

BUILDING CODE

DESIGN LIVE LOADS
CODE REFERENCE2009 INTERNATIONAL BUILDING CODE

DEAD LOADS:	
3/4" GYP-CRETE	8 psf
INTERIOR PARTITIONS, ETC.....	15 psf
ROOFING AND INSULATION.....	4 psf
DECKING.....	2 psf
JOISTS.....	5 psf
CEILING.....	2 psf
HVAC.....	2 psf

GRAVITY LIVE LOADS:	
ROOF	25 psf
GUEST ROOMS	40 psf
PUBLIC CORRIDORS	100 psf
STAIRS & LANDINGS	100 psf
BALCONIES.....	100 psf

HANDRAILS & GUARDRAILS:
HANDRAIL LOADS-250 lbs concentrated load at any point in any direction OR 50 pif applied in any direction (WHICHEVER LOADING CONDITION PRODUCES THE HIGHEST STRESS WILL BE USED)

GUARDRAIL LOADS-250 lbs concentrated load at any point in any direction APPLIED AT THE TOP OF THE RAIL, 50 pif in any direction APPLIED HORIZONTALLY AT THE REQUIRED GUARDRAIL HEIGHT w/ A SIMULTANEOUS 100 pif load APPLIED VERTICALLY DOWNWARD, OR A 250 lbs concentrated load at any point in any direction APPLIED TO A 1 SQ. FT. AREA (WHICHEVER LOADING CONDITION PRODUCES THE HIGHEST STRESS WILL BE USED)

SEISMIC DESIGN DATA:
IMPORTANCE FACTOR, $I_e = 1.00$
SPECTRAL RESPONSE, $S_s = 0.148$, $S_1 = 0.0824$
SITE CLASS = D
SPECTRAL RESPONSE CO-EFFICIENTS, $S_{as} = 0.158$, $S_{a1} = 0.132$
SEISMIC DESIGN CATAGORY = B
SEISMIC FOR RESISTING SYSTEMS:
LIGHT FRAMED SHEAR WALLS w/ WOOD PANELS
SEISMIC BASE SHEAR = 61 KIPS
RESPONSE COEFFICIENT, $C_s = 0.024$
EQUIVALENT LATERAL FORCE PROCEDURE

WIND LOAD DESIGN DATA:	
WIND LOAD IMPORTANCE FACTOR (1)	1.00
BASIC WIND SPEED	90 MPH
WIND EXPOSURE CATEGORY	B
WIND REFERENCE PRESSURE	18 psf
(AT TOP OF BUILDING)	

GENERAL:

- SEE ARCHITECTURAL DRAWINGS FOR ANGLES, CLIPS, BARS, PLATES AND OTHER ITEMS ATTACHED TO STRUCTURAL MEMBERS.
- PROVIDE TEMPORARY BRACING AS REQUIRED TO MAINTAIN ALIGNMENT AND SECURITY OF STRUCTURES DURING CONSTRUCTION.
- DO NO CUTTING, DRILLING, OR MODIFYING OF STRUCTURAL MEMBERS WITHOUT THE APPROVAL OF THE ARCHITECT.
- THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS BEFORE STARTING WORK. THE ARCHITECT SHALL BE NOTIFIED OF ANY DISCREPANCY.
- THE DESIGN ADEQUACY AND SAFETY OF ERECTION BRACING, SHORING, ETC., IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- THE CONTRACTOR SHALL COORDINATE THE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS WITH THE STRUCTURAL DRAWINGS.
- CONTRACTOR TO VERIFY ALL WALL, COLUMN, AND SLAB LOCATIONS, THICKNESS, AND DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
- THE GENERAL CONTRACTOR SHALL COORDINATE THE PLACEMENT OF FOOTINGS, COLUMNS, SLAB, WALLS, SHAFTS, ETC., WITH ALL SUBCONTRACTORS INVOLVED.
- FOUNDATIONS ARE DESIGNED TO BEAR ON RAMMED AGGREGATE PIER FOUNDATIONS. FOOTINGS ARE DESIGNED FOR A SOILS BEARING PRESSURE OF **4,000 psf** AT THE BOTTOM OF THE FOOTINGS. ALL FOOTINGS SHALL BE SET BELOW THE FROST DEPTH.
- ALL FOUNDATION & SOILS WORK SHALL BE DONE IN ACCORDANCE WITH THE PROJECT SOILS REPORT PREPARED BY ARDMAN & ASSOCIATES, INC. DATED DECEMBER 20, 2013 AND THE GEOPIER SYSTEM FOUNDATION RECOMMENDATION MEMO DATED APRIL 18, 2014.
- VERIFY ALL OPENING SIZES AND LOCATIONS ON THE STRUCTURAL DRAWINGS w/ THE MECHANICAL DRAWINGS.

STRUCTURAL STEEL:

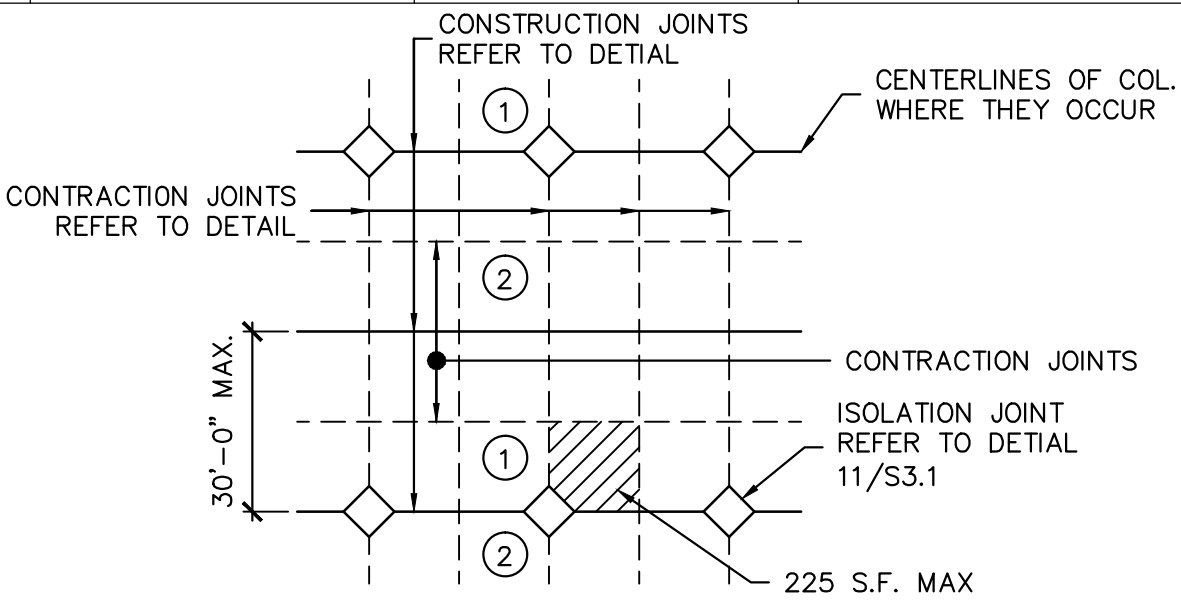
- STRUCTURAL STEEL SHALL CONFORM TO ASTM SPECIFICATION A-992 ($F_y=50$ ksi).
- STRUCTURAL STEEL SHALL BE DETAILED IN ACCORDANCE WITH STANDARD PRACTICES OF A.I.S.C.
- STRUCTURAL STEEL TO BE DESIGNED, FABRICATED, ERECTED, ETC., AS PER AISC MANUAL OF STEEL CONSTRUCTION, LATEST EDITION.
- SUBMIT SHOP DRAWINGS OF STRUCTURAL STEEL PRIOR TO FABRICATION OF SAME.
- VERIFY w/ ARCHITECT, MECHANICAL, AND ELECTRICAL DRAWINGS FOR STAIR DETAILS, RAILINGS, ANGLES, ETC., NOT SHOWN ON STRUCTURAL DRAWINGS.
- SEE ARCHITECTURAL DRAWINGS FOR MATERIAL AND TREADS.
- PROVIDE ALL NECESSARY CLIP ANGLES, BOLTS, HANDRAILS, ETC., TO COMPLETE STAIR PORTION OF PROJECT.
- STEEL TUBES SHALL BE ASTM A500 GRADE B.

