsible for the structural design to perform Structural Observation.
2. At the conclusion of the work included in the permit, the Structural Observer shall submit to the Building Official a written statement that the visits have been made and identify any reported deficiencies that, to the best of the Structural Observer's knowledge, have been resolved.
3. The Structural Observations shall be performed by the registered design professional responsible for the structural design. The structural observer shall be registered / licensed to practice by the State of California
4. The structural observer shall submit a written statement to Inspection Services a written statement that the site visits have been made and identify any reported deficiencies that to the best of the structural observer's knowledge have bee resolved. The structure will not be in compliance until the registered professional has notified Inspection Services, in writing, that all deficiencies are resolved.

STRUCTURAL OBSERVATION REQ'D (when checked)	REQUIRED IF CHECKED	REMARKS
CONCRETE		PRIOR TO CONCRETE POUR OF EACH ITEM
1. PILE CAGES (PRIOR TO LOWERING THEM)		
2. FOUNDATIONS	X	
3. CONCRETE COLUMN & WALLS	X	
4. FIRST FLOOR CONCRETE DECK	X	
5. HOLDOWN, ANCHOR BOLTS & EMBEDED ITEMS.	X	
MASONRY		AT EACH LEVEL AND PRIOR TO GROUTING OF WALL
1. MASONRY WALLS AND REINFORCING		
2. –		
WOOD FRAMING		AT EACH LEVEL PRIOR TO COMMENCING WITH ADDITIONAL LEVEL, PRIOR TO COVERING FRAMING AND PRIOR TO FRAMING FINAL INSPECTION
1. GENERAL FRAMING	X	
2. DIAPHRAGM NAILING	X	
3. SHEAR WALL NAILING	X	
4. HOLDOWN SYSTEM	X	PRIOR TO DRYWALL
STRUCTURAL STEEL:		
1. MOMENT FRAME CONNECTIONS	X	PRIOR TO COVERING OF CONNECTIONS

5. Project Applicant (Owner/Contractor) shall coordinate forms and proceedure with the Building Department.

6. Contractor Responsibility per CBC 1706: Each contractor responsible for the construction of a main wind- or seismic-force-resisting system, designated seismic system or a wind- or seismic-resisting component listed in the statement of special inspections (Section 1705) shall submit a written statement of responsibility to the building official and the owner prior to the commencement of work on the system or component. The contractor's statement of resonsibitility shall contain the following:
i) Acknowledgement of awareness of the special requirements contained in the statement of in the statement of special inspections.
ii) Acknowledgement that control will be exercised to obtain conformance with the construction documents approved by the building official
iii) Procedure for exercising control within the contractor's organization, the method and frequency of reporting and the distribution of the reports
iv) Identification and qualifications of the person(s) exercising such control and their position(s) in the organization.

MASONRY

ALL MASONRY CONSTRUCTION SHALL BE IN ACCORDANCE WITH CBC CHAPTER 21.

ALL CMU WALLS SHALL BE FULLY SOLID GROUTED UNLESS NOTED OTHERWISE.

AN INDEPENDENT TESTING AGENCY SHALL PERFORM PRE-CONSTRUCTION TESTING OF THE BLOCK, MORTAR AND GROUT.

CMU – TEST PER ASTM C140
GROUT – TEST PER ASTM C1019
MORTAR – TEST PER ASTM C1586

THE MINIMUM COMPRESSIVE STRENGTH OF MASONRY WALL ASSEMBLIES AT 28 DAYS (f'm) SHALL BE 1,500 PSI FOR ALL CMU WALLS U.N.O. MORTAR SHALL BE TYPE M OR S.

THE MINIMUM STRENGTH OF MASONRY UNITS, AND GROUT AT 28 DAYS SHALL BE:

SPECIFIED f'm	MASONRY UNIT	GROUT
= 1500 p.s.i.	1900 p.s.i.	2000 p.s.i.

COMPLIANCE WITH THE REQUIREMENTS FOR f'm=1500 PSI SHALL BE IN ACCORDANCE WITH CBC SECTION 2105.2.2.1.2 UNIT STRENGTH METHOD USING FULL STRESSES IN DESIGN AND COMPRESSIVE STRENGTH. (ALTERNATIVELY A PRISM TEST METHOD MAY BE USED PER CBC 2105.2.2.2 AND APPROVED BY THE STRUCTURAL ENGINEER).

ALL GROUT MIX DESIGNS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW PRIOR TO CONSTRUCTION. ALL GROUT SHALL BE INSPECTED BY A REGISTERED INSPECTOR.

GROUT SHALL CONSIST OF MIXTURE OF CEMENTITIOUS MATERIALS AND AGGREGATE TO WHICH POTABLE WATER IS ADDED SUCH THAT THE MIXTURE WILL FLOW WITHOUT SEGREGATION OF THE CONSTITUENTS.

VERTICAL CELLS TO BE FILLED SHALL HAVE VERTICAL ALIGNMENT AND SHALL BE CLEAR OF OVERHANGING MORTAR, DEBRIS OR OTHER OBSTRUCTIONS

ALL HORIZONTAL REINFORCING BARS SHALL BE PLACED IN BOND BEAM UNITS.

PROVIDE CORNER BARS TO MATCH HORIZONTAL WALL REINFORCEMENT AT WALL INTERSECTIONS, AND CORNER BARS OR HOOKS AT ENDS OF BOND BEAMS.

ENDS OF WALLS, WALL INTERSECTIONS, AND AT OPENINGS: 1-#5 VERT.
AROUND OPENINGS, EXTENDING 16" BEYOND CORNERS: 2-#5 HORIZ.
ALL COURSES CONTAINING INSERTS: 2-#5
TOP COURSE OF PARAPET WALLS: 2-#5

PROVIDE 1/2" MINIMUM CLEAR SPACE BETWEEN REINFORCING BARS AND THE MASONRY UNIT WALLS.

HEIGHT OF GROUT POURS SHALL CONFORM TO CBC 2104.1.2.7.
A HORIZONTAL CONSTRUCTION JOINT SHALL BE FORMED BETWEEN GROUT POURS BY STOPPING GROUT A MINIMUM OF 1.5" BELOW A MORTAR JOINT, WHERE BOND BEAMS OCCUR, GROUT POUR SHALL BE STOPPED A MINIMUM OF 1/2" BELOW THE TOP OF MASONRY WEBS.

DURING PLACING, GROUT SHALL BE CONSOLIDATED BY MECHANICAL VIBRATION BEFORE LOSS OF PLASTICITY SUCH AS TO FILL THE GROUT SPACE AND SHALL BE RE-CONSOLIDATED BY MECHANICAL VIBRATION 15 MINUTES LATER TO MINIMIZE VOIDS DUE TO WATER LOSS.

ALL MASONRY WORK SHALL BE CONTINUOUSLY INSPECTED.

CONCRETE MASONRY UNITS SHALL NOT BE "WETTED" UNLESS SPECIFICALLY APPROVED.

REFER TO ARCHITECTURAL DRAWINGS FOR MASONRY UNITS, JOINT TYPE, AND LAYING PATTERN.

MINIMUM SPLICE LENGTHS:

BAR SIZE	SPLICE LENGTH
#5 BARS	30"
#6 BARS	40"

CONCRETE

PROVIDE CONCRETE AND PERFORM ALL WORK ACCORDING TO CURRENT ACI 318 AND ACI 347.
ALL CONCRETE SHALL BE NORMAL WEIGHT.
AGGREGATE SHALL CONFORM TO ASTM C-33.
TYPE II & V PORTLAND CEMENT TYPICAL – SHALL CONFORM TO ASTM C-150.
MIX WATER SHALL BE POTABLE, FREE OF DELETERIOUS AMOUNTS OF ACIDS, ALKALIS OR ORGANIC MATERIALS, SALTS OR OILS IN ACCORDANCE WITH ACI 318-3.4.
CONCRETE MIXING SHALL CONFORM TO ASTM C-94.
ALL ADDITIVES OR ADMIXTURES, IF USED, REQUIRE PRIOR APPROVAL FROM THE STRUCTURAL ENGINEER.

CONCRETE SHALL CONFORM TO THE FOLLOWING:

LOCATION	28-DAY MIN. COMPRESSIVE STRENGTH
FOOTINGS, GRADE BEAMS, AND SLAB ON GRADE	5,000 p.s.i.
WALLS	4,000 p.s.i.
COLUMNS, SUSPENDED SLABS	5,000 p.s.i.
MISC. CONCRETE AND TOPPING	2,500 p.s.i.

ALL CONCRETE MIX DESIGNS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW PRIOR TO CONSTRUCTION. ALL CONCRETE SHALL BE INSPECTED BY A REGISTERED INSPECTOR.

GROUT (FOR UNDER STEEL BASE PLATES AND WHERE SHOWN): NON-SHRINK, NON-FERROUS, PREMIXED GROUT DEVELOPING A MINIMUM COMPRESSIVE STRENGTH OF 7250 psi AT 28 DAYS.

MAXIMUM SLUMPS IN THE DESIGN ARE INTENDED FOR CONTROL OF CONCRETE MIX WATER CEMENT RATIO WITH OR WITHOUT LOW-RANGE WATER REDUCING ADMIXTURE. MIX DESIGNS USING HIGH-RANGE WATER REDUCING ADMIXTURE MAY BE USED. FOR SUSPENDED SLABS AND BEAMS SLUMP SHALL BE MEASURED AT THE POINT OF PLACEMENT. SLUMP TESTS SHALL BE IN ACCORDANCE WITH ASTM C-143 AND PORTLAND CEMENT ASSOCIATION "METHOD OF TEST FOR SLUMP".

ALL CONCRETE TEST REPORTS (SHOWING COMPRESSIVE STRENGTHS AND SHRINKAGE) SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER AND THE BUILDING DEPARTMENT.

CONCRETE SHALL BE CURED IN ACCORDANCE WITH CBC 1905.11.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR DESIGN OF ALL FORMWORK. USE MECHANICAL VIBRATORS THROUGHOUT TO COMPACT CONCRETE. USE KEYED CONSTRUCTION JOINTS AS DIRECTED BY THE STRUCTURAL ENGINEER, UNLESS DETAILED ON DRAWINGS. CHECK ALL APPLICABLE DRAWINGS FOR LOCATIONS OF BLOCKOUTS, ANCHORS AND EMBEDDED INSERTS BEFORE CONCRETE IS PLACED. NO CONDUIT, BOXES OR OTHER INSERTS ARE PERMITTED IN COLUMNS WITHOUT THE PRIOR WRITTEN APPROVAL OF THE STRUCTURAL ENGINEER. DO NOT CUT SLAB REINFORCEMENT AT OPENINGS, SPREAD REINFORCEMENT AROUND OPENINGS. PROVIDE ALL NECESSARY CARRY BARS REQUIRED TO MAINTAIN REINFORCEMENT IN POSITION. REINFORCING STEEL MUST BE INSPECTED BY THE STRUCTURAL ENGINEER BEFORE CONCRETE IS PLACED.

DO NOT REMOVE FORMS FOR FOOTINGS AND WALLS UNTIL A MINIMUM OF 24 HOURS AFTER CONCRETE IS PLACED AND THE CONCRETE HAS ATTAINED A STRENGTH OF AT LEAST 1500psi. NO EXCEPTIONS WILL BE PERMITTED. FORMS FOR MILD STEEL REINFORCED SUSPENDED SLABS MAY BE REMOVED AND RESHORING INSTALLED AFTER THE CONCRETE HAS ATTAINED AT LEAST TWO-THIRDS OF THE SPECIFIED STRENGTH. RESHORING TO REMAIN IN PLACE UNTIL CON-CRETE HAS DEVELOPED FULL STRENGTH. STRENGTH OF CONCRETE AT TIME OF STRIPPING FORMS TO BE DETERMINED BY TESTING FIELD CURED CONCRETE CYLINDERS, COSTS FOR WHICH TO BE BORNE BY THE CONTRACTOR.

UNLESS OTHERWISE NOTED ON ARCHITECTURAL OR STRUCTURAL DRAWINGS ALL EXPOSED CORNERS OF WALLS, COLUMNS, BEAMS, SLABS, ETC., SHALL BE FORMED WITH A 3/4" CHAMFER.

PIPES AND CONDUITS:

PIPES AND CONDUITS SHALL NOT BE PLACED EMBEDDED IN STRUCTURAL CONCRETE EXCEPT AS SPECIFICALLY APPROVED BY THE STRUCTURAL ENGINEER.

SUBMIT SHOP DRAWINGS SHOWING ALL CONDUIT SIZES, LOCATIONS AND SPACINGS.

ALL OPENINGS IN CONCRETE WALLS AND SLABS SHALL BE SLEEVED BEFORE PLACING CONCRETE. CORING SHALL NOT BE PERMITTED IN COLUMNS, WALLS, SUSPENDED SLABS, BEAMS, AND FOOTINGS. WHERE SLEEVES ARE IN CONFLICT WITH STEEL DO NOT CUT REINFORCING STEEL, BEND REBAR

CONDUIT SHALL BE PLACED IN THE MIDDLE 1/3 OF THE THICKNESS OF THE SLAB. CLEAR DISTANCE BETWEEN CONDUITS SHALL BE 6" MINIMUM.

CONDUITS AND PIPES EMBEDDED SHALL COMPLY WITH CBC 1906.3.

CONSTRUCTION JOINTS:

THE SURFACES OF ALL CONSTRUCTION JOINTS SHALL BE FREE FROM LOOSE DEBRIS, THOROUGHLY CLEANED AND ALL LAITANCE REMOVED. IMMEDIATELY BEFORE NEW CONCRETE IS PLACED, ALL CONSTRUCTION JOINTS SHALL BE WET DOWN AND STANDING WATER REMOVED.

U.N.O. ALL CONSTRUCTION JOINTS SHALL HAVE A MINIMUM OF 3/4" ROUGHENED AMPLITUDE OR 2X4 SHEAR KEY

SPLATTER OF CONCRETE ON REINFORCING STEEL SHALL BE REMOVED BY WIRE BRUSHING.

REINFORCING STEEL

ALL REINFORCING BARS SHALL BE ASTM A-615 GRADE 60 DEFORMED BILLET STEEL BARS EXCEPT NOTED BELOW:

1. ALL ZONE REINFORCING BARS IN THE SHEAR WALLS AND LONGITUDINAL BARS IN THE MOMENT FRAMES (BEAMS AND COLUMNS) SHALL COMPLY WITH LOW ALLOY A706 STEEL GRADE 60.

SUBSTITUTION OF A615 REBAR FOR A706 IS ACCEPTABLE AS LONG AS THE PROPERTIES PROVIDED IN SUPPORTING MATERIAL TESTS REPORTS ARE CONSISTENT WITH THE CRITERIA OUTLINED IN SECTION 21.1.5.2 OF ACI 318:

THE YIELD STRENGTH DOES NOT EXCEED THE SPECIFIED YIELD BY MORE THAN 18,000 psi AND THE RATIO OF THE ACTUAL TENSILE STRENGTH TO THE ACTUAL YIELD STRENGTH IS NOT LESS THAN 1.25.

NOTE: A615 IS NOT AN ACCEPTABLE SUBSTITUTE WHERE WELDING IS REQUIRED.

2. ALL BARS REQUIRED TO BE WELDED SHALL CONFORM TO ASTM A-706 GRADE 60 DEFORMED BARS. (LOCATION OF WELDS MUST BE PRE-APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO WELDING)

3. WELDED SMOOTH WIRE FABRIC SHALL CONFORM TO ASTM A-185.

PLACEMENT OF REINFORCEMENT SHALL BE IN ACCORDANCE WITH ACI "MANUAL OF STANDARD PRACTICE".

A CERTIFIED COPY OF THE MILL TEST SHOWING PHYSICAL AND CHEMICAL ANALYSIS ON EACH HEAT OF REINFORCING STEEL DELIVERED MAY BE REQUIRED UPON REQUEST.

TIE WIRE USED SHALL BE 16 GA. OR HEAVIER, BLACK ANNEALED WIRE.

REINFORCING STEEL, AT THE TIME OF CONCRETE PLACEMENT, SHALL BE FREE OF RUST, MUD, OIL, OR OTHER COATING THAT MIGHT ADVERSELY AFFECT BONDING CAPACITY.

UNLESS OTHERWISE SHOWN ON THE DRAWINGS ALL REINFORCING BAR HOOKS SHALL CONFORM TO ACI "STANDARD HOOK" BEND DIMENSIONS.

REINFORCING BARS SHALL NOT BE BENT OR STRAIGHTENED IN A MANNER THAT WILL DAMAGE THE MATERIAL INCLUDING BUT NOT LIMITED TO HEATING, RE-BENDING, AND NOTCHING.

REINFORCING BARS SHALL CONFORM TO THE DIMENSIONS SHOWN ON THE DRAWINGS WITH FABRICATION TOLERANCES IN ACCORDANCE WITH THE ACI "MANUAL OF STANDARD PRACTICE".

BARS SHALL BE SECURELY TIED AND ALL DOWELS SHALL BE WIRED IN PLACE TO PREVENT DISPLACEMENT DURING THE CONCRETE PLACEMENT.

REINFORCING SPLICES SHALL BE MADE ONLY AS SHOWN ON PLANS. ALL SPLICES FOR UNCOATED REINFORCING NOT DETAILED SHALL BE SPLICED AS PER SCHEDULE (1/SD1.0) OR AS APPROVED BY THE STRUCTURAL ENGINEER.

MINIMUM LAP OF MESH SHALL BE NOT LESS THAN:

THE SPACING OF THE CROSS WIRE PLUS TWO INCHES, OR
60 DIAMETERS, OR
8 INCHES; WHICHEVER IS GREATEST.

DOWELS BETWEEN FOOTINGS AND WALLS OR COLUMNS SHALL BE THE SAME SIZE AS THE VERTICAL REINFORCEMENT SPECIFIED ON THE DRAWINGS; UNLESS OTHERWISE SHOWN MINIMUM EMBEDMENT OF DOWELS SHALL BE 22 BAR DIAMETERS PLUS A STANDARD 90° HOOK.

PROVIDE CORNER BARS AS PER DETAIL (11/SD1.0)

UNLESS NOTED OTHERWISE ON PLAN, NOT LESS THAN 2-#5 BARS SHALL BE PROVIDED AROUND ALL WINDOW AND DOOR OPENINGS, EXTENDING THE REINFORCING 36" BEYOND CORNERS.

REINFORCEMENT SHOWN ON PLAN:

TOP STEEL: SOLID LINE
BOTTOM STEEL: BROKEN LINE

WELDING OF REINFORCING STEEL:

ONLY ALLOY A706 REINFORCING GRADE 60 IS PERMITTED TO BE WELDED. WELDING OF REINFORCING STEEL SHALL HAVE PRIOR APPROVAL OF THE STRUCTURAL ENGINEER. WELDING OF REINFORCING STEEL SHALL BE PERFORMED BY WELDERS SPECIFICALLY CERTIFIED AS REQUIRED BY GOVERNING CODE AUTHORITY. OTHER REINFORCING GRADES (NON A706 REINFORCING STEEL) COMPLYING WITH AWS D1.4 MAY BE PERMITTED TO BE WELDED WITH PRE-APPROVAL FROM THE STRUCTURAL ENGINEER AND THE GOVERNING CODE AUTHORITY. SUBMIT PROPOSED PROCEDURE AND REINFORCING TEST REPORTS TO THE STRUCTURAL ENGINEER AND THE GOVERNING CODE AUTHORITY FOR APPROVAL PRIOR TO EXECUTION.

CONTRACTOR TO PROVIDE AN ALLOWANCE FOR APPROXIMATELY 5000 LBS OF REINFORCING STEEL, BENT OR STRAIGHT TO BE USED AT THE DISCRETION OF THE STRUCTURAL ENGINEER.

ALL CONCRETE TO BE REINFORCED UNLESS NOTED OTHERWISE. MINIMUM REINFORCEMENT FOR WALLS AND SLABS TO BE AS FOLLOWS U.N.O.:

WALL OR SLAB THICKNESS	WALL REINFORCEMENT	SLAB REINF. E.W. BOTTOM
6" OR LESS	#4 @ 12" E.W.	#4 @ 12"
6 1/4" TO 9 3/4"	#4 @ 16" E.W.	#5 @ 16"
10" TO 12"	#5 @ 12" E.W. E.F.	#5 @ 16"

CONCRETE COVER FROM REINFORCING TO FACE OF CONCRETE SHALL BE AS FOLLOWS:

CONCRETE CAST AGAINST EARTH		3"
FORMED CONCRETE EXPOSED TO EARTH		2"
WALLS		1"
STRUCTURAL SLABS	AT TOP AT BOTTOM	3/4" 1"
BEAMS	TO STIRRUPS	1 1/2"
COLUMNS	TO TIES	1 1/2"
HANGER INSERTS IN CONCRETE SLABS SHALL BE PLACED WITH 2" CONCRETE COVERAGE FROM TOP OF THE SLAB.		

GENERAL NOTES:

1. All construction and workmanship shall conform to the 2016 California Building Code.
2. These notes shall be used in conjunction with the plans and any discrepancies shall be bought to the attention of the Architect/Engineer.
3. Contractor must check all dimensions, framing conditions, and site conditions before starting work. Architect/Engineer shall be notified immediately of any discrepancies or possible deficiencies.
4. Conditions not specifically shown shall be constructed similar to the details for the respective materials.
5. The drawings and specifications represent the finished structure. All bracing, temporary supports, shoring, etc. is the sole responsibility of the Contractor. Observation visits to the site by the Architect/Engineer do not include inspection of construction procedures. The Contractor is solely responsible for all construction methods and for safety conditions at the worksite. These visits shall not be construed as continuous and detailed inspections.
6. Design, materials, equipment, and products other than those described below or indicated on the drawings may be considered for use, provided prior written approval is obtained from the Owner, Architect/Engineer, and the applicable governing code authority. Approval of submittal and shop drawings shall not be considered as written approval unless a written request specifying the substitution is provided and accepted by Architect/Engineer.
7. All conditions noted as existing are based on the information currently available at the time of preparation of these drawings. The Contractor is to verify all conditions before starting work. Should conditions arise which are different from those shown on the drawings, the Architect/Engineer shall be notified immediately and additional drawings based on more accurate information will be prepared.
8. NOTICE TO THE APPLICANT/OWNER/OWNER'S AGENT/ARCHITECT or ENGINEER OF RECORD: By using this permitted construction drawings for construction/installation of the work specified herein, you agree to comply with the requirements of City Building Department for special inspections, structural observations, construction material testing and off-site fabrication of building components, contained in the statement of special inspections and, as required by the California construction codes.
9. NOTICE TO THE CONTRACTOR/BUILDER/INSTALLER/SUB-CONTRACTOR/OWNER-BUILDER: By using this permitted construction drawings for construction/installation of the work specified herein, you acknowledge and are aware of the requirements contained in the statement of special inspections. You agree to comply with the requirements of the City Building Department for special inspections, structural observations, construction material testing and off-site fabrication of building components, contained in the statement of special inspections and as required by the California construction codes.
10. The special inspector must be certified by the City Building Department in the category of work required to have special inspection.
11. The construction materials testing laboratory must be approved by the City Building Department for the testing of materials, systems, components and, equipments.
12. Fabrication must be registered and approved by the City Building Department for the fabrication of members and assemblies on the premises of the fabricator's shop.
13. Fabricators shall submit an 'Application to perform Off-Site Fabrication' to the Inspection Services Division for approval prior to commencement of fabrication.
14. Fabricator shall submit a 'Certificate of Compliance for Off-Site Fabrication to the Inspection Services Division prior to erection of fabricated items and assemblies.

FOUNDATIONS:

1. All existing fill soil and disturbed natural soils are to be excavated and replaced with properly compacted fill.
2. All excavations, filling, backfilling, recompaction and building pad shall be inspected and approved by a Soils Engineer prior to the placement of any concrete or reinforcing. The contractor shall be responsible for all shoring necessary to support cut and/or fill sections during excavation and for forming and placement of concrete.
3. Footings are to be carried a minimum of 48" (Unless Noted Otherwise) into firm undisturbed natural soils or approved compacted fill.
4. Design bearing pressure is 4,000 psf (48" deep) (DL+LL) with 33 % increase for seismic or wind loading.
5. Relatively non-expansive fill should be used in backfilling behind walls. All walls shall be adequately shored during backfilling operation.
6. Fasteners in preservative-treated wood (Anchor Bolt, Nails, Screws...etc) are to be approved silicon bronze, copper, stainless steel or hot-dipped zinc coated steel (CBC.2304.9.5)
7. All excavation, grading, compaction, etc. shall be accomplished and performed in accordance with the Soils Report. The Soils Report is hereby made a part of these General Notes and all recommendations contained therein shall be considered as minimums. Soils Report prepared by:

   SILICON VALLEY SOIL ENGINEERING, 2391 ZANKER RD., SUITE 350, SAN JOSE, CA 95131
   P: 408-324-1400.
   REPORT NO.: SV1300, DATED: OCTOBER 1, 2014

WOOD FRAME:

1. All structural lumber shall be Douglas Fir of the following grades, conforming to the Standard Grading Rules for West Coast Lumber, No. 16, unless noted otherwise.

Rafters, joists	No. 2	1250 F
Beams and stringers	No. 1	1300 F
Posts and timbers	No. 1	1200 F
2x4 joists and rafters	No. 2	1250 F
Studs, blocking, plates	No. 2	1250 F

2. All wood bearing on concrete or masonry or within 8" of earth shall be pressure treated Douglas Fir.
3. Each sheet of plywood shall be identified by a registered stamp or brand of the American Plywood Association.
4. Roof sheathing shall be plywood (CDX) or OSB. Use "EXTERIOR" OR "EXPOSURE 1" identified plywood/OSB where exposed to weather. Plywood/OSB for floor sheathing shall be APA RATED SHEATHING or STURD-I-FLOOR Plywood/OSB. All plywood/OSB shall be glued with exterior type glue. All plywood/OSB shall conform to U.S. Product Standard PS 1-95 or PS2 Performance Standard for Wood-based Structural-Use Panels.
5. Nails shall be common. Nailing shall be per Table 2304.9.1 of the California Building Code unless noted otherwise on the plans or details.
6. All metal connectors shall be Simpson Strong-Tie connectors. The nails for these connectors shall be joist hanger nails as manufactured or specified by Simpson Co.
7. Provide 1x6 let-in bracing (at approx. 45 degrees) every 25'-0" maximum in stud walls not sheathed. Bracing shall run continuous from sill plate to top plate. Nail with 2-8d per stud and 3-8d each end to plates.
8. Provide fire stops at all intersections of stud walls at floor, ceiling and roof. Fire stops shall be 2x nominal thickness of wood and shall be the full width of the enclosed space. Place fire stops at a maximum spacing of 10'-0" in the vertical direction. Provide 2x fire stops in all furred spaces, vertical and horizontal, and at a maximum spacing of 10'-0" in each direction and at the same lines as fire stops in adjacent stud walls.
9. Top plates of all stud walls shall be 2 pieces the same size as studs. Splices to lap 4'-0" minimum and be nailed per the details.
10. Bolt holes in wood shall be 1/32" to 1/16" larger than the nominal bolt diameter. All bolts shall have standard cut washers under the head and nut unless noted otherwise.
11. All bolts shall be retightened prior to the application of sheathing, plaster, etc.
12. Structural members shall not be cut for pipes, etc. unless specifically detailed.
13. Provide 2x solid blocking between joists and rafters at all supports. Blocking shall be one piece and the full depth of the joist or rafter.
14. Cross bridging or solid blocking shall be provided at 8'-0" o.c. maximum for all floor joists more than 12" deep and at 10'-0" o.c. for all rafters more than 8" deep.
15. Provide double joists under partitions which are parallel to the floor joists.
16. Fasteners in preservative-treated wood (Anchor Bolt, Nails, Screws...etc) are to be approved silicon bronze, copper, stainless steel or hot-dipped zinc coated steel (CBC.2304.9.5)

STRUCTURAL STEEL:

1. All fabrication and erection shall conform to the latest edition of the AISC Manual of Steel Construction.
2. All welding shall comply with A.W.S. Specifications and is to be done by Certified Welders approved by the Department of Building and Safety. All welding will be done by electronic arc process using ASTM E70XX electrodes and shall be performed with approved electrodes as required per the A.I.S.C. specification. Welds are designed at full stress and must be done in the shop of a licensed fabricator. Special contiuous deputy inspection is required for all field welding.
3. All structural steel shall conform to ASTM A572, Grade 50.
4. Anchor bolts and unfinished bolts shall conform to ASTM F1554-99, GRADE 36, UNO.
5. All pipe shall conform to ASTM A53, Grade B (35 ksi).
6. All tube shall conform to ASTM A500, Grade B (46 ksi).
7. Plates and miscellaneous iron shall conform to ASTM A36, Unless Otherwise Noted.
8. All structural steel shall be fabricated and erected by a licensed fabricator approved by the Department of Building and Safety.
9. All grout under steel bearing plates shall be solid dry pack or non-shrink grout placed as directed by the manufacturer.
10. Weld at "Demand Critical Connection" of moment resisting frame beams to columns shall be in accordance with the AISC Seismic Design Manual and ANSI/AISC 341-5 Sections 7.3, 7.3a and 7.3b. Weld filler material must have a Charpy V-Notch toughness of 20ft-lb at -20°F.
11. Steel exposed to the weather and harsh environments shall be galvanized, UNO.

REVISIONS:

HCP ENGINEERING
2466 REDROCK DR
CORONA, CA, 92882
P: 951-738-0840
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GENERAL NOTES & TYPICAL DETAILS

REGISTERED PROFESSIONAL ENGINEER
HITESH C. PATEL
No. C50389
Exp. 6-30-19
CIVIL
STATE OF CALIFORNIA

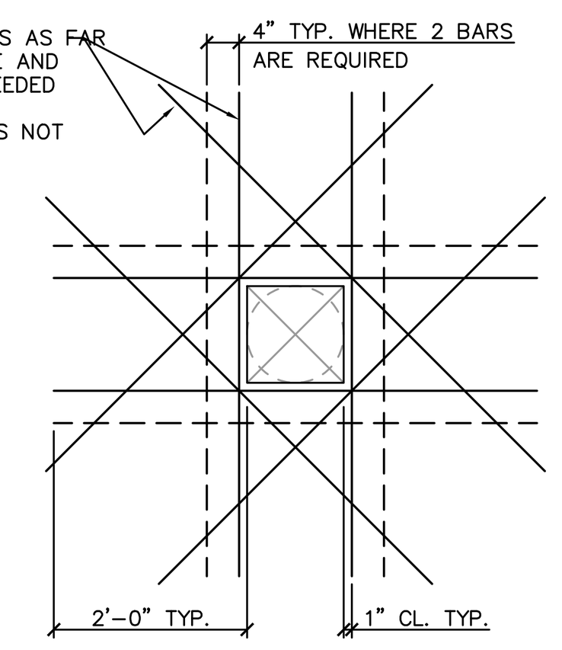
8/25/17

–

PLOT DATE: 8/25/17
SCALE:
DRAWN BY:
JOB NO. 2017-06
SHEET NO.
SN-1

21

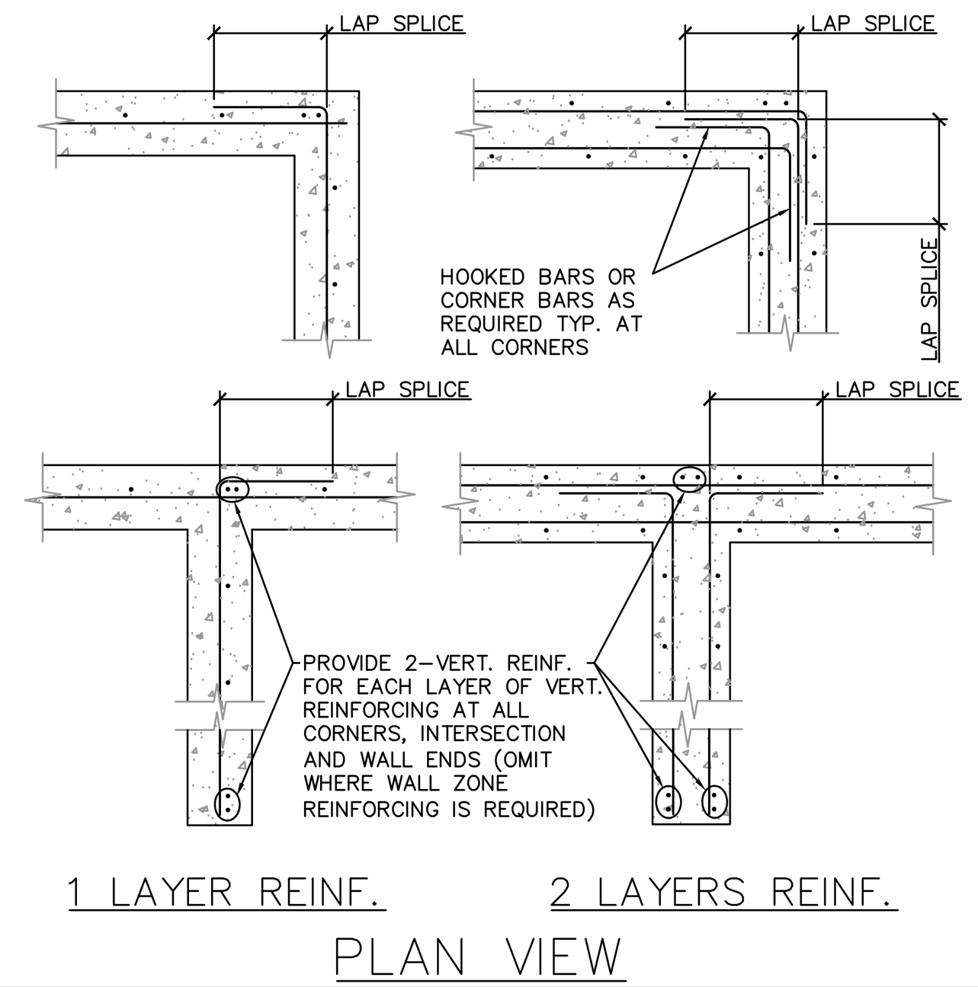
TYP. REINFORCING AT SLAB / WALL OPENING — 16