WOOD NOTES:

- STRESS GRADE SAWED LUMBER SHALL CONFORM TO SPECIFICATIONS FOR KILN DRIED NO. 2 SOUTHERN PINE WITH ALLOWABLE BENDING STRESS OF 1500 psi. MODULUS OF ELASTICITY= 1600 ksi (MINIMUM).
- USE JOIST HANGERS AND FRAMING ANCHORS, 18 ga. MINIMUM, GALVANIZED, SIZED FOR FULL LOAD CAPACITY OF SUPPORTED MEMBERS.
- PROVIDE DOUBLE STUDS (MINIMUM) EACH SIDE OF ALL WALL OPENINGS.
- PROVIDE SOLID BLOCKING IN WOOD FLOOR CONSTRUCTION UNDER POSTS, COLUMNS, AND MULTIPLE STUDS.
- PROVIDE BRIDGING IN FLOOR AND ROOF CONSTRUCTION PER APPLICABLE CODES. MINIMUM= 1 ROW OF BRIDGING AT MID SPAN.
- PROVIDE CONTINUOUS HORIZONTAL BLOCKING IN ALL STUD WALLS PER APPLICABLE CODES. MINIMUM= 1 ROW OF BLOCKING AT MID HEIGHT.
- CONSTRUCT WOOD CONNECTIONS PER RECOMMENDATIONS OF THE NATIONAL FOREST PRODUCTS ASSOCIATION.
- USE THE LATEST EDITION OF THE IBC FOR MINIMUM ACCEPTABLE NAILING (FASTENER SCHEDULE).
- WHERE PLYWOOD FILLERS ARE USED WITH LINTELS OR BEAMS, THEY SHALL BE CONTINUOUS FOR THE FULL LENGTH OF THE BEAM OR LINTEL, AND THEY SHALL BE GLUED AND NAILED TO 2x's WITH 2 ROWS OF 10d NAILS AT 12" O.C. MINIMUM.
- MICROLAM AND PARALLAM LUMBER SHALL CONFORM TO MACMILLAN SPECIFICATIONS, OR EQUAL.
- FLOOR AND ROOF TRUSSES SHALL BE DESIGNED BY A REGISTERED ENGINEER FOR THE MAXIMUM LOAD APPLICABLE PER LOCAL AND NATIONAL CODES. L/480 SHALL BE THE MAXIMUM TOTAL LOAD DEFLECTION AT MID-SPAN FOR ROOF AND FLOOR JOISTS. SUBMIT CALCULATIONS AND SHOP DRAWINGS TO THE ARCHITECT FOR APPROVAL PRIOR TO FABRICATING.
- VERSALAM BEAMS SHALL CONFORM TO THE FOLLOWING SPECIFICATIONS:
 $F_b=2,900$ PSI MINIMUM BENDING STRESS; $E=2,000,000$ PSI; $F_v=290$ PSI;
 F_c , PARALLEL = 3,000 PSI
- FLOOR JOISTS TO BE EQUAL TO 11 $\frac{1}{8}$ " BCI 4500S-1.8 SP FLOOR JOISTS AS MANUFACTURED BY BOISE CASCADE OR EQUAL.
- ALL EXTERIOR WOOD TO BE PRESSURE TREATED LUMBER, TYP.

CONCRETE & MASONRY:

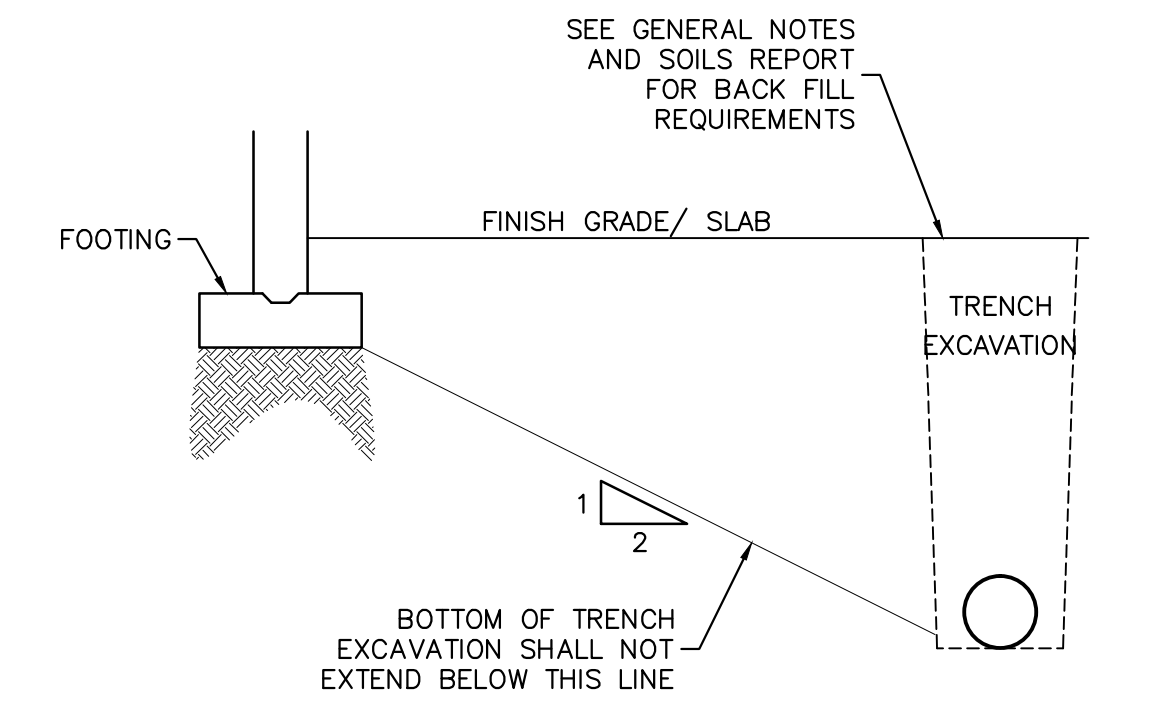
- CONCRETE STRENGTH TO BE A MINIMUM OF 4000 PSI AT 28 DAYS (SEE SPEC) (SEE NOTE #3 CONCERNING TESTING OF CONCRETE)
- CONCRETE EXPOSED TO WEATHER TO BE AIR - ENTRAINED (6% MAX. - 3% MIN.)
- CONCRETE AND REINFORCING STEEL TO BE AS PER LATEST ACI 318 AND ACI 301. TESTING OF CONCRETE TO BE IN ACCORDANCE w/ ACI 301 BY AN INDEPENDENT TESTING AGENCY AT CONTRACTORS' EXPENSE.
- PROVIDE SHOP DRAWINGS DETAILING REINFORCING STEEL PRIOR TO FABRICATION OF SAME. (INCLUDE IN SUBMITTALS ELEVATIONS OF ALL REINFORCED WALLS AND COLUMNS UNLESS OTHERWISE NOTED). DETAILING SHALL BE ACCOMPLISHED BY AN EXPERIENCED DETAILER AND ACCORDING TO ACI 318.
- ALL SLAB ON GRADE TO BE SUPPORTED ON RAMMED AGGREGATE PIERS. ALL SLAB ON GRADE TO BE 4" CONCRETE w/ 6x6W2.1/W2.1 ON 4" GRANULAR FILL OR APPROVED FILL RECOMMENDED BY GEOTECHNICAL ENGINEER OF RECORD, UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL DRAWINGS FOR ANY DEPRESSED AREAS, VAPOR BARRIERS, ETC..
- THE MINIMUM CONCRETE COVER SHALL BE IN ACCORDANCE WITH A.C.I. 318.
- ALL HOLLOW CONCRETE MASONRY UNITS TO MEET A.S.T.M. SPECIFICATIONS C90, GRADE N, TYPE 1, WITH MINIMUM ULTIMATE COMPRESSIVE PRISM STRENGTH ($f'm$) OF 1,500 PSI.
- ALL MORTAR SHALL MEET A.S.T.M. SPECIFICATIONS FOR TYPE "S" MORTAR EXCEPT AS SHOWN OTHERWISE WITH A MINIMUM COMPRESSIVE STRENGTH OF 1,800 P.S.I. ADDITIVES CONTAINING CALCIUM CHLORIDE SHALL NOT BE USED.
- HORIZONTAL JOINT REINFORCING SHALL BE HEAVY AT 16" C.C. EXCEPT AS SHOWN OTHERWISE.
- ALL MASONRY CORNERS SHALL HAVE 3 VOIDS REINFORCED w/ (1) #5 EACH VOID AND GROUTED.
- ALL CELLS WHERE REINFORCING IS SPECIFIED SHALL BE FILLED w/ CONCRETE GROUT.
- REINFORCING SHALL BE A-615 GRADE 60 EXCEPT #3 BARS SHALL BE GRADE 40 IN ACCORDANCE WITH LATEST A.S.T.M. SPECIFICATIONS
- REINFORCING IN ALL CONCRETE FOOTING AND WALLS SHALL BE CONTINUOUS AROUND CORNERS.
- LAP ALL STEEL 36 BAR DIAMETER OR 18" MINIMUM AT SPLICES AND CORNERS.
- GROUT SHALL CONFORM TO A.S.T.M. 476, 3000 PSI STRENGTH.
- ALL BLOCK CELLS BELOW GRADE SHALL BE FILLED SOLID WITH CONCRETE OR GROUT.
- CONSTRUCTION OF LOAD-BEARING CONCRETE MASONRY SHALL CONFORM TO SPECIFICATIONS BY THE NATIONAL CONCRETE MASONRY ASSOCIATION AND A.C.I. 531
- PROVIDE 2 #5 BARS EXTRA E.S. OF ALL OPENINGS IN CONCRETE SLABS U.N.O.
- PROVIDE 8"x16" U-BLOCK LINTEL w/ 2 #6 BOT. OVER ALL OPENINGS IN MASONRY WALLS U.N.O. BEAR LINTELS 16" MIN E.E.
- PROVIDE 1 #5 EXTRA VERTICAL BAR EACH SIDE OF OPENINGS IN MASONRY WALLS U.N.O.



NOTES:

- SLAB SHALL BE POURED IN STRIP PATTERN.
①=FIRST POUR ②=SECOND POUR
- STRIPS TO BE DIVIDED BY CONSTRUCTION JOINTS AT THE CENTERLINE OF COLS. AND SUBDIVIDED AS REQ'D WITH CONTRACTION JOINTS SPACED AT INTERVALS NOT EXCEEDING 15' IN EITHER DIRECTION.
- IN AREAS WHERE COLUMNS DO NOT OCCUR PROVIDE CONSTRUCTION AND/OR CONTRACTION JOINTS AS SHOWN ABOVE.
- CONTRACTOR CAN SUBMIT A PLAN WITH SAW CUT JOINTS IN BOTH DIRECTIONS FOR APPROVAL. MAX AREA BETWEEN JOINTS SHALL BE 225 S.F.

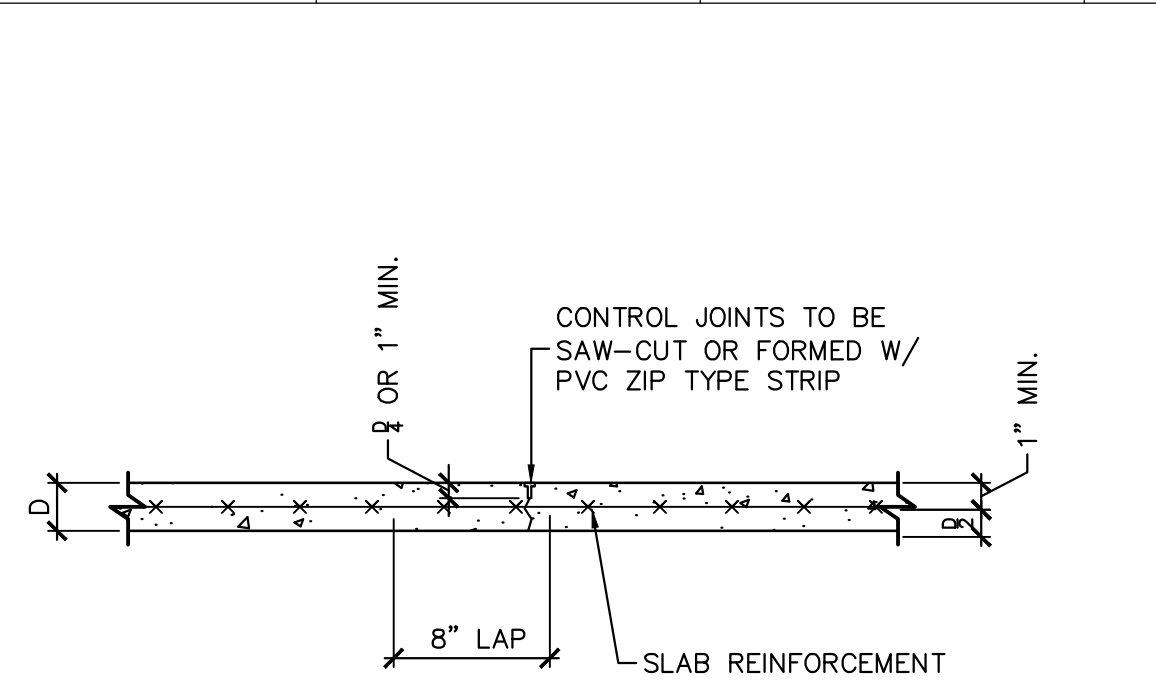
① SLAB ON GRADE POUR DETAIL
SCALE: N.T.S. DET067



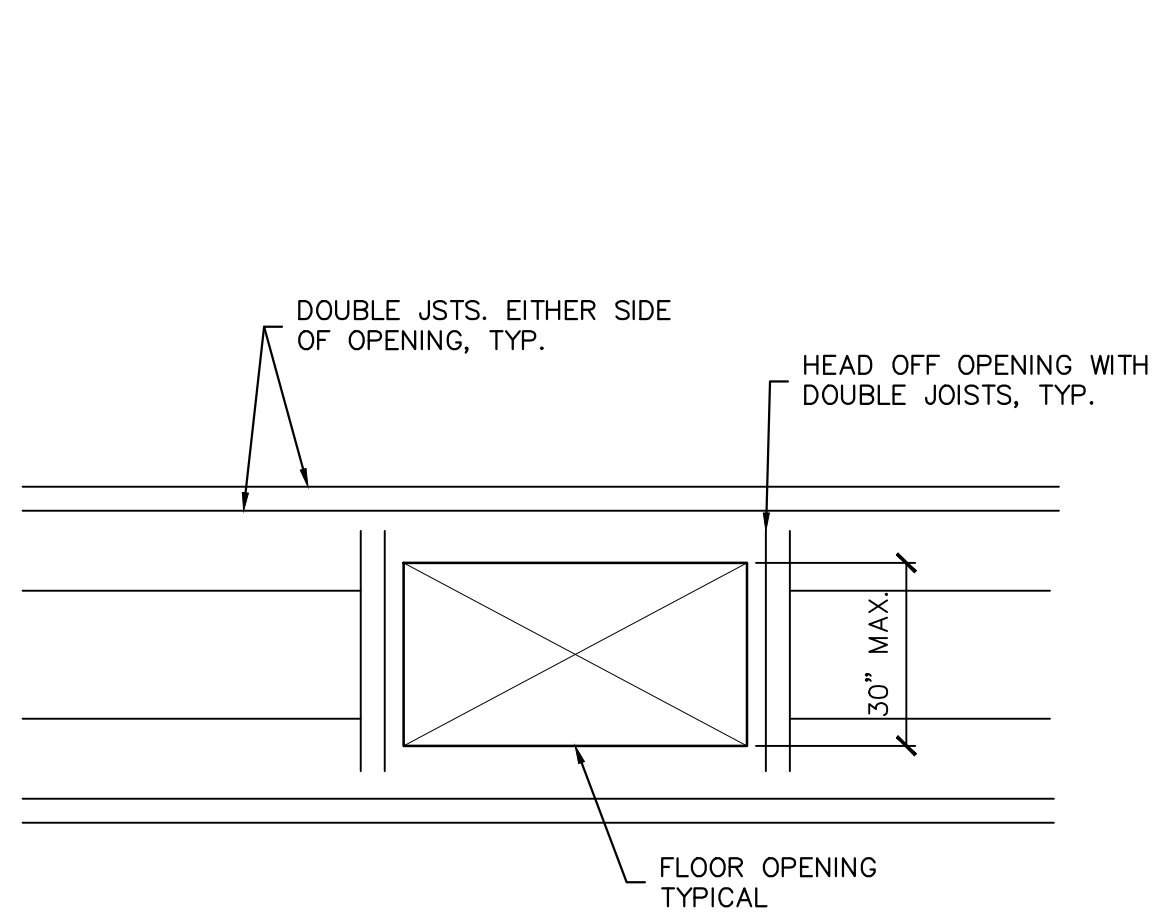
NOTES:

- THE CONTRACTOR SHALL COORDINATE ALL EXCAVATION OPERATIONS WITH BUILDING FOUNDATION REQUIREMENTS.
- SLOPE MAY BE ALTERED TO 1:1 IF CONDITIONS ALLOW AND APPROVED BY ARCHITECT/ENGINEER.

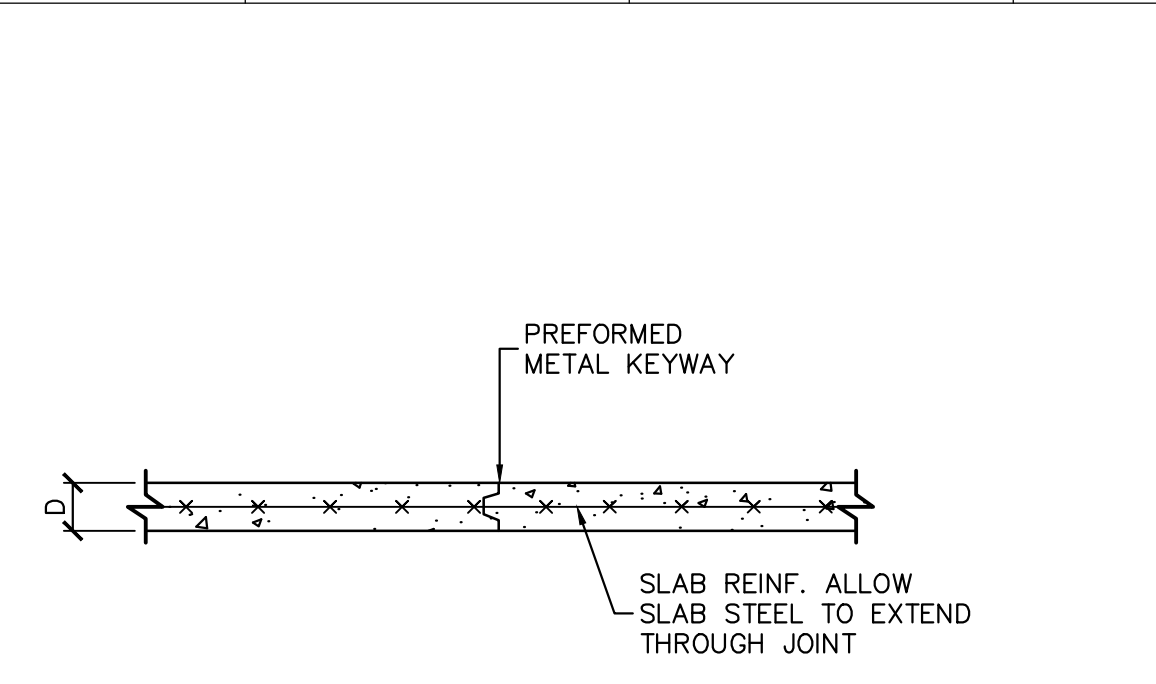
④ EXCAVATION PARALLEL TO FOOTING
SCALE: 1/2"=1'-0" DET070