MAXIMUM DIMENSION OF OPENING	REINFORCING
12" TO 18"	1-#5 EA. SIDE
18" TO 24"	1-#5 EA. SIDE TOP AND BOTTOM.
24" TO 36"	2-#5 EA. SIDE TOP AND BOTTOM PLUS 1-#5x8'-6" DIAGONAL CENTERED ABOUT EA. CORNER TOP AND BOTTTOM.

NOTE:
1. NO OPENING OR CLUSTER OF OPENINGS TO BE CLOSER THAN 2'-0" FROM COLUMN FACE UNLESS APPROVED BY STRUCTURAL ENGINEER. CLUSTERS OF SMALL HOLES THAT ARE LARGER THAN 12" TO BE REINFORCED AS ONE OPENING.
2. SLAB/WALL OPENING REINFORCING IS IN ADDITION TO TYPICAL SLAB/WALL REINFORCING

HORIZ. REINF. AT INTERSECTING WALL & FTG — 11

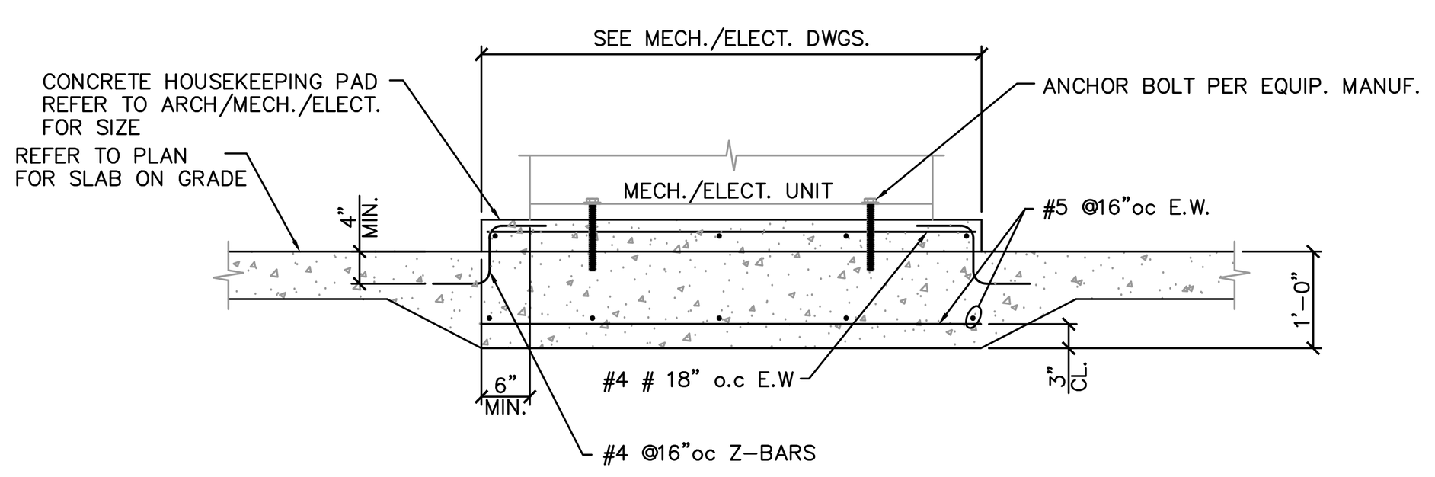

LAP SPLICE
HOOKED BARS OR CORNER BARS AS REQUIRED TYP. AT ALL CORNERS
PROVIDE 2-VERT. REINF. FOR EACH LAYER OF VERT. REINFORCING AT ALL CORNERS, INTERSECTION AND WALL ENDS (OMIT WHERE WALL ZONE REINFORCING IS REQUIRED)
1 LAYER REINF.
2 LAYERS REINF.
PLAN VIEW

TYP. REINFORCING SPLICE — 1

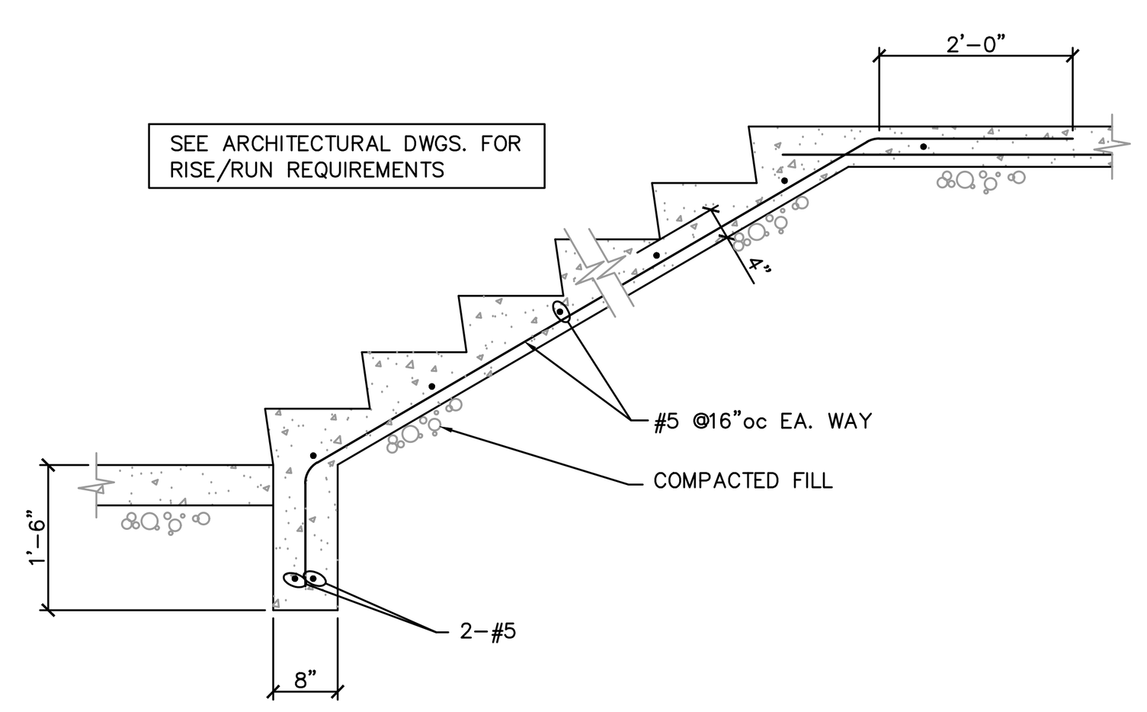
BAR SIZE	F'c = 3000 p.s.i.		F'c = 4000 p.s.i.		F'c = 4500 p.s.i.		F'c = 5000 p.s.i.	
	TOP BARS HORIZONTAL WALL BARS	BOTTOM BARS & VERT. BARS	TOP BARS HORIZONTAL WALL BARS	BOTTOM BARS & VERT. BARS	TOP BARS HORIZONTAL WALL BARS	BOTTOM BARS & VERT. BARS	TOP BARS HORIZONTAL WALL BARS	BOTTOM BARS & VERT. BARS
#3	18"	16"	16"	16"	16"	16"	16"	16"
#4	28"	22"	24"	20"	24"	18"	22"	18"
#5	42"	32"	36"	28"	34"	26"	32"	26"
#6	56"	44"	50"	38"	46"	36"	44"	34"
#7	82"	64"	72"	54"	68"	52"	64"	50"
#8	94"	72"	82"	62"	76"	60"	72"	56"
#9	106"	82"	92"	70"	86"	66"	82"	64"
#10	118"	92"	102"	80"	96"	74"	92"	70"
#11	132"	102"	114"	88"	108"	82"	102"	78"

MINIMUM COVER SHALL BE GREATER OF 3/4" OR BAR DIAMETER.
MINIMUM BAR SPACING (NOT LAPPING BARS) SHALL BE 2 BAR DIAMETERS CLEAR.