② CONTROL JOINT IN SLAB ON GRADE
SCALE: 3/4"=1'-0" DET068



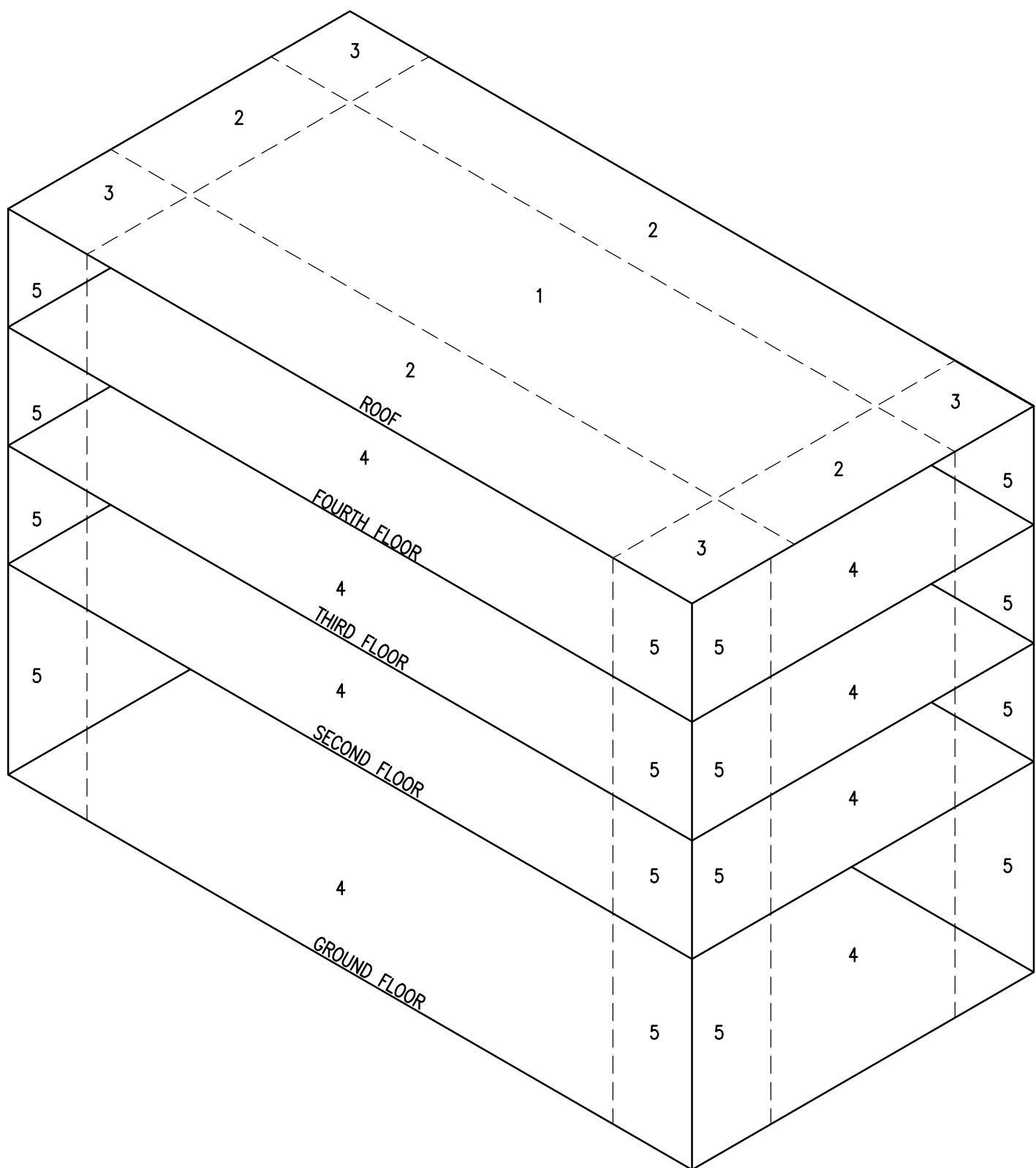
⑤ TYPICAL FLOOR PENETRATION DETAIL
SCALE: 1/2"=1'-0" DET061



③ CONSTRUCTION JOINT IN SLAB ON GRADE
SCALE: 3/4"=1'-0" DET069

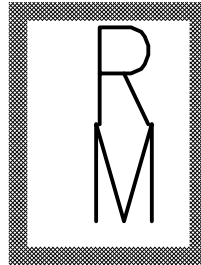
	C1	C2	C3	C4	C5	C6	C7
ROOF EL=138'-1" (JST BRG)				HSS 5x5x $\frac{5}{8}$ $F_y = 46$ ksi	HSS 5x5x $\frac{5}{8}$ $F_y = 46$ ksi		
4th FLR. EL=129'-11"				HSS 5x5x $\frac{5}{8}$ $F_y = 46$ ksi	HSS 5x5x $\frac{5}{8}$ $F_y = 46$ ksi		
3rd FLR. EL=120'-8"				HSS 5x5x $\frac{5}{8}$ $F_y = 46$ ksi	HSS 5x5x $\frac{5}{8}$ $F_y = 46$ ksi		
2nd FLR. EL=111'-6"	HSS 6x6x $\frac{5}{8}$ $F_y = 46$ ksi 12"x1"x12" PL w/ (4) 1"ø x 1'-8" A.B.	HSS 8x8x $\frac{5}{8}$ $F_y = 46$ ksi 14"x1"x14" PL w/ (4) 1"ø x 1'-6" A.B.	HSS 8x8x $\frac{5}{8}$ $F_y = 46$ ksi 14"x1"x12" PL w/ (4) 1"ø x 1'-6" A.B.	HSS 5x5x $\frac{5}{8}$ $F_y = 46$ ksi 12"x3/4"x12" PL w/ (4) 3/8" x 1'-6" A.B.	HSS 8x8x $\frac{5}{8}$ $F_y = 46$ ksi 14"x1"x14" PL w/ (4) 1"ø x 1'-6" A.B.	HSS 6x6x $\frac{5}{8}$ $F_y = 46$ ksi 12"x8"x12" PL w/ (4) 3/8" x 1'-0" A.B.	
GRND. FLR. EL=100.00'							

NOTE:
ALL STEEL COLUMNS TO HAVE $\frac{5}{8}$ " CAP PLATE MINIMUM



COMPONANTS AND CLADDING WIND PRESSURES			
	ZONE	MAX P (PSF)	MIN P (PSF)
ROOF	1	10.27	-19.51
	2	17.63	-24.31
	3	17.63	-24.31
WALL STUDS	4	17.88	-19.07
	5	17.88	-21.37
WINDOWS	4	18.15	-19.34
	5		

NOTES:
ZONE 4 WIDTH = 5'-0"



Richard Molenaar

ARCHITECT



PROJECT NAME:

HOME2
SUITES BY HILTON

W Mane Street West Monroe, Louisiana

SHEET TITLE:

**GENERAL NOTES, DETAILS,
& COL. SCHEDULE**

SCALE: AS NOTED

PROJECT NO.

LATEST REVISION:

2014052

DRAWN BY:

CHECKED BY:

SHEET NUMBER:

GP

MD

DATE:

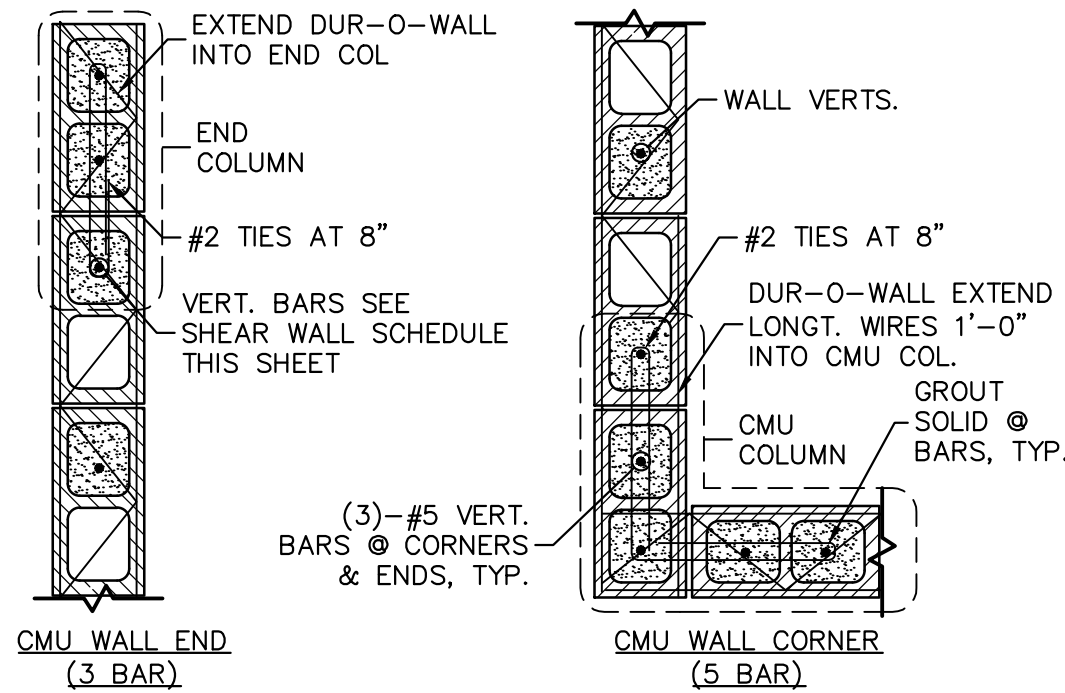
5-19-14

S-101

CMU WALL REINFORCING SCHEDULE

LEVEL	WALL ENDS	WALL CORNERS	REINFORCEMENT
4TH WALL	8"x24" CMU COL. w/ 3 #6 VERTS #2 AT 8" TIES	8"x24" CMU COL. w/ 5 #6 VERTS #2 AT 8" TIES	8" CONC. BLOCK WALL #5 AT 32" VERTS. FILL ALL CELLS
3RD WALL	8"x24" CMU COL. w/ 3 #6 VERTS #2 AT 8" TIES	8"x24" CMU COL. w/ 5 #6 VERTS #2 AT 8" TIES	8" CONC. BLOCK WALL #5 AT 32" VERTS. FILL ALL CELLS
2ND WALL	8"x24" CMU COL. w/ 3 #6 VERTS #2 AT 8" TIES	8"x24" CMU COL. w/ 5 #6 VERTS #2 AT 8" TIES	8" CONC. BLOCK WALL #5 AT 24" VERTS. FILL ALL CELLS
GRND WALL	8"x24" CMU COL. w/ 3 #6 VERTS #2 AT 8" TIES	8"x24" CMU COL. w/ 5 #6 VERTS #2 AT 8" TIES	8" CONC. BLOCK WALL #5 AT 24" VERTS. FILL ALL CELLS

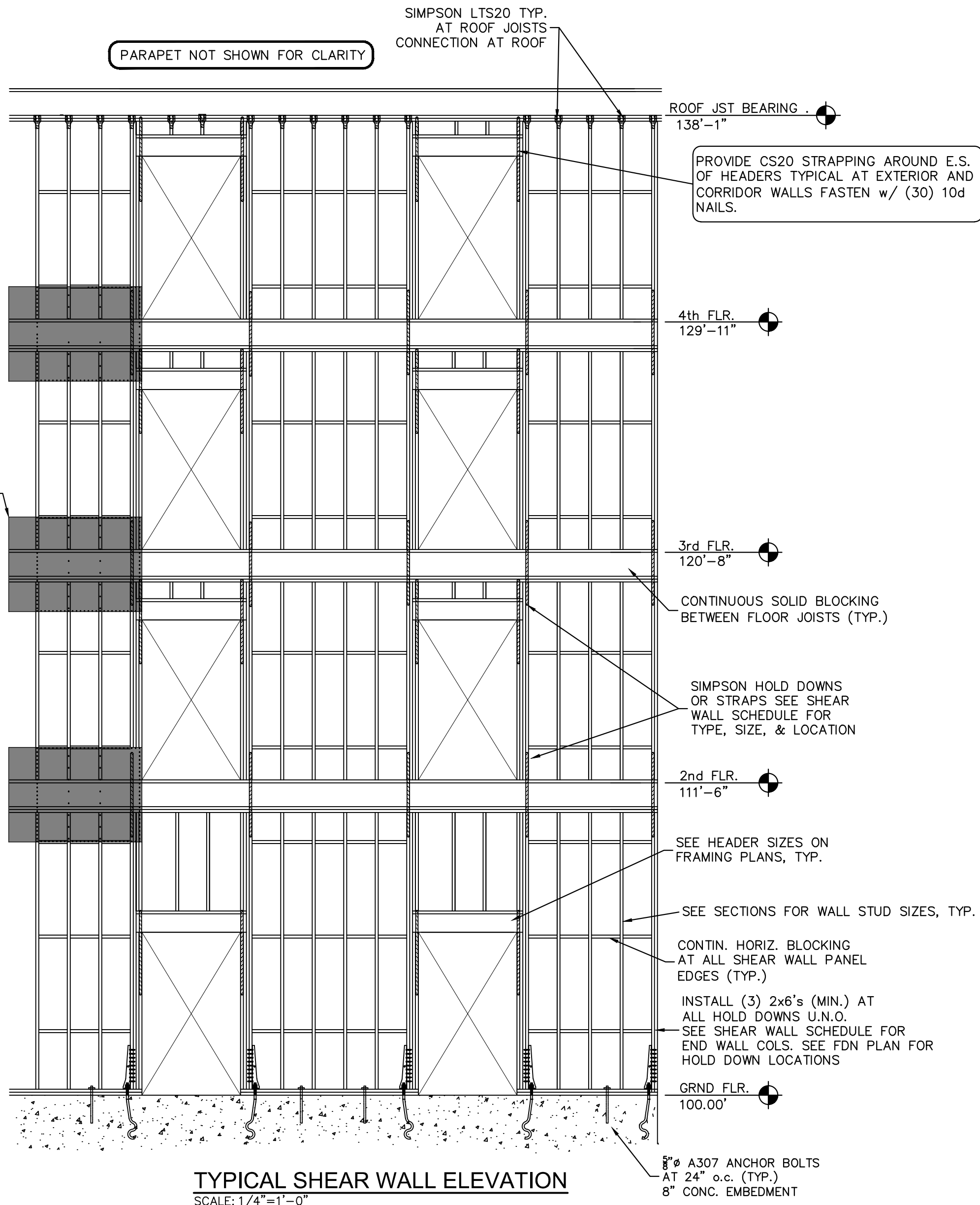
NOTE:
SEE SHEAR WALL END AND CORNER DETAILS ON 1/S-102
HATCH INDICATES CMU WALLS ON PLANS



- NOTES:
1. FILL CELLS SOLID WITH VERTICAL REINFORCING, TYP.
2. FILL ALL BLOCK CELLS SOLID AT CORNERS AND END WALL.
3. LAY BLOCK IN RUNNING BOND & WITH INTERLOCKING CORNERS & T-INTERSECTIONS.

1 CMU END WALL & CORNER WALL DETAILS
SCALE: 3/4"=1'-0"

DET118



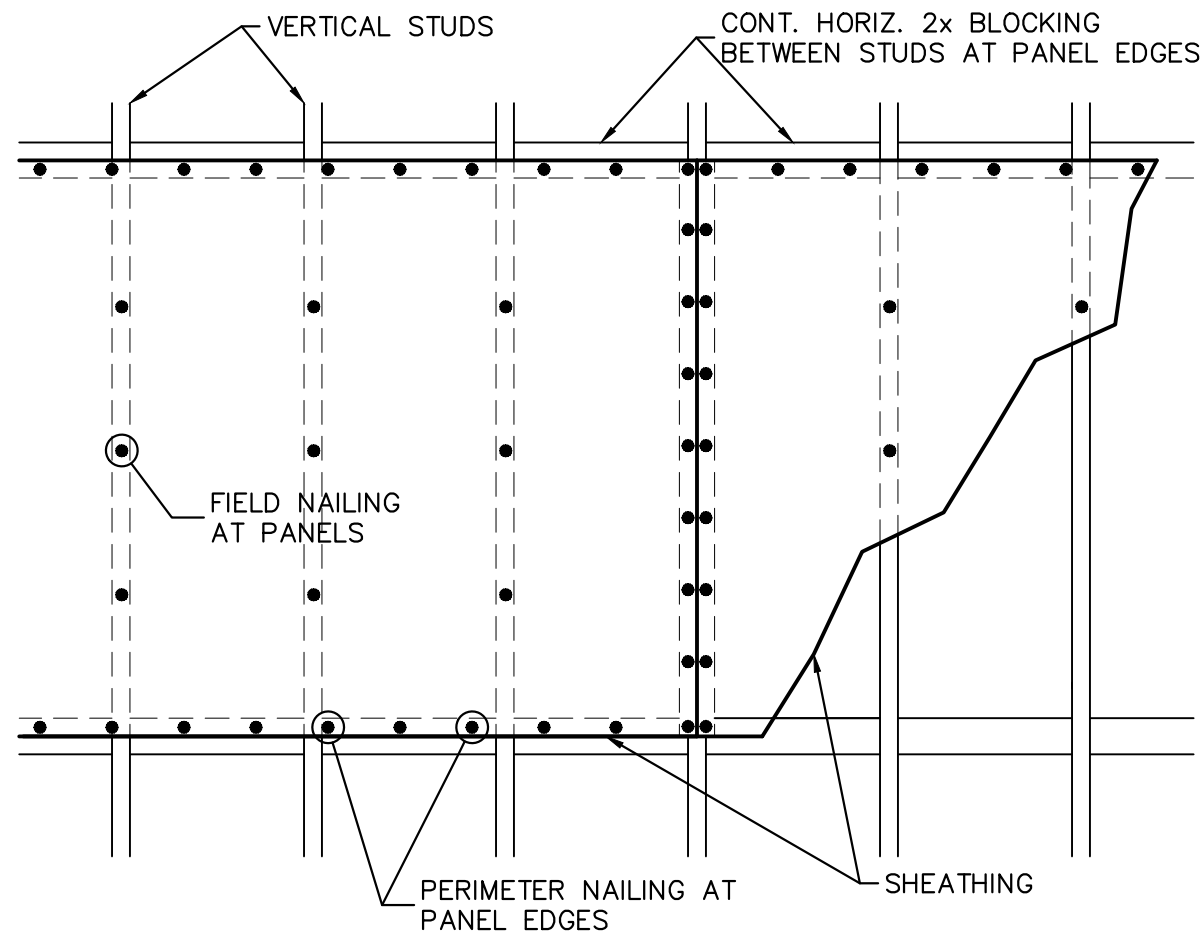
- NOTES:
1. SEE TYPICAL PLYWOOD SHEATHING FASTENING REQUIREMENTS PER SHEARWALL SCHEDULES THIS SHEET
2. SEE FOUNDATION PLAN FOR LOCATIONS OF HOLD DOWNS. SEE NOTES & DETAILS ON THIS SHEET
3. EXTERIOR PLYWOOD TO LAP AT FLOOR LEVELS (TYP.)