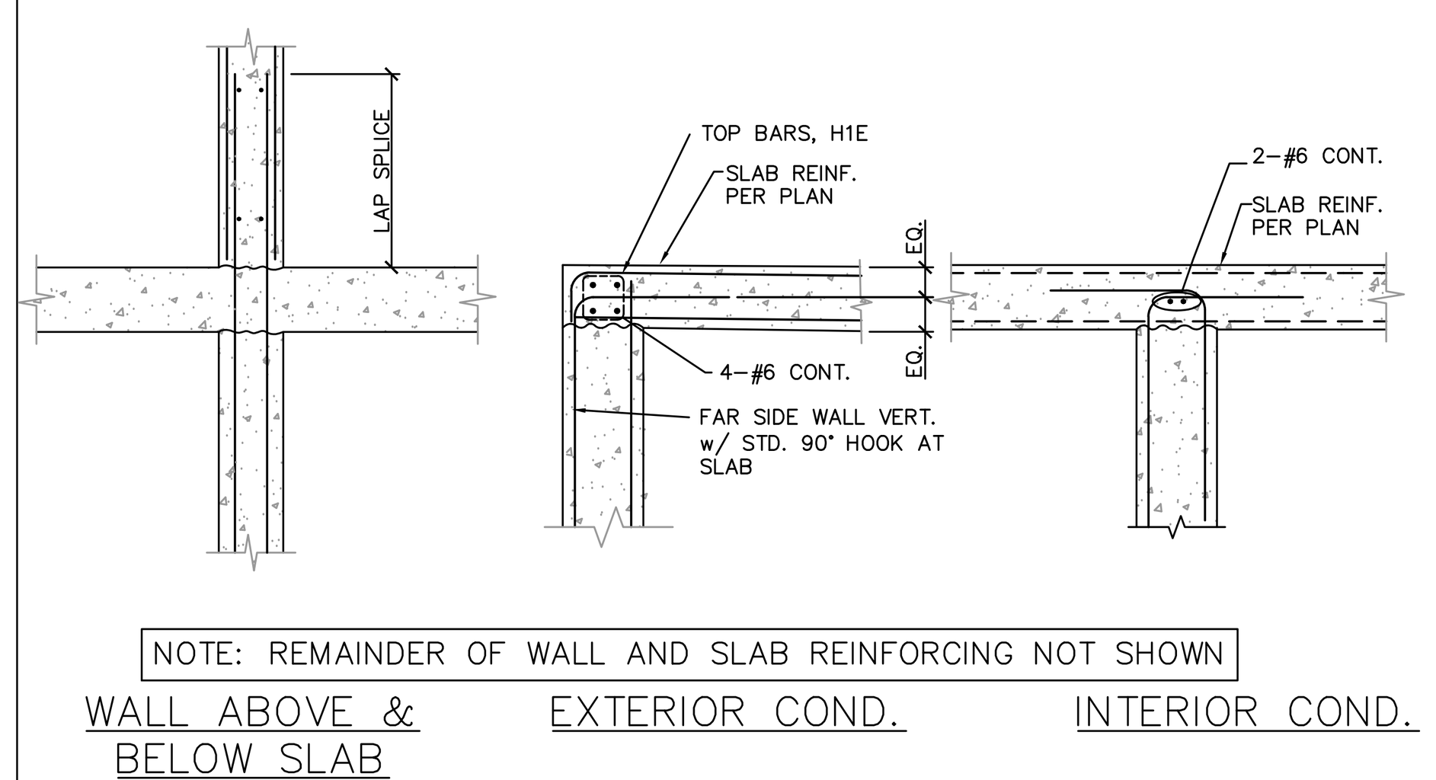
HOUSEKEEPING PAD — 22



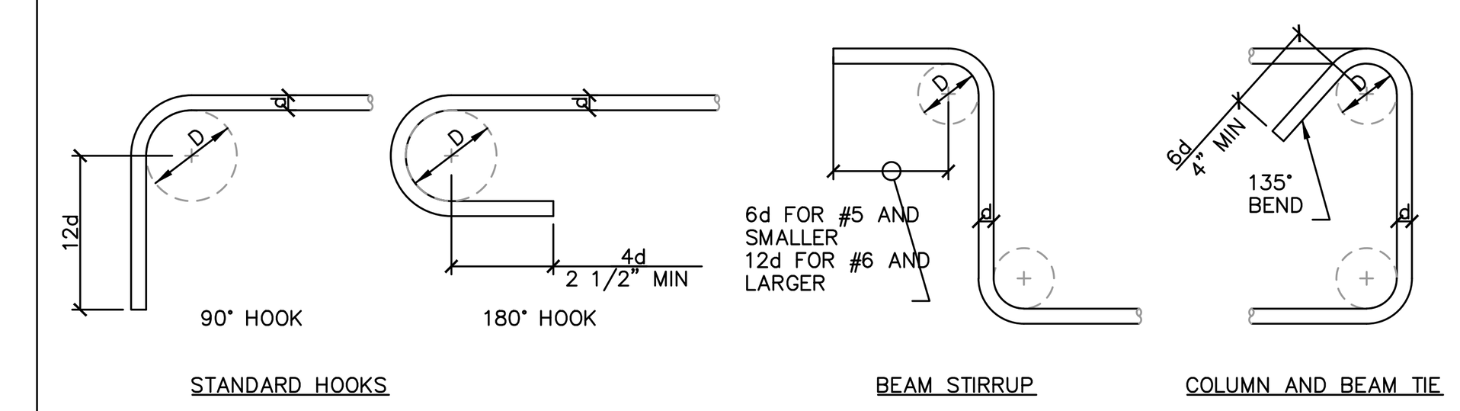
TYP. STAIRS ON GRADE — 17

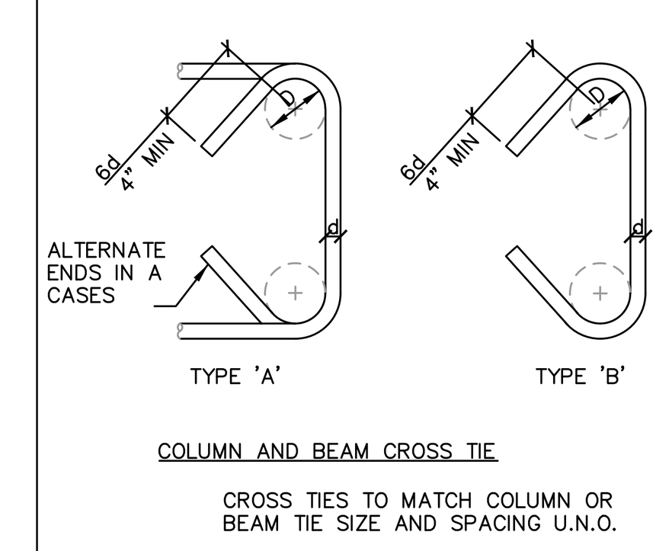


TYP. WALL TO SLAB REINFORCING — 12



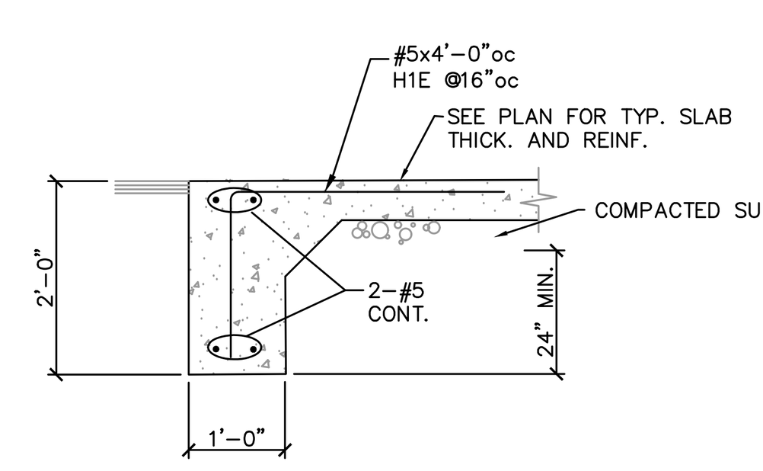
TYP. REINFORCING DETAILS — 2



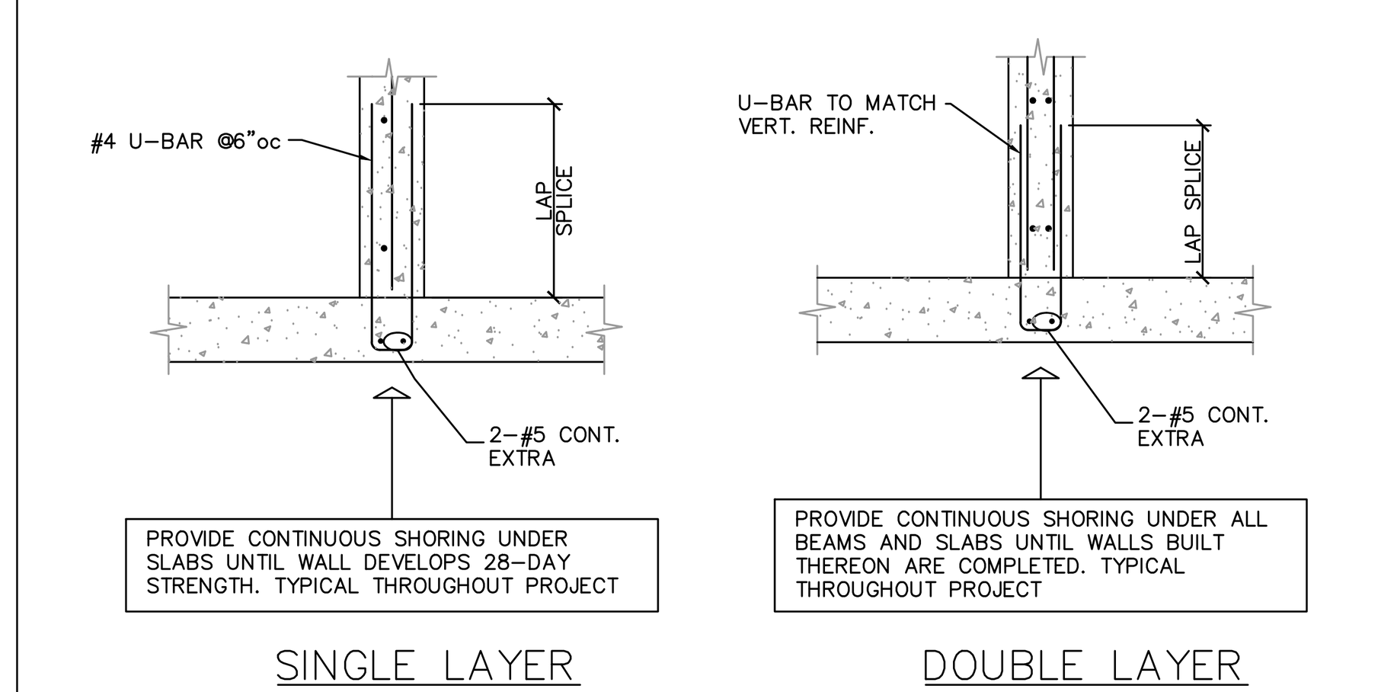


HOOK/BEND SCHEDULE		
BAR SIZE	"D" FOR TIES & STIRRUPS ONLY	"D" FOR NON-TIES & NON-STIRUPS ONLY
#3	1 1/2"	2 1/4"
#4	2"	3"
#5	2 1/2"	3 3/4"
#6	4 1/2"	4 1/2"
#7-#8	-	6d
#9-#11	-	8d
#12-#18	-	10d

TYP SLAB EDGE — 18



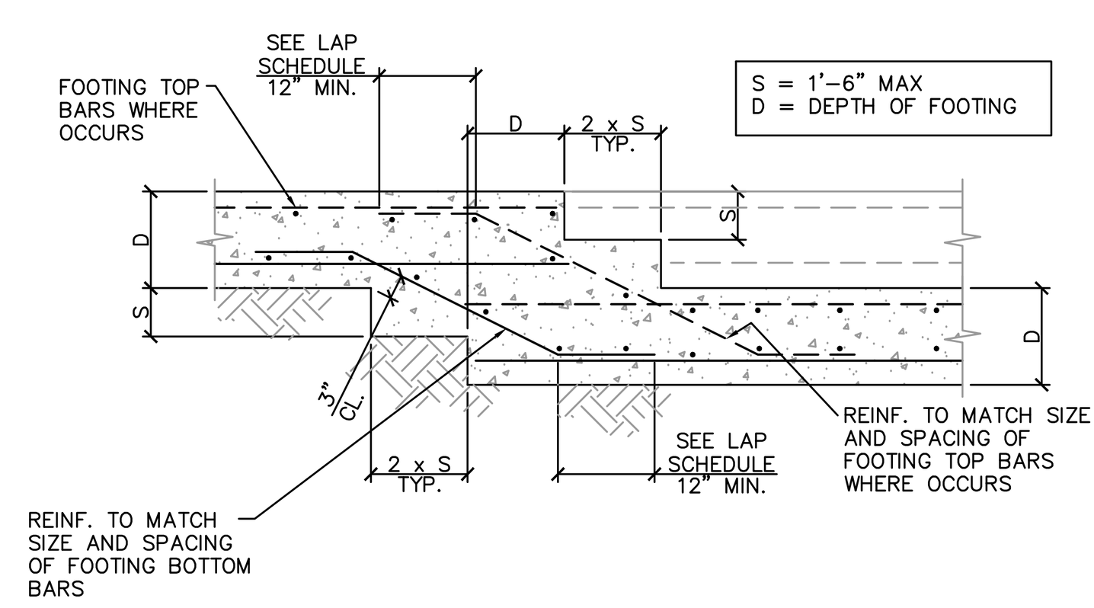
TYPICAL CONCRETE WALL AT SLAB — 13


#4 U-BAR @6"oc
U-BAR TO MATCH VERT. REINF.
LAP SPLICE
2-#5 CONT. EXTRA
PROVIDE CONTINUOUS SHORING UNDER SLABS UNTIL WALL DEVELOPS 28-DAY STRENGTH. TYPICAL THROUGHOUT PROJECT
PROVIDE CONTINUOUS SHORING UNDER ALL BEAMS AND SLABS UNTIL WALLS BUILT THEREON ARE COMPLETED. TYPICAL THROUGHOUT PROJECT
SINGLE LAYER
DOUBLE LAYER

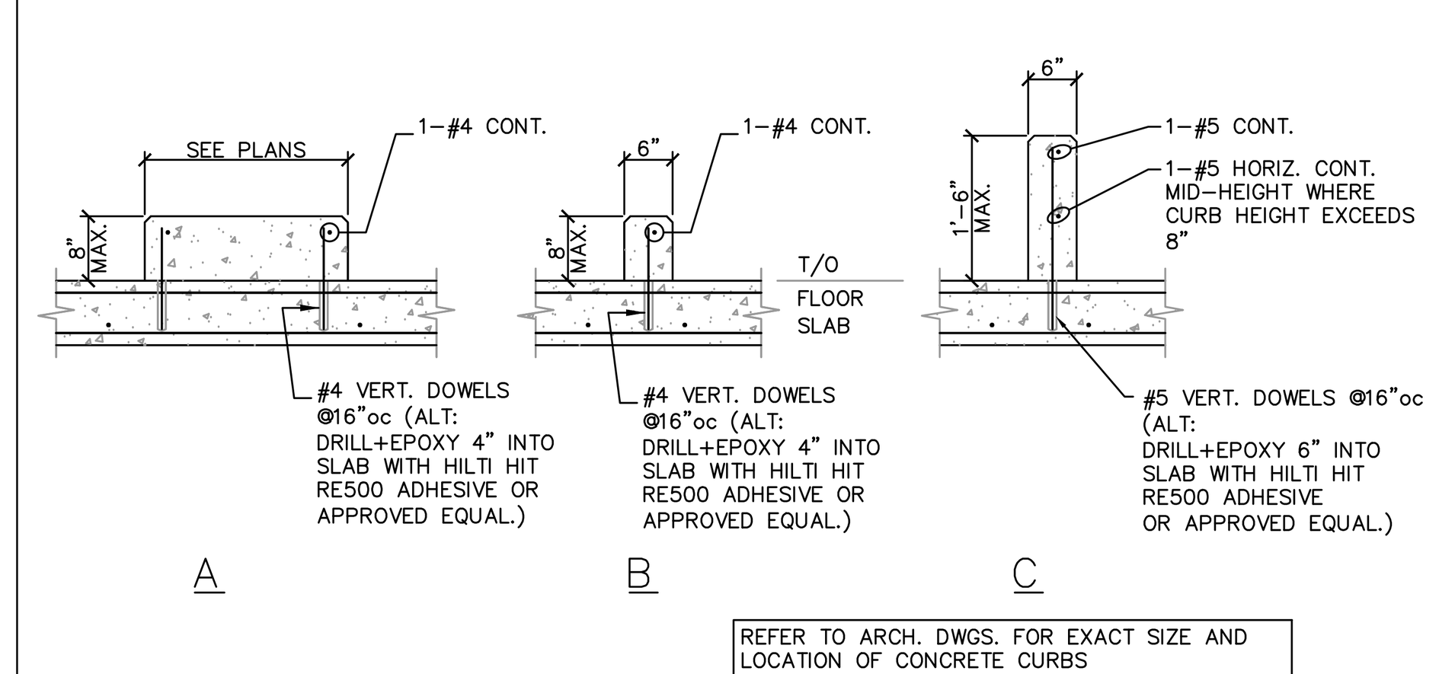
FIRE RESISTANCE STRUCTURAL COMPONENTS — 3

RATED FIRE RESISTIVE PERIOD FOR STRUCTURAL COMPONENTS AND MATERIALS	
WALLS: CONCRETE 8" THICK OR GREATER CONCRETE MASONRY UNIT 8" THICK OR GREATER FULLY GROUTED	4 Hr 4 Hr
CONCRETE COLUMNS: MIN COVER TO TIES 1 1/2"	4 Hr
CONCRETE BEAMS: MIN COVER TO TIES 1 1/2"	4 Hr
CONCRETE SLABS: MILD REINFORCED SLABS MIN BOT COVER 1" PT SLAB INTERIOR SPANS MIN BOT STRAND COVER 1" PT SLAB EXTERIOR SPANS MIN BOT STRAND COVER 1 1/2"	4 Hr 3 Hr 2 Hr