SHEAR WALL SCHEDULE

LEVEL	SHEAR WALLS	HOLD DOWN TYP. AT EACH END OF SHEAR WALL	BOTTOM PLATE & CONNECTION	END WALL COL. AT HOLD DOWN
4TH WALL	7/16" STRUCTURAL I OSB ONE SIDE OF WALL w/ BLOCKING ATTACH w/ 8d NAILS w/ 18" MIN. PENETRATION AT 6" SPACING AT PANEL EDGES AND 12" SPACING INFILL	SIMPSON CS18 STRAP FASTEN w/ (22) 8d x 3" NAILS TO STUDS 12" END LENGTH REQ'D	SINGLE BOT. PLATE w/ 16d NAILS AT 6" o.c. STAGGARD	(2) 2x6 STUD COL.
3RD WALL	7/16" STRUCTURAL I OSB ONE SIDE OF WALL w/ BLOCKING ATTACH w/ 8d NAILS w/ 18" MIN. PENETRATION AT 6" SPACING AT PANEL EDGES AND 12" SPACING INFILL	SIMPSON CS18 STRAP FASTEN w/ (22) 8d x 3" NAILS TO STUDS 12" END LENGTH REQ'D	SINGLE BOT. PLATE w/ 16d NAILS AT 6" o.c. STAGGARD	(2) 2x6 STUD COL.
2ND WALL	7/16" STRUCTURAL I OSB ONE SIDE OF WALL w/ BLOCKING ATTACH w/ 8d NAILS w/ 18" MIN. PENETRATION AT 4" SPACING AT PANEL EDGES AND 12" SPACING INFILL	SIMPSON CMST14 STRAP FASTEN w/ (58) 16d x 3" NAILS TO STUDS 30" END LENGTH REQ'D	SINGLE BOT. PLATE w/ 16d NAILS AT 6" o.c. STAGGARD	(2) 2x6 STUD COL.
*GRND WALL	7/16" STRUCTURAL I OSB ONE SIDE OF WALL w/ BLOCKING ATTACH w/ 8d NAILS w/ 18" MIN. PENETRATION AT 3" SPACING AT PANEL EDGES AND 12" SPACING INFILL	SIMPSON HDU14-SDS2.5 FASTEN w/ (30) SDS SCREWS 1/2"x2 1/2" TO STUDS 1" SB w/ 18" MIN. EMBED	SINGLE BOT. PLATE w/ 8" BOLTS AT 24" o.c. w/ WASHERS 8" EMBED. TYP.	(3) 2x6 STUD COL.

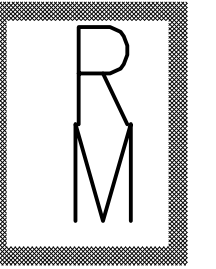
SHEAR WALL SCHEDULES
SCALE: N/A

- NOTES:
1. ALL EXTERIOR WALLS SHALL BE SHEATHED WITH 7/16" PLYWOOD OR OSB & NAILED PER SHEAR WALL SCHEDULE.
2. ALL SHEAR WALLS WILL BE BLOCKED AT PANEL EDGES TYP.
3. REFER TO THIS SHEET FOR TYPICAL SHEAR WALL ELEVATION.
* WHEN THERE IS NO GROUND FLOOR SHEAR WALL AND THE 2ND FLR. FLOOR SHEAR WALL BEARS ON A STEEL GIRDER.
FOLLOW THE SHEAR WALL SCHEDULE FOR THE WALLS ABOVE AND SEE DETAIL 7/S-302 FOR WALL TO GIRDER HOLD DOWN.



2 SHEARWALL SHEATHING FASTENING DETAIL
SCALE: 3/4"=1'-0"

DET001



Richard Molenaar
ARCHITECT



PROJECT NAME:

HOME2
SUITES BY HILTON

W Mane Street West Monroe, Louisiana

SHEET TITLE:

**SHEARWALL SCHEDULE
AND DETAILS**

SCALE: AS NOTED

PROJECT NO.

LATEST REVISION:

2014052

DRAWN BY:

CHECKED BY:

SHEET NUMBER:

GP

MD

DATE:

5-19-14

S-102

Verification and Inspection	FREQUENCY OF INSPECTION		REFERENCE STANDARD	IBC SECTION	CHECK IF NOT REQ'D
	Continuous during task listed	Periodically during task listed			
1. Materials verification of high-strength bolts, nuts and washers:					
a. Identification markings to conform to ASTM standards specified in the approved construction documents.	---	X	Applicable ASTM material specifications: AISC 360, Section A 3.3	---	☒
b. Manufacturer's certificate of compliance required.	---	X	---	---	☒
2. Inspection of high strength bolting:					
a. Snug-tight joints	---	X	AISC 360 Section M2.5	1704.3.3	☒
b. Pretensioned and slip-critical joints using turn-of-nut with matchmarking, twist-off bolt or direct tension indicator methods	X	X			☒
c. Pretensioned and slip-critical joints using turn-of-nut with matchmarking, twist-off bolt or direct tension indicator methods	X	---			☒
3. Material verification of structural steel:					
a. For Structural steel, identification markings to conform to AISC 360.	---	X	AISC 360 Section M5.5		☐
b. Identification markings to conform to ASTM standards specified in the approved construction documents.	---	X	Applicable ASTM material standards		☐
c. Manufacturer's certified test reports	---	X			
4. Material verification of weld filler materials:					
a. Identification markings to conform to AWS standards specified in the approved construction documents.	---	X	AISC 360, Section A3.5 and applicable AWS documents	---	☐
b. Manufacturer's certificate of compliance required.	---	X	---	---	☐
5. Inspection of Welding:					
a. Structural Steel and cold-formed metal deck					
1) Completed and partial penetration groove welds.	X	---	AWS D1.1	1704.3.1	☐
2) Multipass fillet welds	X	---			☐
3) Single-pass fillet welds > 5/16"	X	---			☐
4) Plug and slot welds	X	---	AWS D1.3	---	☐
5) Single-pass fillet welds < or = 5/16"	---	X			☐
6) Floor deck welds.	---	X			☒
b. Reinforcing steel:					
1) Verification of the weldability of reinforcing steel other than ASTM A 706.	---	X	AWS D1.4 ACI 318: Section 3.5.2	---	☐
2) Reinforcing steel-resisting flexural and axial forces in intermediate and special moment frames, and boundary elements of special reinforced concrete shear walls and shear reinforcement.	X	---			☐
3) Shear reinforcement.	X	---			☐
4) Other reinforcing steel.	---	X			☐
6. Inspection of the steel frame joint details for compliance with the approved construction documents:					
a. Details such as bracing and stiffening.	---	X	---	1704.3.2	☐
b. Member locations.	---	X			☐
c. Application of joint details at each connection.	---	X			☐

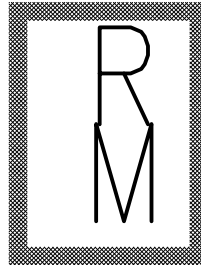
Table 1704.5.1 Level 1 Special Inspections						
Inspection Task	FREQUENCY OF INSPECTION		REFERENCE FOR CRITERIA			CHECK IF NOT REQ'D
	Continuous during task listed	Periodically during task listed	IBC SECTION	ACI 530/ASCE 5/TMS 402	ACI 530/ASCE 6/TMS 602	
1. Compliance with required inspection provisions of the construction documents and the approved submittals shall be verified	---	X	---	---	Art. 1.5	☐
2. Verification of F_{re} and F_{act} prior to construction except where specifically exempted by this code	---	X	---	---	Art. 1.4B	☐
3. Verification of slump flow and VSI as delivered to the site for self-consolidating grout	X	---	---	---	Art. 1.5B.1b.3	☐
4. As masonry construction begins, the following shall be verified to ensure compliance:						
a. Proportions of site-prepared mortar:	---	X	---	---	Art. 2.6A	☐
b. Construction of mortar joints.	---	X	---	---	Art. 3.3B	☐
c. Location of reinforcement & connectors.	---	X	---	---	Art. 3.4, 3.6A	☐
d. Prestressing technique.	---	X	---	---	Art. 3.6B	☐
e. Grade and size of prestressing tendons & anchorages.	---	X	---	---	Art. 2.4B, 2.4H	☐
5. The inspection program shall verify:						
a. Size & location of structural.	---	X	---	---	Art. 3.3G	☐
b. Type, size and location of anchors, including other details of anchorage of masonry to structural members, frames or other construction.	---	X	---	Sec. 1.2.2(e), 2.1.4, 3.1.6	---	☐
c. Specified size, grade and type of reinforcement.	---	X	---	Sec. 1.13	Art. 2.4, 3.4	☐
d. Welding of reinforcing bars.	X	---	---	Sec. 2.1.10.7.2, 3.3.3.4(b)	---	☐
e. Protection of masonry during cold weather (temperature below 40°F) or hot weather (temperature above 90°F).	---	X	---	Sec. 2104.3, 2104.4	Art. 1.8C, 1.8D	☐
f. Application and measurement of prestressing force.	---	X	---	---	Art. 3.6B	☐
6. Prior to grouting, the following shall be verified:						
a. Grout Space is clean.	---	X	---	---	Art. 3.2D	☐
b. Placement of reinforcement and connectors and prestressing tendons and anchorages.	---	X	---	Sec. 1.13	Art. 3.4	☐
c. Proportions of site-prepared grout and prestressing grout for bonded tendons.	---	X	---	---	Art. 2.6B	☐
d. Construction of mortar joints.	---	X	---	---	Art. 3.3B	☐
7. Grout placement shall be verified to ensure compliance with code and construction document provisions:	X	---	---	---	Art. 3.5	☐
a. Grouting of prestressing bonded tendons.	X	---	---	---	Art. 3.6C	☐
8. Preparation of any required grout specimens, mortar specimens and/ or prisms shall be observed.	---	X	---	Sec. 2105.2.2, 2105.3	Art. 1.4	☐
9. Compliance with required inspection provisions of the construction documents and the approved submittals shall be verified.	---	X	---	---	Art. 1.5	☐