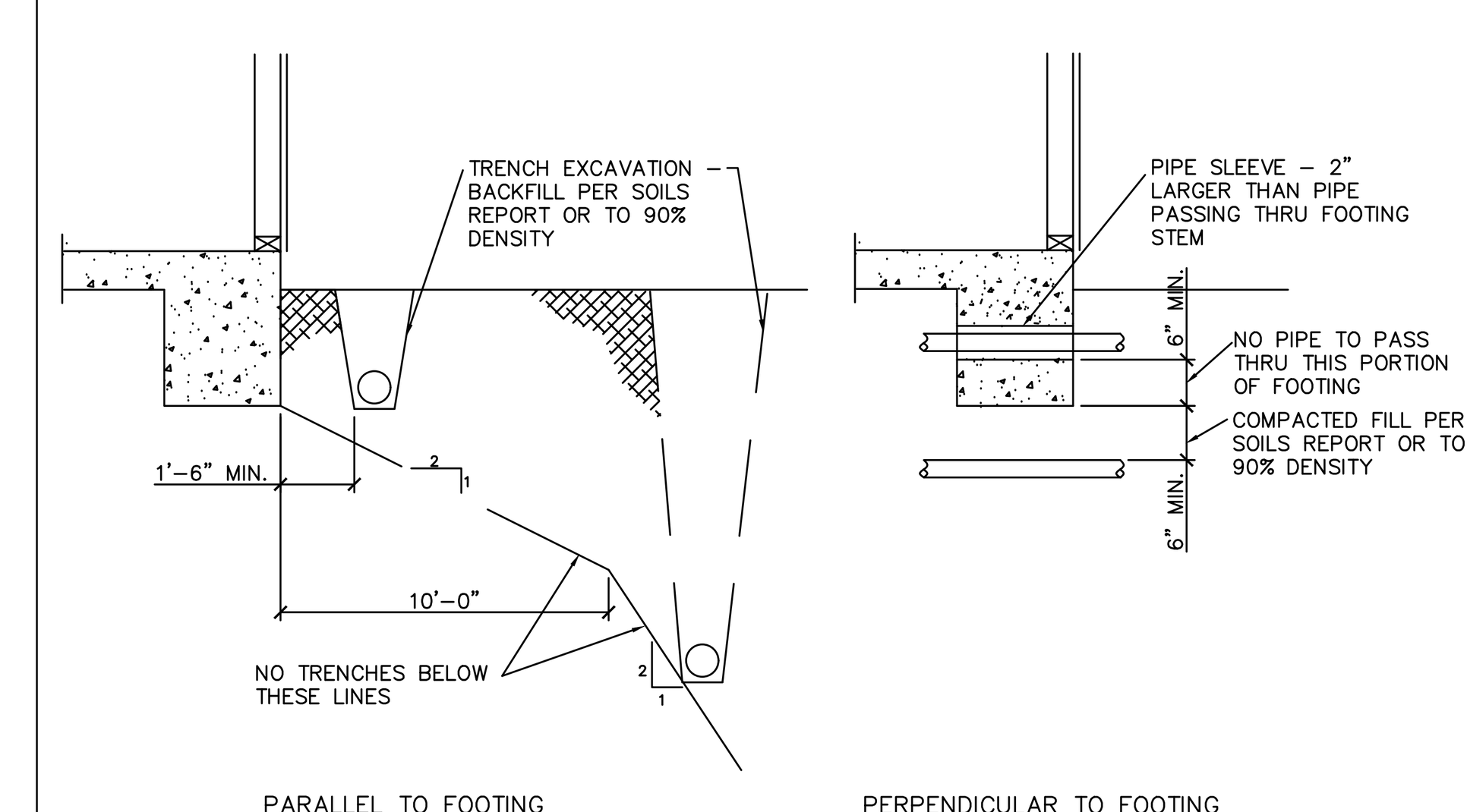
TYPICAL STEPPED FOOTING — 19



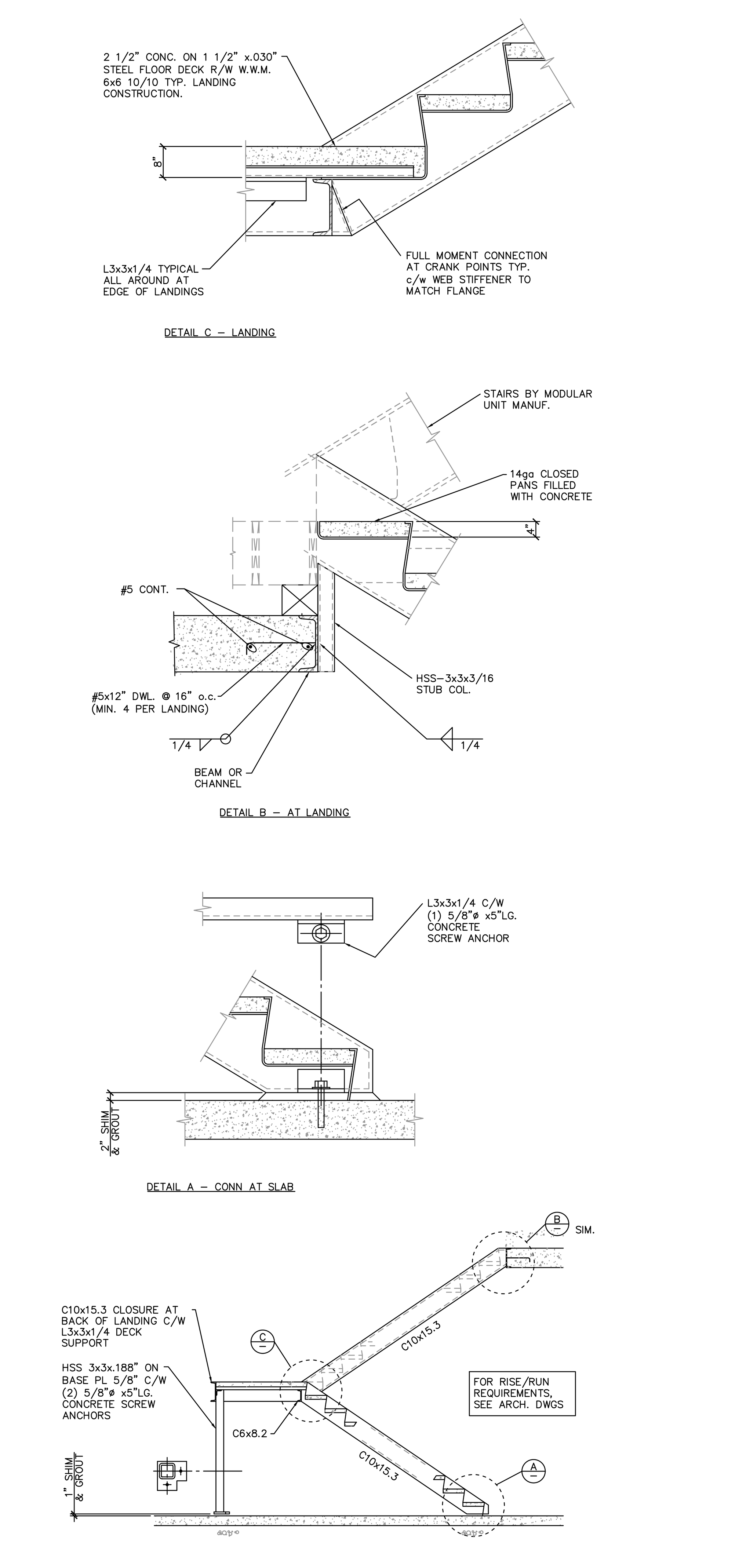
TYPICAL CURB ON SLAB — 14


SEE PLANS
1-#4 CONT.
#4 VERT. DOWELS @16"oc (ALT: DRILL+EPOXY 4" INTO SLAB WITH HILTI HIT RE500 ADHESIVE OR APPROVED EQUAL.)
T/O FLOOR SLAB
1-#5 CONT.
1-#5 HORIZ. CONT. MID-HEIGHT WHERE CURB HEIGHT EXCEEDS 8"
#5 VERT. DOWELS @16"oc (ALT: DRILL+EPOXY 6" INTO SLAB WITH HILTI HIT RE500 ADHESIVE OR APPROVED EQUAL.)
A
B
C
REFER TO ARCH. DWGS. FOR EXACT SIZE AND LOCATION OF CONCRETE CURBS

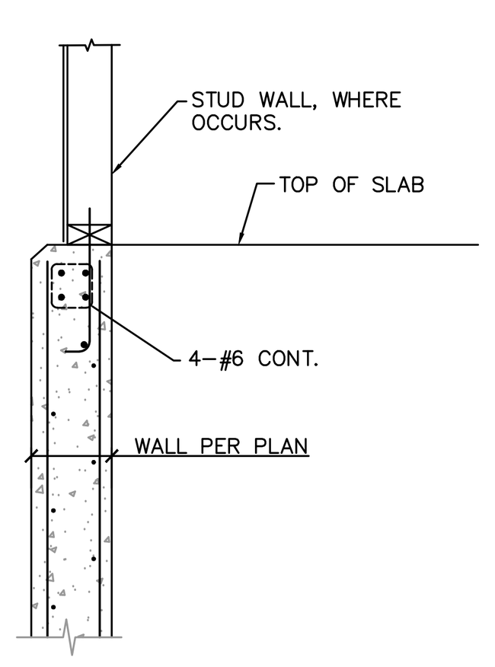
TRENCHING DETAIL — 4


TRENCH EXCAVATION BACKFILL PER SOILS REPORT OR TO 90% DENSITY
1'-6" MIN.
10'-0"
NO TRENCHES BELOW THESE LINES
PIPE SLEEVE - 2" LARGER THAN PIPE PASSING THRU FOOTING STEM
NO PIPE TO PASS THRU THIS PORTION OF FOOTING
COMPACTED FILL PER SOILS REPORT OR TO 90% DENSITY
PARALLEL TO FOOTING
PERPENDICULAR TO FOOTING

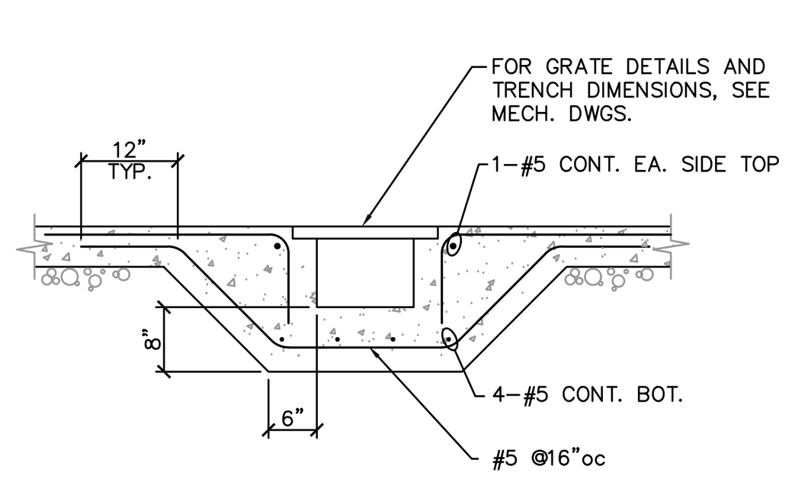
PREFAB. STAIRS — 23


2 1/2" CONC. ON 1 1/2" x.030" STEEL FLOOR DECK R/W W.W.M. 6x6 10/10 TYP. LANDING CONSTRUCTION.
L3x3x1/4 TYPICAL ALL AROUND AT EDGE OF LANDINGS
FULL MOMENT CONNECTION AT CRANK POINTS TYP. c/w WEB STIFFENER TO MATCH FLANGE
DETAIL C - LANDING
STAIRS BY MODULAR UNIT MANUF.
14ga CLOSED PANS FILLED WITH CONCRETE
#5 CONT.
#5x12" DWL. @ 16" o.c (MIN. 4 PER LANDING)
HSS-3x3x3/16 STUB COL.
BEAM OR CHANNEL
DETAIL B - AT LANDING
L3x3x1/4 C/W (1) 5/8"ø x5"LG. CONCRETE SCREW ANCHOR
DETAIL A - CONN AT SLAB
C10x15.3 CLOSURE AT BACK OF LANDING C/W L3x3x1/4 DECK SUPPORT
HSS 3x3x.188" ON BASE PL 5/8" C/W (2) 5/8"ø x5"LG. CONCRETE SCREW ANCHORS
C6x8.2
C10x15.3
FOR RISE/RUN REQUIREMENTS, SEE ARCH. DWGS
SIM.

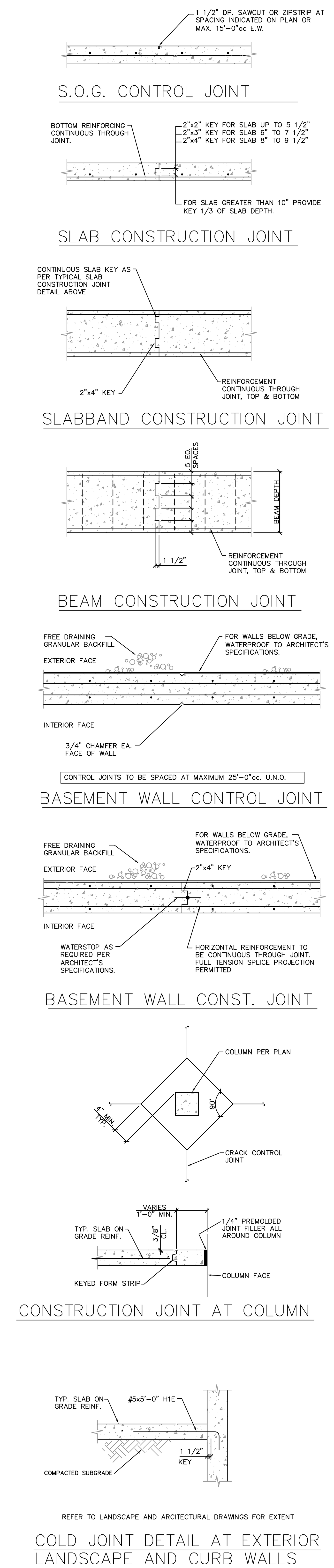
STUD WALL ABOVE ON CONC. WALL — 20



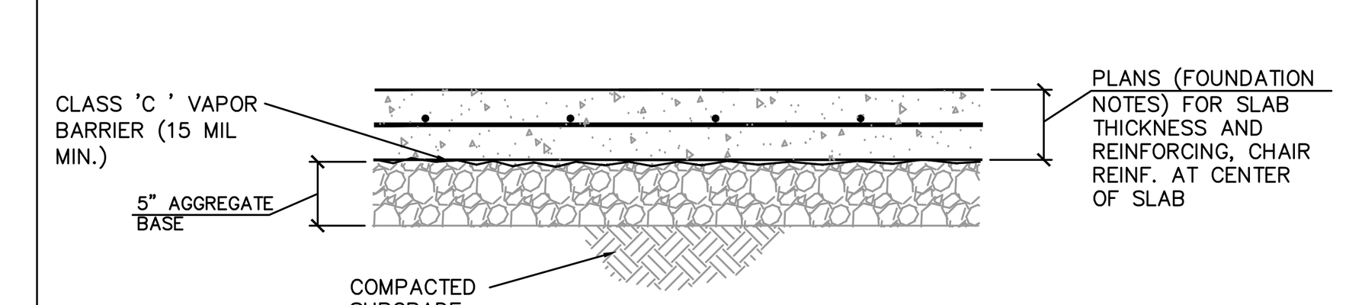
TYP. TRENCH — 15



TYPICAL CONSTRUCTION JOINTS — 10


1 1/2" OP. SAWCUT OR ZIPSTRIP AT SPACING INDICATED ON PLAN OR MAX. 15'-0"oc E.W.
S.O.G. CONTROL JOINT
BOTTOM REINFORCING CONTINUOUS THROUGH JOINT.
2"x2" KEY FOR SLAB UP TO 5 1/2"
2"x3" KEY FOR SLAB 6" TO 7 1/2"
2"x4" KEY FOR SLAB 8" TO 9 1/2"
FOR SLAB GREATER THAN 10" PROVIDE KEY 1/3 OF SLAB DEPTH.
SLAB CONSTRUCTION JOINT
CONTINUOUS SLAB KEY AS PER TYPICAL SLAB CONSTRUCTION JOINT DETAIL ABOVE
2"x4" KEY
REINFORCEMENT CONTINUOUS THROUGH JOINT, TOP & BOTTOM
SLABBAND CONSTRUCTION JOINT
BEAM DEPTH
REINFORCEMENT CONTINUOUS THROUGH JOINT, TOP & BOTTOM
BEAM CONSTRUCTION JOINT
FREE DRAINING GRANULAR BACKFILL
FOR WALLS BELOW GRADE, WATERPROOF TO ARCHITECT'S SPECIFICATIONS.
EXTERIOR FACE
INTERIOR FACE
3/4" CHAMFER EA. FACE OF WALL
CONTROL JOINTS TO BE SPACED AT MAXIMUM 25'-0"oc. U.N.O.
BASEMENT WALL CONTROL JOINT
2"x4" KEY
WATERSTOP AS REQUIRED PER ARCHITECT'S SPECIFICATIONS.
HORIZONTAL REINFORCEMENT TO BE CONTINUOUS THROUGH JOINT. FULL TENSION SPLICE PROJECTION PERMITTED
BASEMENT WALL CONST. JOINT
COLUMN PER PLAN
CRACK CONTROL JOINT
TYP. SLAB ON GRADE REINF.
1/4" PREMOLDED JOINT FILLER ALL AROUND COLUMN
COLUMN FACE
KEYED FORM STRIP
CONSTRUCTION JOINT AT COLUMN
TYP. SLAB ON GRADE REINF.
#5x5'-0" H1E
COMPACTED SUBGRADE
1 1/2" KEY
REFER TO LANDSCAPE AND ARCITECTURAL DRAWINGS FOR EXTENT
COLD JOINT DETAIL AT EXTERIOR LANDSCAPE AND CURB WALLS

TYPICAL SLAB-ON-GRADE — 5



NOTES:
1. PROVIDE CONSTRUCTION JOINT OR CONTROL JOINT TO ENCLOSE APPROX. SQUARE AREA NOT EXCEEDING 400 SQ. FT. (15'-0" o.c. MAX.), LOCATE UNDER PARTITION WHEREVER POSSIBLE, SUBMIT LAYOUT FOR APPROVAL
2. WATERPROOF VAPOR RETARDER MIN. OF 15 POLYTHYLENE SHEETING, RAVEN INDUSTRIES "VAPORBLOCK 15", STEGO INDUSTRIES 15 MIL "STEGO WRAP" OR W.R. MEADOWS SEALTIGHT 15 MIL "PERMINATOR". INSTALLED PER ASTM E-1643-98

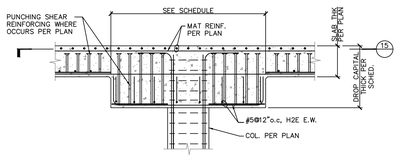
Arris STUDIO ARCHITECTS
ADDRESS
1306 JOHNSON AVENUE
SAN LUIS OBISPO, CA 93401
CONTACT
805.547.2240
ARRIS-STUDIO.COM
ARCHITECT OF RECORD/CONSULTANT
CONSULTANT
HCP ENGINEERING
No. C50389
Exp. 6-30-2017
8/25/17

CLIENT
SRI KRISHNA ENTERPRISE
300 GATEWAY BOULEVARD
SOUTH SAN FRANCISCO, CA 94080

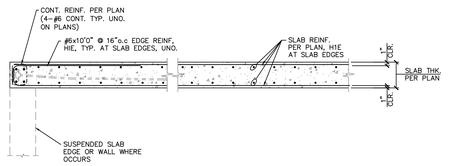
PROJECT
Home2 Suites by Hilton
550 GATEWAY BOULEVARD
SOUTH SAN FRANCISCO, CA

#	REVISION	DATE

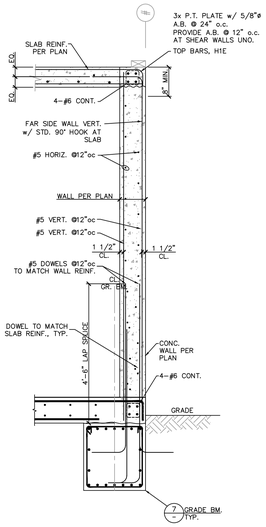
PROJECT
2017-06
DATE
March 9, 2017
SHEET
SD1.0