Inspection Task	FREQUENCY OF INSPECTION		REFERENCE SECTION	IBC SECTION	CHECK IF NOT REQ'D
	Continuous during task listed	Periodically during task listed			
1. Inspection of reinforcing steel, including prestressing tendons, and placement.	---	X	ACI 318: 3.5, 7.1-7.7	1913.4	☐
2. Inspection of reinforcing steel welding in accordance with Table 1704.3, Item 5B.	---	---	AWS D1.4 ACI 318: 3.5.2	---	☐
3. Inspect bolts to be installed in concrete prior to and during placement of concrete where allowable loads have been increased.	X	---	---	1911.5, 1912.1	☐
4. Inspection of anchors installed in hardened concrete	---	X	ACI 318: 3.8.6, 8.1.3, 21.2.8	1912.1	☐
5. Verifying use of required design mix.	---	X	ACI 318: Ch. 4, 5.2-5.4	1904.2.2, 1913.2, 1913.3	☐
6. At the time fresh concrete is sampled to fabricate specimens for strength tests, perform slump and air content tests, and determine the temperature of concrete	X	---	ASTM C 172 ASTM C 311 ACI 318: 5.6, 5.8	1913.10	☐
7. Inspection of concrete and shotcrete placement for proper application techniques.	X	---	ACI 318: 5.9, 5.10	1913.6, 1913.7, 1913.8	☐
8. Inspections for maintenance if specified during temperature and techniques.	---	X	ACI 318: 5.11-5.13	1913.9	☐
9. Inspection of prestressed concrete: a. Application of prestressing forces. b. Grouting of bonded prestressing tendons in the seismic-force-resisting system.	X X X	---	ACI 318: 18.20 ACI 318: 18.18.4	---	☐
10. Erection of precast concrete members.	---	X	ACI 318: Ch. 16	---	☐
11. Verification of in-situ concrete strength, prior to stressing of tendons in posttensioned concrete and prior to removal of shores and forms from beams and structural slabs.	---	X	ACI 318: 6.2	---	☐
12. Inspect formwork for shape, location and dimensions of the concrete member being formed.	---	X	ACI 318: 6.1.1	---	☐

VERIFICATION AND INSPECTION	CONTINUOUS DURING TASK	PERIODICALLY DURING TASK	Check if NOT Req'd
1. Verify materials below footings are adequate to achieve the design bearing capacity.	-	X	☐
2. Verify excavations are extended to proper depth and have reached proper material.	-	X	☐
3. Perform classification and testing of controlled fill materials.	-	X	☐
4. Verify use of proper materials, densities and lift thickness during placement and compaction of controlled fill.	X	-	☐
5. Prior to placement of controlled fill, observe subgrade and verify site has been prepared properly.	-	X	☐

VERIFICATION AND INSPECTION	CONTINUOUS DURING TASK	PERIODICALLY DURING TASK	Check if NOT Req'd
1. Verify pile materials, sizes and lengths comply with the requirements.	X	---	☒
2. Determine capacities of test piles and conduct additional load tests, as required.	X	---	☒
3. Observe driving operations and maintain complete and accurate records for each pile.	X	---	☒
4. Verify placement locations and plumbness, confirm type and size hammer, record number of blows per foot of penetration, determine required penetrations to achieve design capacity, record tip and butt elevations and document any pile damage.	X	---	☒
5. For steel piles, perform additional inspections in accordance with Section 1704.3	---	---	☒
6. For concrete piles and concrete-filled piles, perform additional inspections in accordance with Section 1704.4	---	---	☒
7. For specialty piles, perform additional inspections as determined by the registered design professional in responsible charge.	---	---	☒

VERIFICATION AND INSPECTION	CONTINUOUS DURING TASK	PERIODICALLY DURING TASK	Check if NOT Required
1. Observe drilling operations and maintain complete and accurate records for each pier.	X	---	☒
2. Verify placement locations and plumbness, confirm pier diameters, bell diameters (if applicable), lengths, embedment into bedrock (if applicable) and adequate end bearing strata capacity.	X	---	☒
3. For concrete piers, perform additional inspections in accordance with Section 1704.4	---	---	☒
4. For masonry piers, perform additional inspections in accordance with Section 1704.5	---	---	☒



Richard Molenaar

A R C H I T E C T



PROJECT NAME

PROJECT NAME:

HOME 2
SUITES BY HILTON

W Mane Street West Monroe, Lousiana

SHEET TITLE:

SPECIAL INSPECTION SCHEDULE

SCALE: AS NOTED

PROJECT NO.

LATEST REVISION

2014052

DRAWN BY:

DRAWN BY: CHECKED BY:

SHEET NUMBER:

GP

MD

DATE: _____

5-19-14

S-103



W Mane Street West Monroe, Lousiana

SHEET TITLE:
FOUNDATION PLAN

SCALE: AS NOTED

PROJECT NO.

LATEST REVISION:

2014052

DRAWN BY: CHECK

SHEET NUMBER:

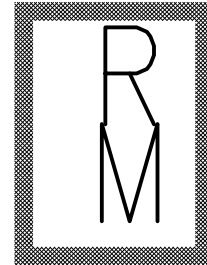
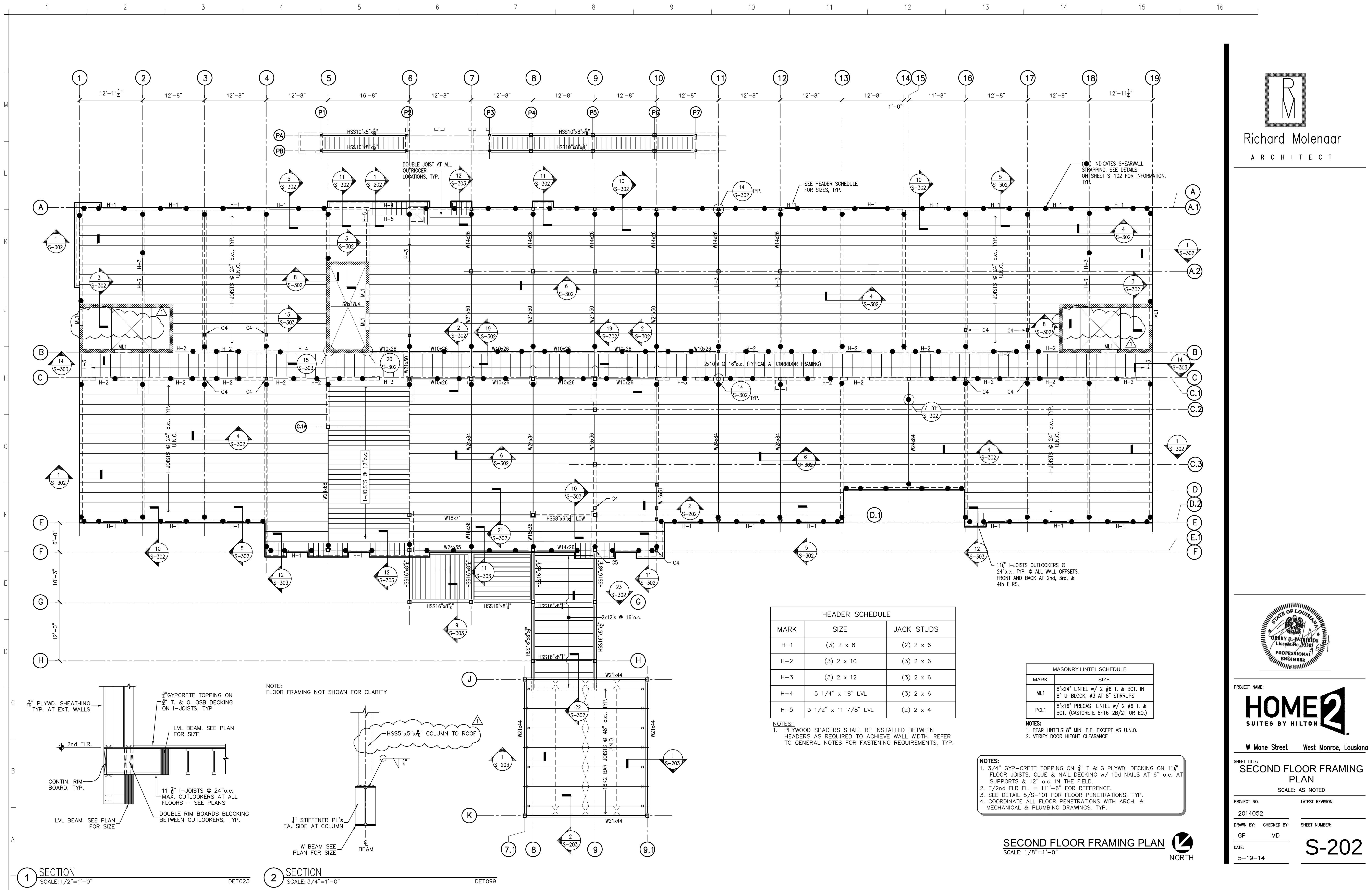
GP