20 **TYP. COLUMN AT DROP CAPITAL**



15 **TYP. RAISED SLAB**



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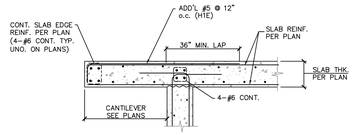
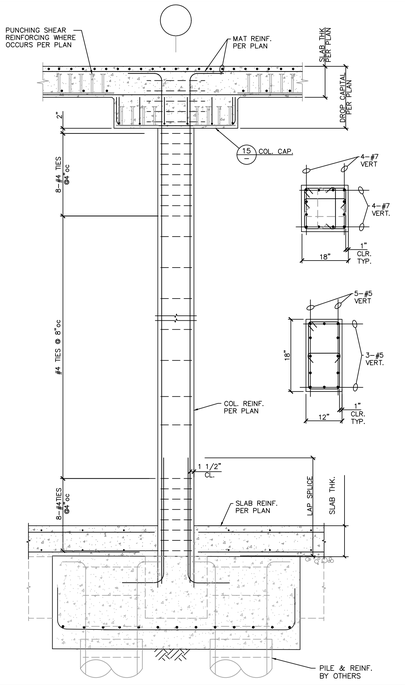


21 16 **CANTILEVERED SLAB** 11 6 1



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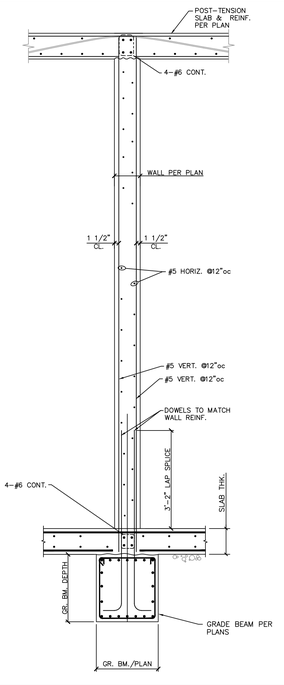

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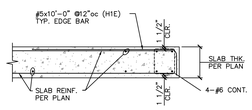
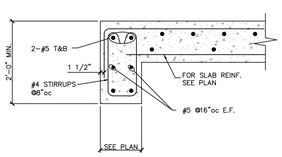
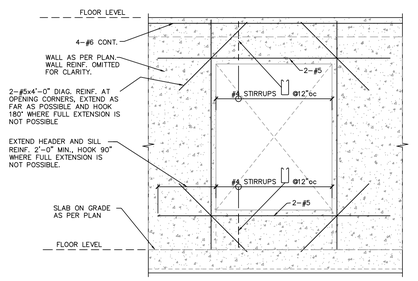
12



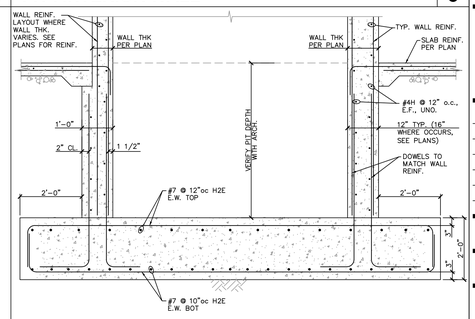
13 **TYP. UNSUPPORTED SLAB EDGE**



14 **LINTEL BEAM**



9 **TYP. REINF. AROUND DOOR & WINDOWS**



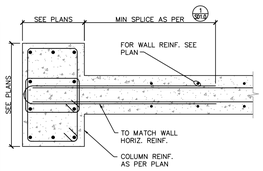
4

NOTES

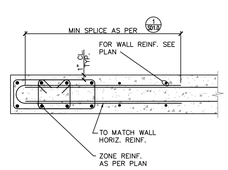
1. REFER TO DRAWING SN–1 FOR GENERAL NOTES
2. REFER TO PLAN FOR LOCATIONS OF ZONES AND WALL TYPES
3. SHEAR WALL CONCRETE STRENGTH SHALL BE THE SAME AS THAT OF WALLS AT SAME LEVEL.
4. VERTICAL ZONE REINFORCING SHALL DEVELOP INTO FOUNDATION, OR INTO CONCRETE WALL OR SLAB BELOW IF THE REINFORCING STARTS AT AN INTERMEDIATE LEVEL. HOOK REBAR ENDS IF THE DEVELOPMENT LENGTH DOES NOT SATISFY THE REQUIREMENTS IN THE GENERAL NOTES.
5. VERTICAL ZONE REINFORCING SHALL BE CONTINUOUS THROUGH ALL FLOORS. LAP SPLICE VERTICAL REBAR AT FLOOR LEVEL IF NECESSARY.
6. FOR OFFSET ZONES AT ADJACENT LEVELS, MAKE CONTINUOUS BARS WHEREVER POSSIBLE. OTHERWISE TERMINATE REBARS FROM LOWER LEVEL WITH HOOKS INTO SLAB, AND START NEW REBARS OF UPPER ZONE WITH DOWELS INTO WALL OR SLAB BELOW.
7. ALL WALL VERTICAL REINFORCING SHALL BE PLACED OUTSIDE THE HORIZ. REINFORCING. HOOK ALL WALL VERTICAL REINFORCING INTO SLABS AT TOP OF WALLS.
8. ALL ZONE TIES TO BE FULLY ENCLOSED WITH 135° HOOKS 4" LONG EACH END

ZONE REINFORCING SCHEDULE

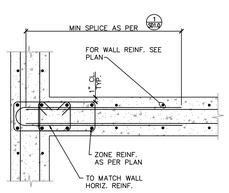
ZONE (Z#)	ZONE LENGHT (in)	WALL THICK (in)	VERTICAL REINFORCING	NOTES
Z6	12"	12"	6–#5	
Z10	24"	12"	10–#5	PROVIDE TYP. WALL REINF. IN ADDITION TO ZONE REINF.
Z18	36"	12"	18–#5	PROVIDE TYP. WALL REINF. IN ADDITION TO ZONE REINF.



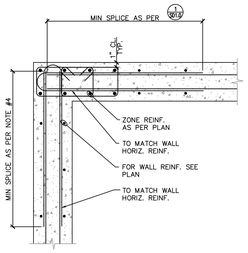
HAMMER



WALL ENDS

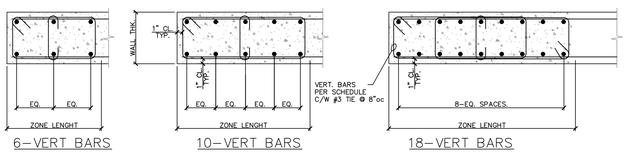


TEE – INTERSECTION



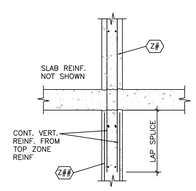
CORNERS

WALL REINFORCING CONFIGURATIONS.



ALL DIMENSIONS ARE TO THE OUTSIDE OF TIES OR CONCRETE

VERTICAL REINFORCING CONFIGURATIONS.



CONT. VERTICAL REINF.

19 **BOUNDRY (ZONE) REINFORCING**

HCP ENGINEERING
2486 REDROCK DR.
CORONA, CA 92881
P: 951.738.0840
F: 951.738.1432
www.hcpengineeringco.com

HOME2
SOUTH SAN FRANCISCO, CA

REGISTERED PROFESSIONAL ENGINEER
HITESH C. PATEL
No. C50389
Exp. 6–30–2017
CIVIL
STATE OF CALIFORNIA
8/25/17

CONCRETE DETAILS

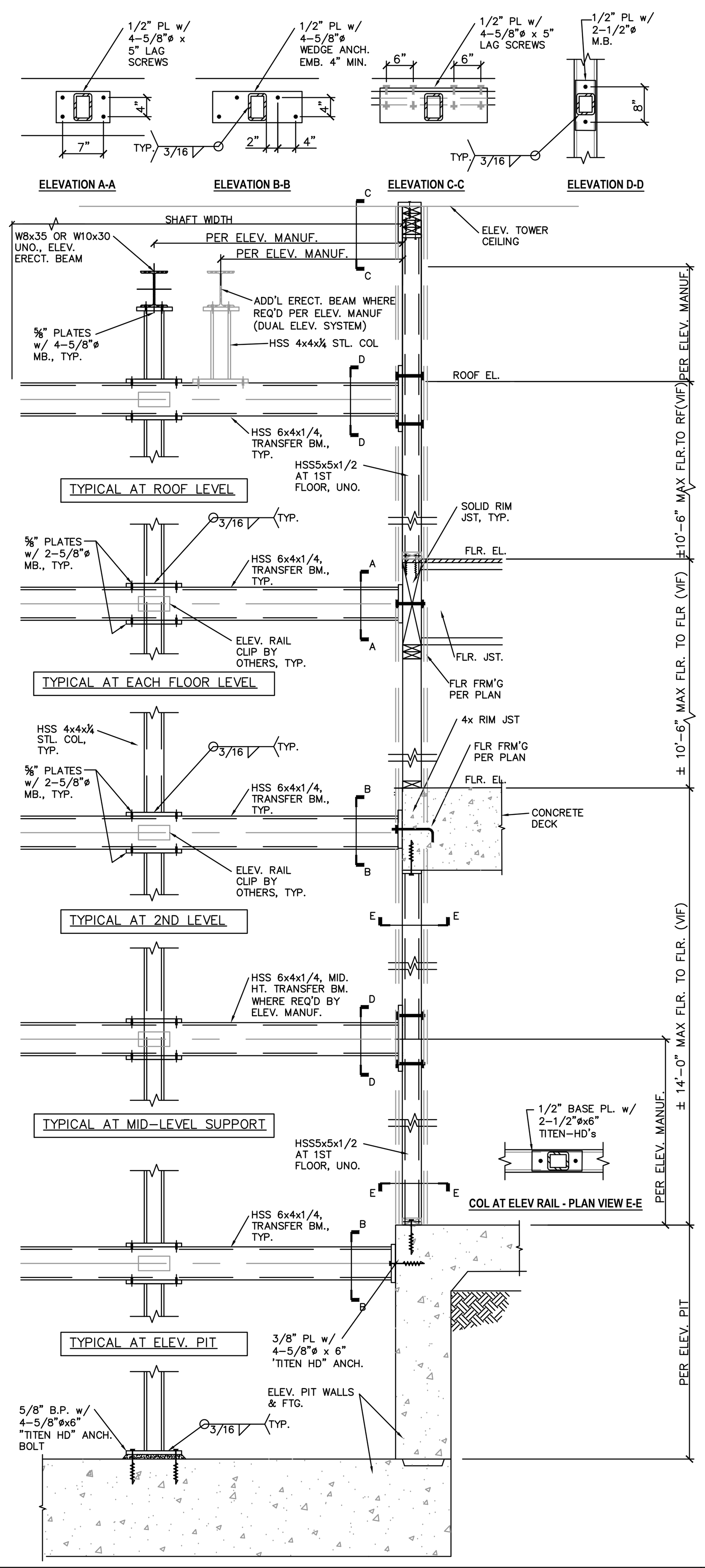
REVISION

PROJECT No.: 2017-06

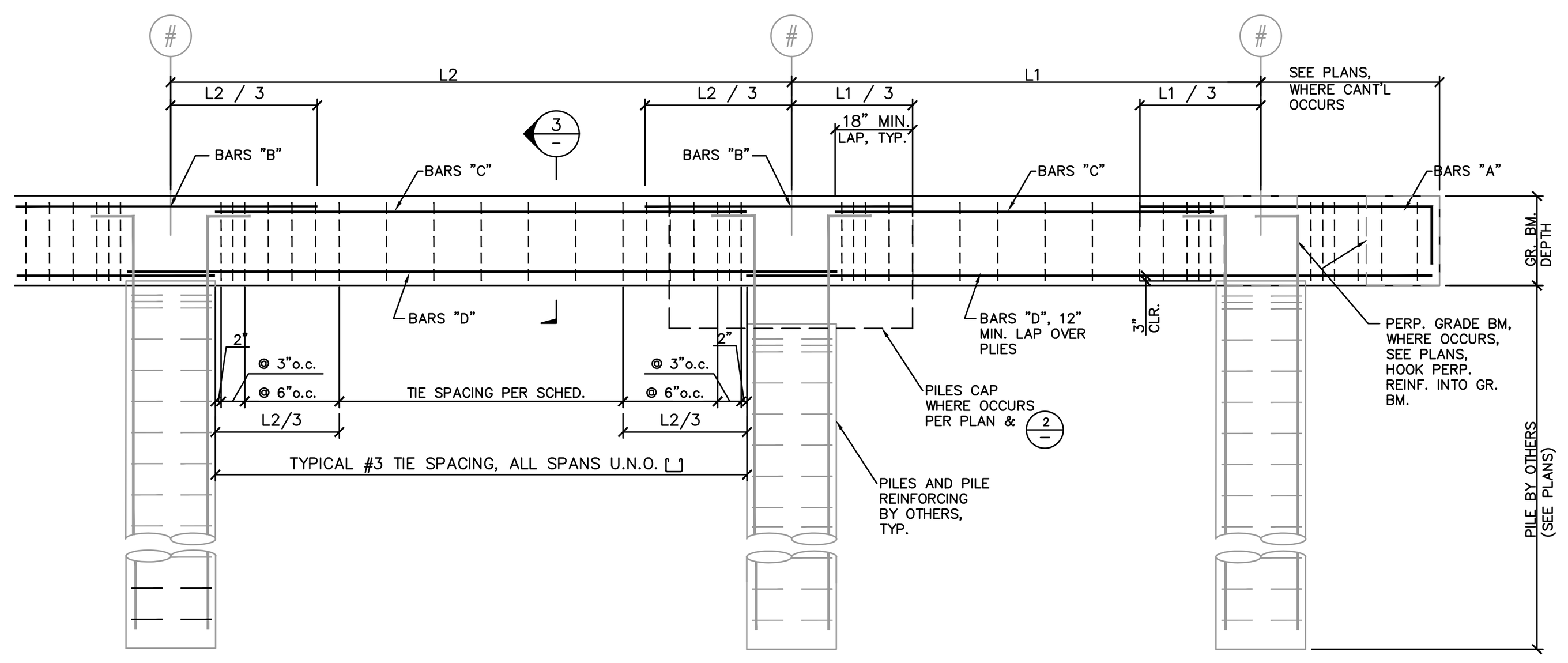
PLOT DATE: August 25, 2017

SHEET: **SD1.1**

F:\HCP ENGINEERING\PROJECTS\2017-PROJECTS\2017-06 Home2-SouthSanFrancisco (VJPatel)\SD-1.2-h2sf CONC WALL DTL.dwg, 8/25/2017 12:57:56 PM, _AutoCAD PDF (General Documentation).pc3

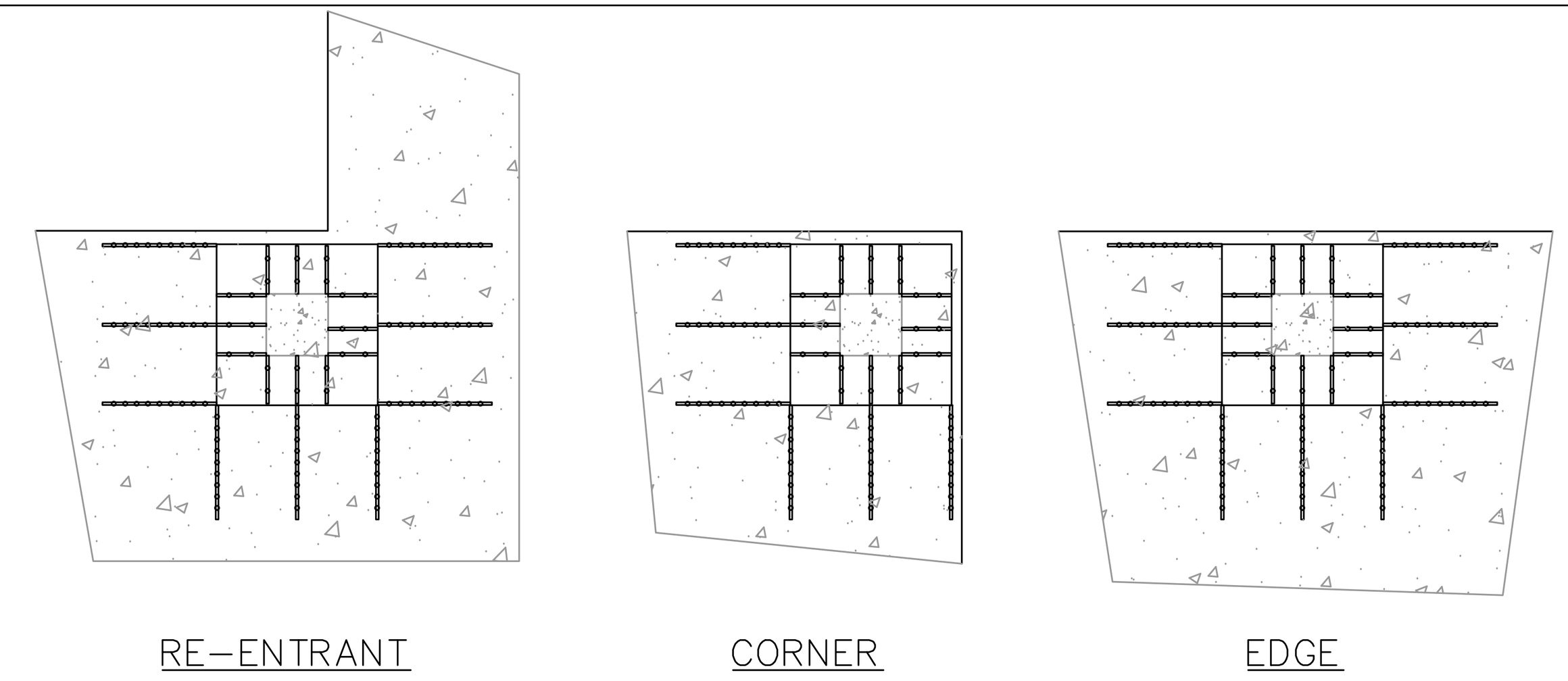


CENTER RAIL STEEL COL. AT ELEVATOR 12

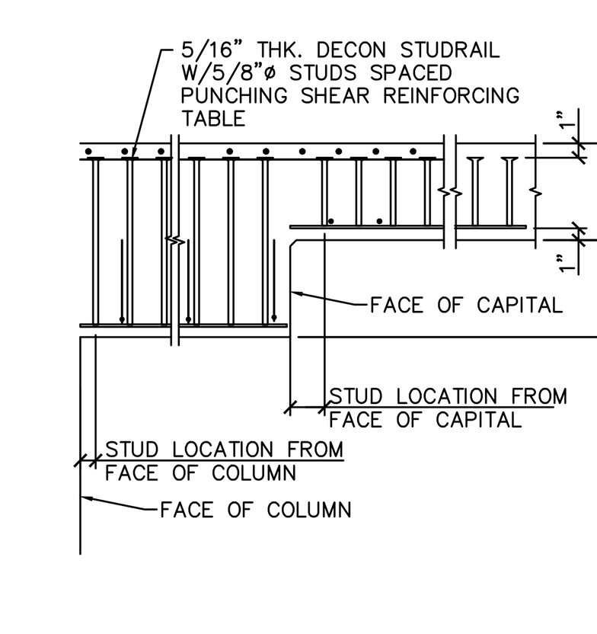


GRADE BEAM SCHEDULE								
GR. BM. MARK	WIDTH	DEPTH	BARS "A" TOP BARS	BARS "B" TOP BARS	BARS "C" TOP BARS	BARS "D" BOTT BARS	TIES SIZE & SPACING	TIES HOOP TYPE
GB-1	24"	30"	7-#8	7-#8	7-#8	6-#8	#4 @ 12" o.c.	SINGLE
GB-2	30"	30"	5-#8	5-#8	5-#8	5-#8	#4 @ 6" o.c.	DOUBLE

TYPICAL GRADE BEAM 7



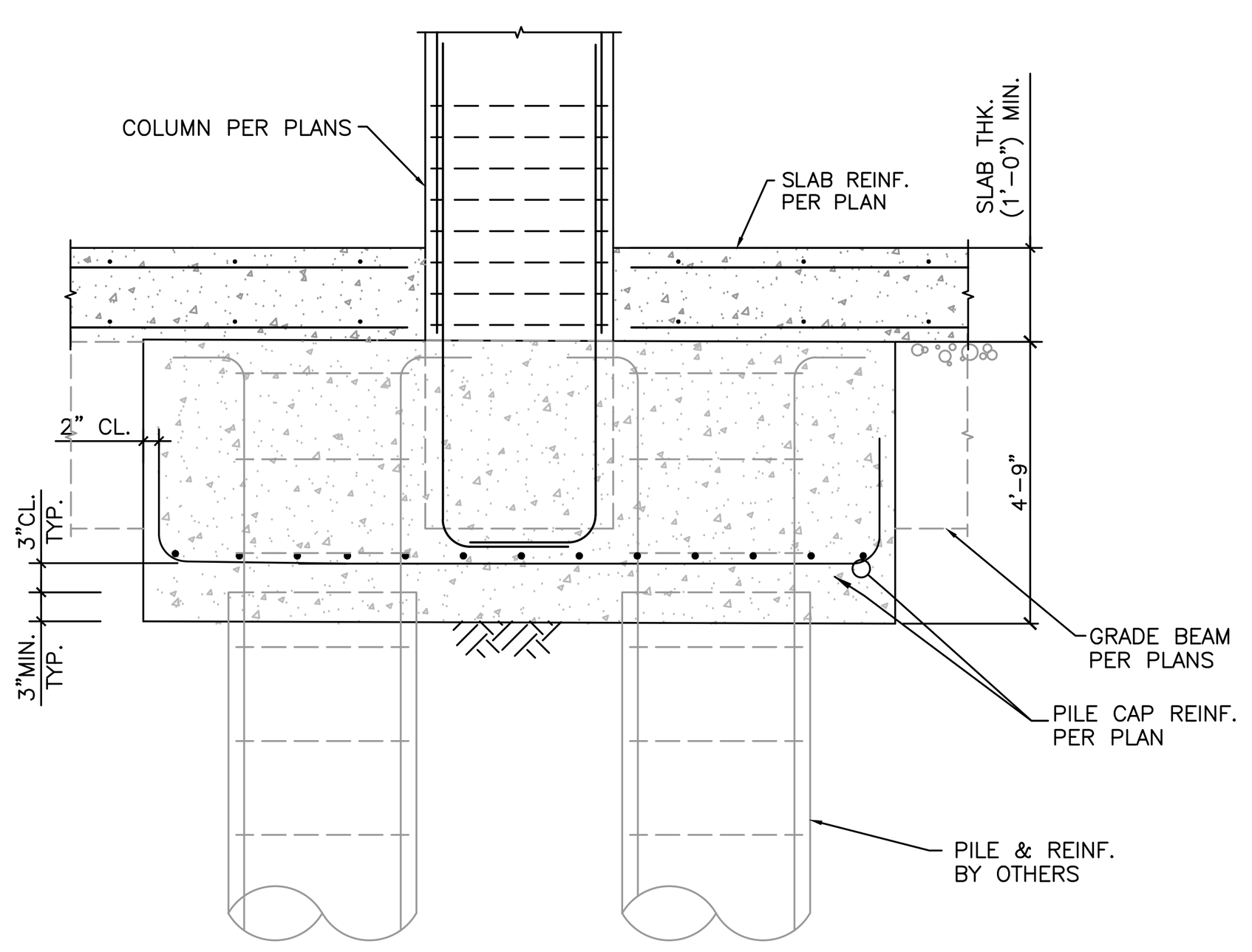
STUD RAIL ARRANGEMENT



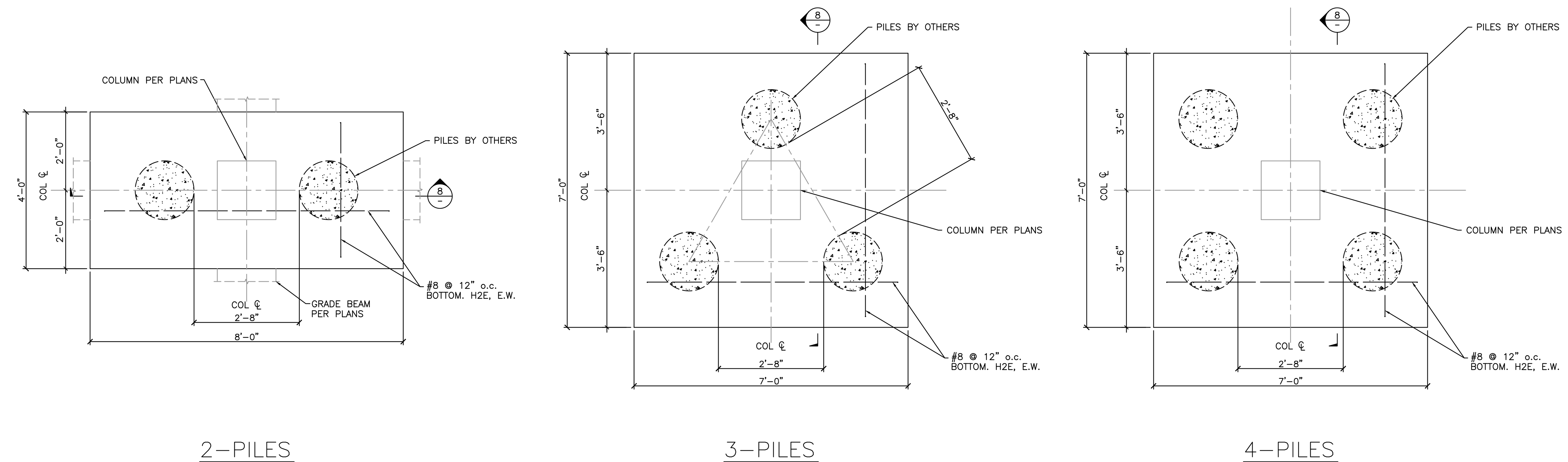
STUD LOCATION

Column ID SR#	Number of studs/rail	Stud diameter (in)	Number of stud rails each face	Stud Arrangement	Stud Location (inches) from face of column or column capital "(#)" = stud number * = stud located from face of column capital ex: (3)16.25 = Stud #3 located 16.25" from face of column (12)5.50* = Stud #12 located 5.50" from face of column capital					
10	7	0.63	3	Interior	5.50	11.00	6.50*	13.00*	19.75*	26.25*
					(7)33.00*					
11	12	0.63	3	Interior	5.50	11.00	3.25*	6.50*	9.75*	13.00*
					(7)16.50*	(8)19.75*	(9)23.00*	(10)26.25*	(11)29.75*	(12)33.00
14	8	0.63	3	Interior	5.50	11.00	6.50*	13.00*	19.75*	26.25*
					(7)33.00*	(8)39.50*				

PUNCHING SHEAR REINFORCING 1



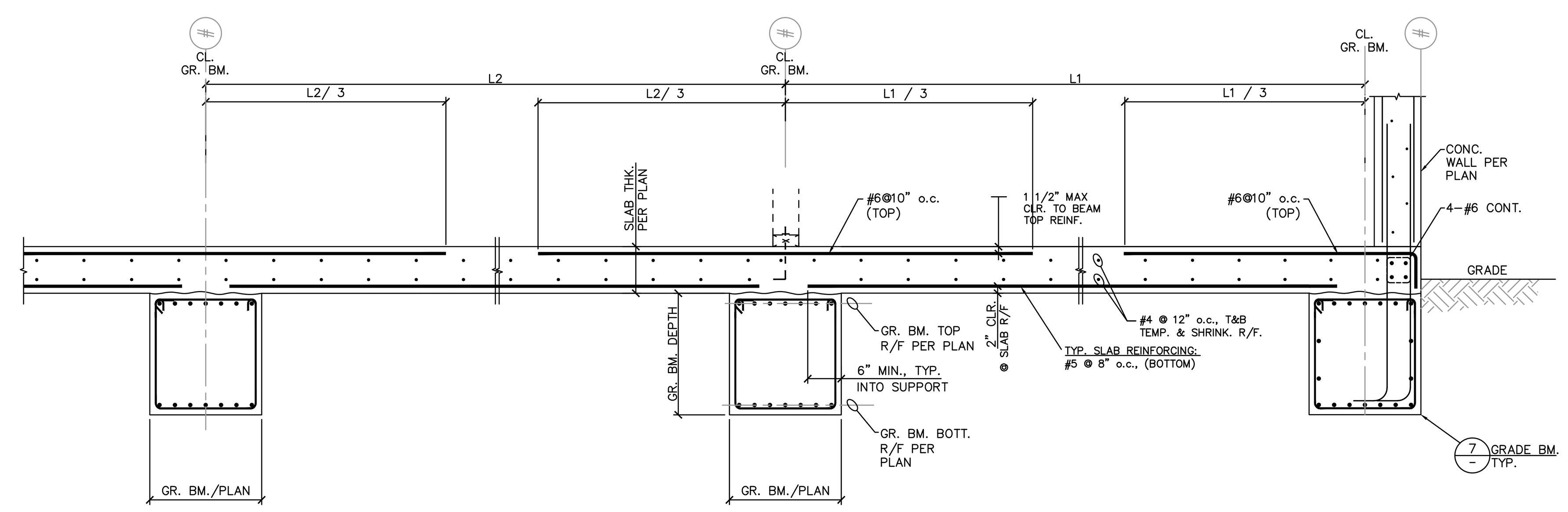
PILE CAP - TYP. SECTION 8



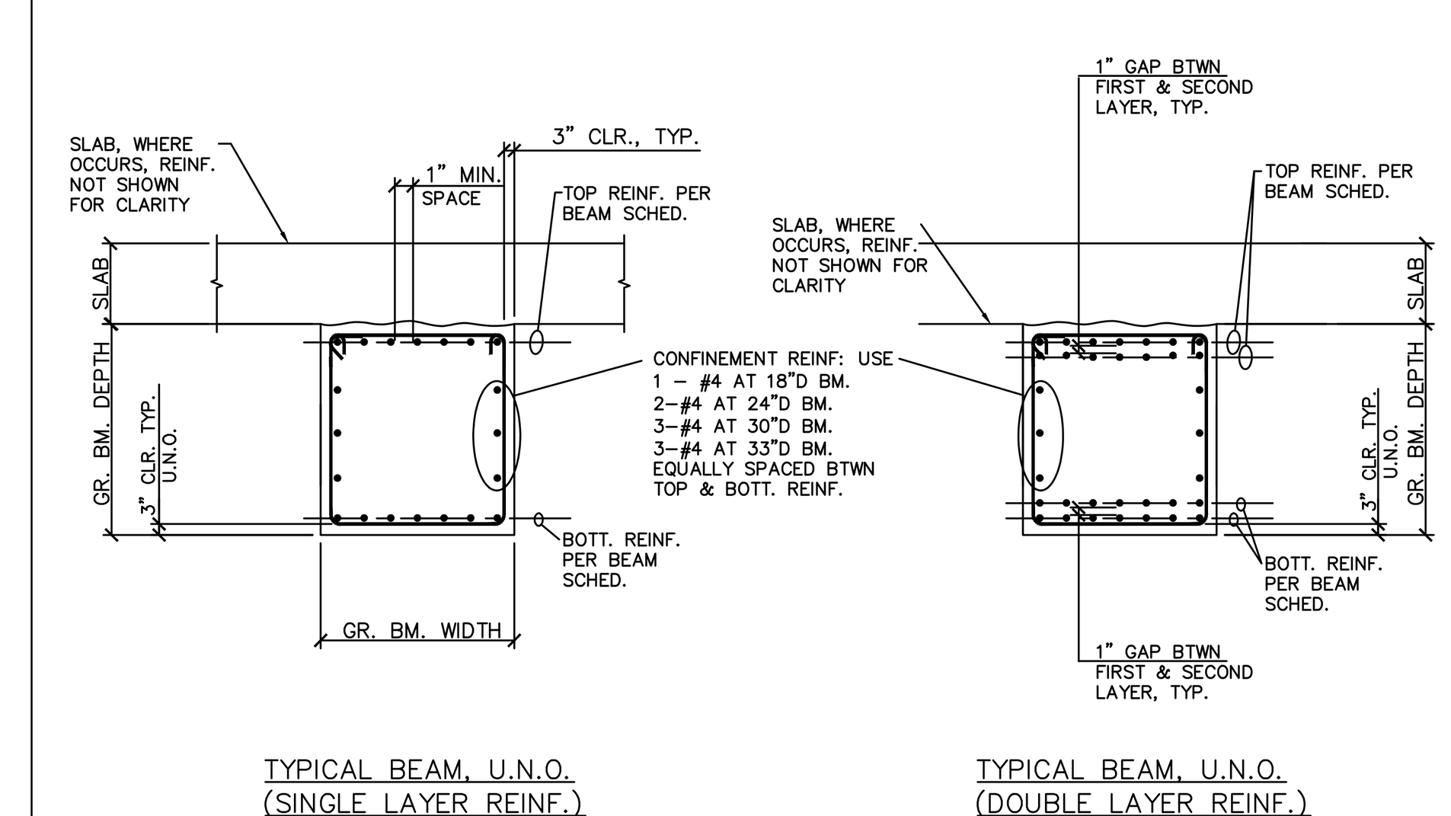
PILE CAP - PLAN VIEW 2

13

10



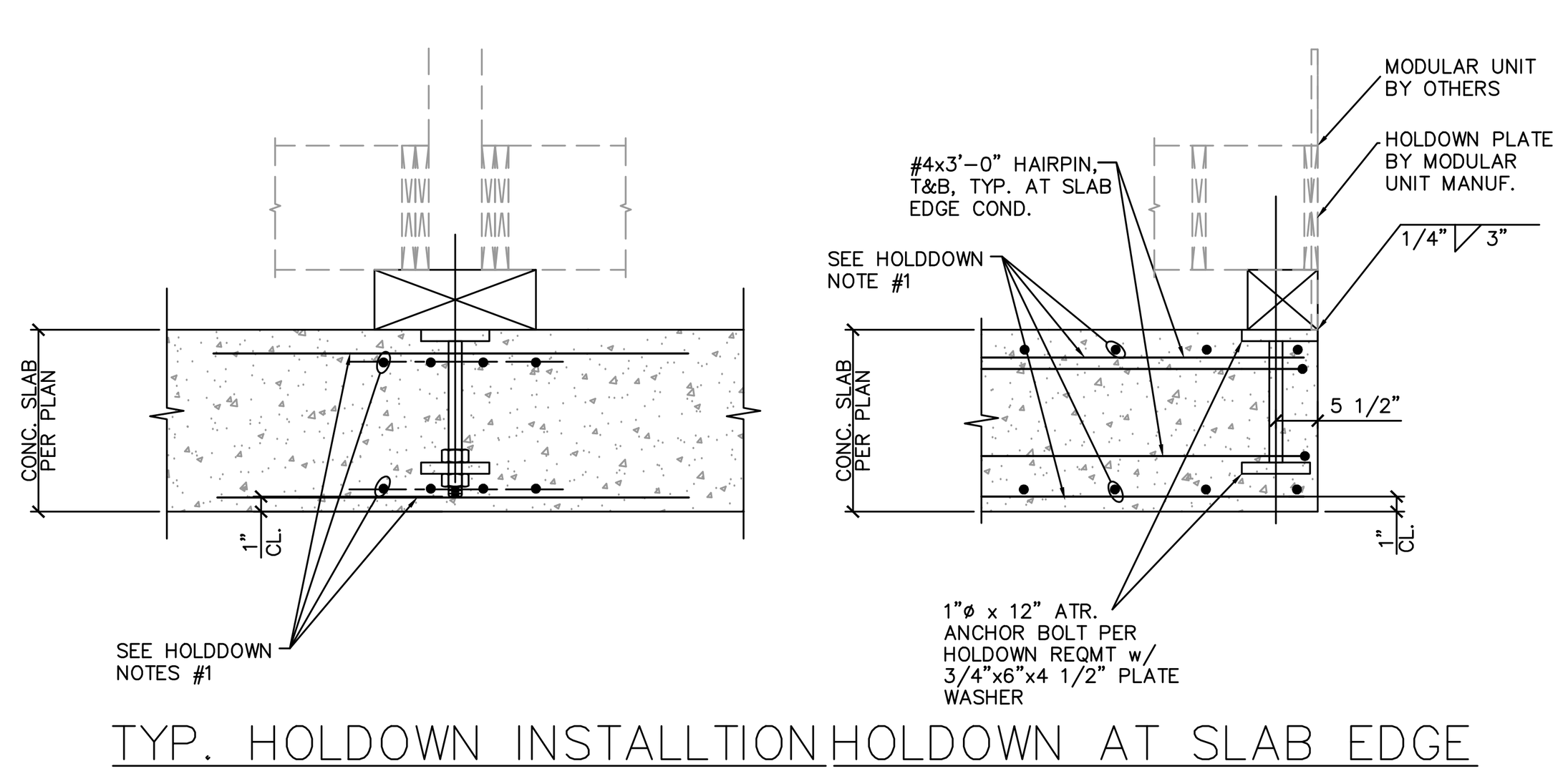
SECTION - TYPICAL SLAB REINFORCING 5



SECTION - TYP. GRADE BEAM REINFORCING 3

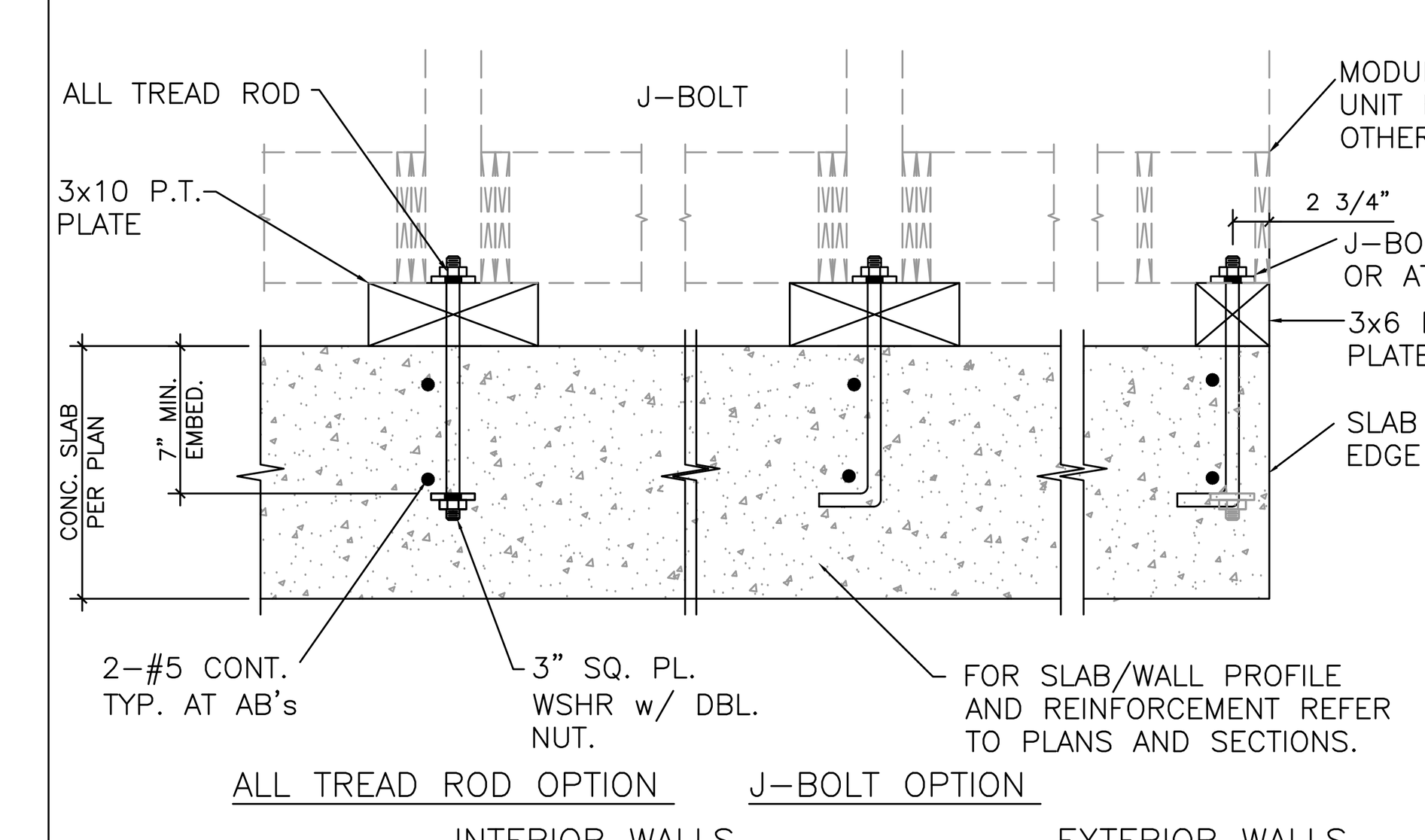
14

11



HOLDOWN NOTES: TYPICAL FOR ALL HOLDOWN LOCATIONS SHOWN ON DRAWING S-1.0
1. FOR ALL HOLDOWNS PROVIDE ADDITIONAL 4-#5x4'-0" @6"oc E.W. TOP+BOT ADDITIONAL REINFORCEMENT, PLACE CENTER WITH HOLDOWN, UNLESS SHOWN OTHERWISE PER PLAN.
2. SEE PLANS FOR REMAINDER TO SLAB REINFORCING.

HOLDOWN ANCHOR DETAILS 6



ANCHOR BOLT SCHEDULE		
DETAIL	ANCHOR BOLT SIZE	SPACING
16A	5/8"	24" o.c.
16B	5/8"	12" o.c.
16C	3/4"	8" o.c. STAGGERED

TYP. ANCH. BOLT AT BEARING / SHEAR WALLS 4

ARRIS STUDIO ARCHITECTS
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1306 JOHNSON AVENUE
SAN LUIS OBISPO, CA 93401
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805.547.2240
ARRIS-STUDIO.COM

CONSULTANT
HCP ENGINEERING

No. C50389
Exp. 6-30-2017
8/25/17

CLIENT
SRI KRISHNA ENTERPRISE
300 GATEWAY BOULEVARD
SOUTH SAN FRANCISCO, CA 94080

PROJECT
Home2 Suites by Hilton
550 GATEWAY BOULEVARD
SOUTH SAN FRANCISCO, CA

CONCRETE DETAILS

#	REVISION	DATE

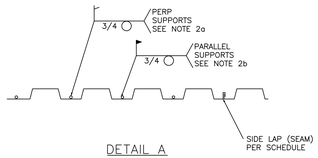
PROJECT 2017-06
DATE March 9, 2017
SHEET SD1.2

METAL ROOF DECK NOTES:

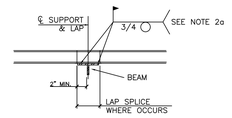
1. METAL ROOF DECK ASSMEBLY TO BE ASC STEEL DECK – TYPE B–36x20 GA. (ICC–ESR–1414) w/ STRUCTURAL CONCRETE FILL (SEE DETAIL 'A').
2. STEEL DECK INSTITUTE SPECIFICATIONS AND RECOMMENDATIONS SHALL BE FOLLOWED.
3. METAL DECK SHALL BE PRIME–PAINTED FORMED A.S.T.M. 611 GRADE C MINIMUM WITH A MINIMUM YIELD STRENGTH OF 38,000 PSI.
4. METAL DECK SHALL BE CONTINUOUS OVER A MINIMUM OF 3 SPANS AND INSTALLED PERPENDICULAR TO STEEL SUPPORT MEMBERS.
5. ALL DECKING SHALL BE PAINTED.
6. COMPLY WITH A.W.S. REQUIREMENT AND PROCEEDURES FOR WELDING SHEET STEEL FOR STRUCTURES.
7. METAL DECK MINIMUM PROPERTIES SHALL BE AS FOLLOWS:

THICKNESS:	20 GA		
WEIGHT (PSF):	1.99	I (IN^4/FT):	0.240
S+ (IN^3):	0.253		
S– (IN^3):	0.623		

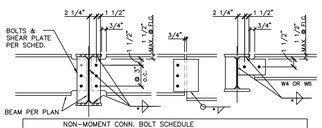
SIDE LAP SPACING: 3/4"ø PUDDLE WELDS @ 12" o.c. OR 3/8"x1" FILLET WELDS @ 6" o.c.
DIAPHRAGM CHORD OR STRUTS: 3/4"ø PUDDLE WELDS @ 12" o.c. OR 3/8"x1" FILLET WELDS @ 6" o.c.



DETAIL A



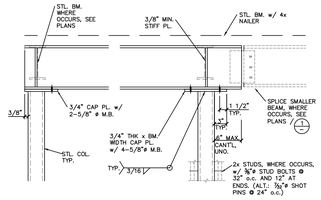
DETAIL B



NON–MOMENT CONN. BOLT SCHEDULE

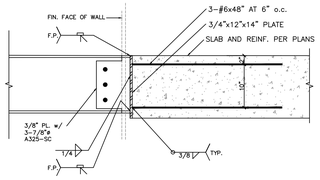
BEAM SIZE	SHEAR PLATE SIZE	BOLTS	BOLT TYPE	* WELD SIZE	REMARKS
W4 OR W6	1/4"	2 – 5/8" ø	A325X	3/16"	
W8 x	3/8"	2 – 7/8" ø	A325X	3/16"	
W10 x	3/8"	2 – 7/8" ø	A325X	3/16"	
W12 x	3/8"	3 – 7/8" ø	A325X	1/4"	
W14 x	3/8"	3 – 7/8" ø	A325X	1/4"	
W16 x	3/8"	4 – 7/8" ø	A325X	1/4"	
W18 x	3/8"	5 – 7/8" ø	A325X	1/4"	
W21 x	7/16"	6 – 7/8" ø	A325X	1/4"	
W24 x	7/16"	7 – 7/8" ø	A325X	3/8"	
W30 x	5/8"	9 – 1" ø	A325X	3/8"	
W36 x	5/8"	11 – 1" ø	A325X	3/8"	

STEEL BEAM CONN. 1

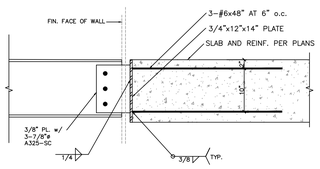


END COL. COND TYP. COL. COND

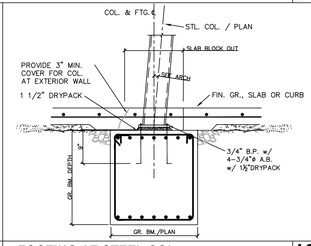
BEAM TO COLUMN CONN. 2



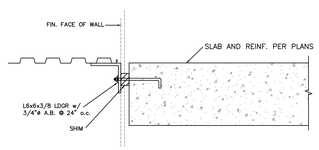
STEEL BEAM CONN. 3



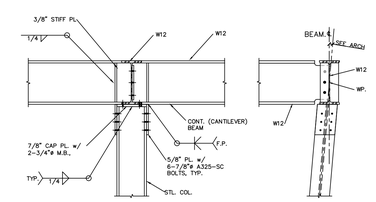
FOOTING AT STEEL COL. 10



STEEL BEAM CONN. 4



MOMENT CONN. (CONT. BM) 6



HCP ENGINEERING
1307 West Sixth Street, Suite 208
Corona, CA 92882–6805
Ph: (951) 738–0840
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REVISIONS:

STEEL FRAMING DETAILS

HOME2 SUITES BY HILTON
550 GATEWAY BLVD.
SOUTH SAN FRANCISCO, CA 94080

REGISTERED PROFESSIONAL ENGINEER
VITESH C. PATEL
No. C50389
Exp. 6–30–2019
CIVIL
STATE OF CALIFORNIA
8/25/17

PLOT DATE: 8/25/2017
SCALE:
DRAWN BY:
JOB NO. 2017–06
SHEET NO. SD-2