DATE: _____

S-201

FOUNDATION PLAN
SCALE: 1/8"=1'-0"





Richard Molenaar
ARCHITECT



PROJECT NAME:
HOME2
SUITES BY HILTON
W Mane Street West Monroe, Louisiana

SHEET TITLE:
SECOND FLOOR FRAMING PLAN
SCALE: AS NOTED

PROJECT NO. 2014052
DRAWN BY: GP
CHECKED BY: MD
DATE: 5-19-14
SHEET NUMBER:
S-202

HEADER SCHEDULE		
MARK	SIZE	JACK STUDS
H-1	(3) 2 x 8	(2) 2 x 6
H-2	(3) 2 x 10	(3) 2 x 6
H-3	(3) 2 x 12	(3) 2 x 6
H-4	5 1/4" x 18" LVL	(3) 2 x 6
H-5	3 1/2" x 11 7/8" LVL	(2) 2 x 4

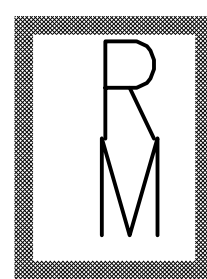
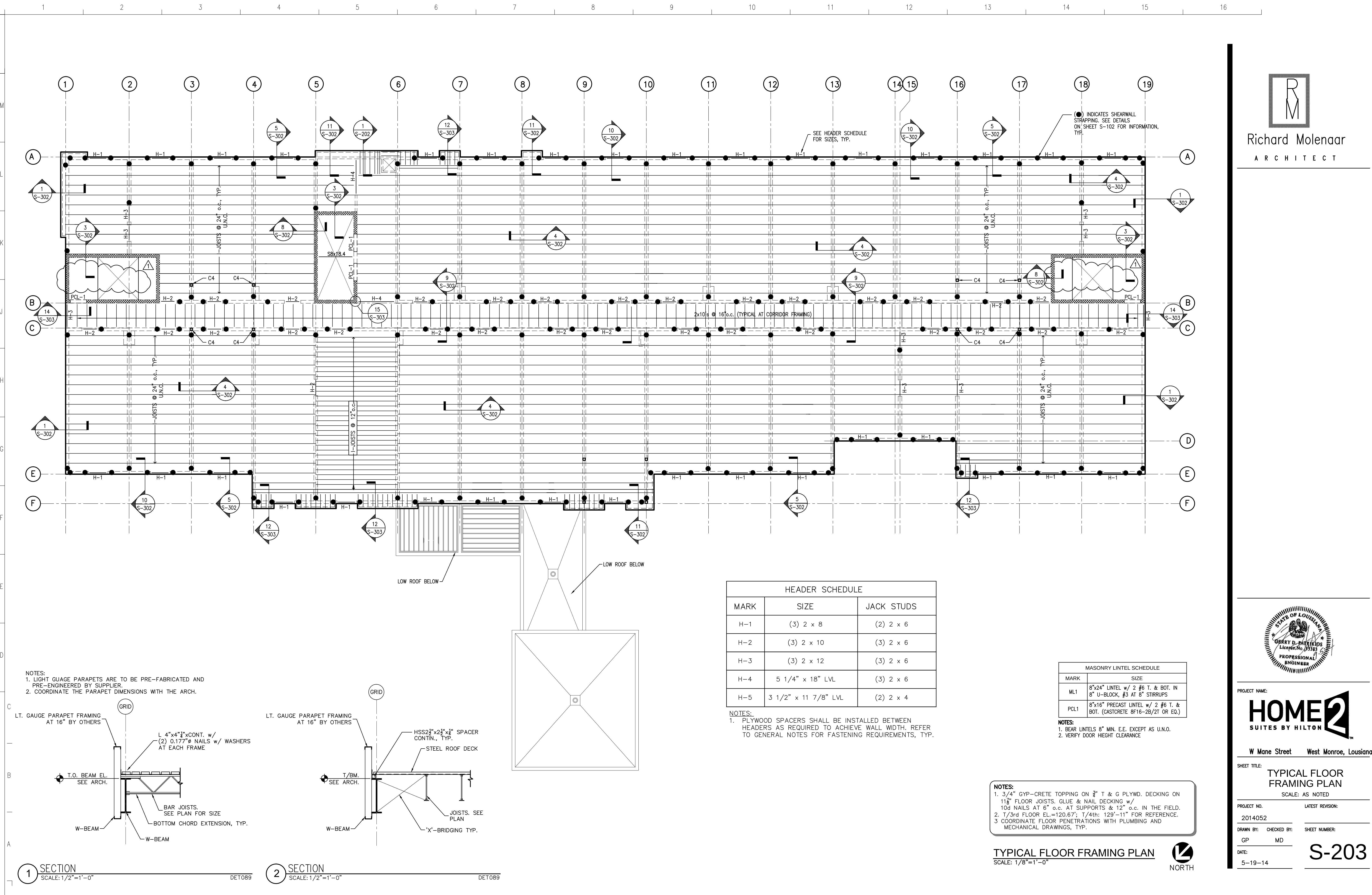
NOTES:
1. PLYWOOD SPACERS SHALL BE INSTALLED BETWEEN HEADERS AS REQUIRED TO ACHIEVE WALL WIDTH. REFER TO GENERAL NOTES FOR FASTENING REQUIREMENTS, TYP.

MASONRY LINTEL SCHEDULE	
MARK	SIZE
ML1	8"x24" LINTEL w/ 2 #6 T. & BOT. IN 8" U-BLOCK, #3 AT 8" STIRRUPS
PCL1	8"x16" PRECAST LINTEL w/ 2 #6 T. & BOT. (CASTORETE 8F16-2B/2T OR EQ.)

NOTES:
1. BEAR LINTELS 8" MIN. E.E. EXCEPT AS U.N.O.
2. VERIFY DOOR HEIGHT CLEARANCE

NOTES:
1. 3/4" GYP-CRETE TOPPING ON 3/4" T & G PLYWD. DECKING ON 11 7/8" FLOOR JOISTS. GLUE & NAIL DECKING w/ 10d NAILS AT 6" o.c. AT SUPPORTS & 12" o.c. IN THE FIELD.
2. T/2nd FLR EL. = 111'-6" FOR REFERENCE.
3. SEE DETAIL 5/S-101 FOR FLOOR PENETRATIONS, TYP.
4. COORDINATE ALL FLOOR PENETRATIONS WITH ARCH. & MECHANICAL & PLUMBING DRAWINGS, TYP.

SECOND FLOOR FRAMING PLAN
SCALE: 1/8"=1'-0"
NORTH



Richard Molenaar
ARCHITECT



PROJECT NAME:
HOME2
SUITES BY HILTON
W Mane Street West Monroe, Louisiana

SHEET TITLE:
TYPICAL FLOOR FRAMING PLAN
SCALE: AS NOTED
PROJECT NO. 2014052
LATEST REVISION:
DRAWN BY: GP
CHECKED BY: MD
SHEET NUMBER:
DATE: 5-19-14
S-203

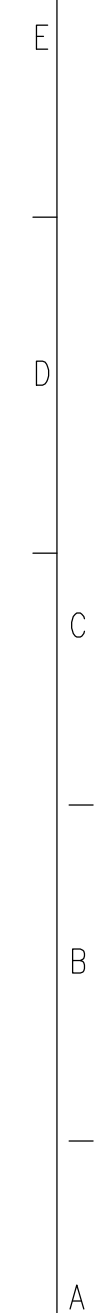
HEADER SCHEDULE		
MARK	SIZE	JACK STUDS
H-1	(3) 2 x 8	(2) 2 x 6
H-2	(3) 2 x 10	(3) 2 x 6
H-3	(3) 2 x 12	(3) 2 x 6
H-4	5 1/4" x 18" LVL	(3) 2 x 6
H-5	3 1/2" x 11 7/8" LVL	(2) 2 x 4

MASONRY LINTEL SCHEDULE	
MARK	SIZE
ML1	8"x24" LINTEL w/ 2 #6 T. & BOT. IN 8" U-BLOCK, #3 AT 8" STIRRUPS
PCL1	8"x16" PRECAST LINTEL w/ 2 #6 T. & BOT. (CASTCRETE 8F16-28/21 OR EQ.)

NOTES:
1. 3/4" GYP-CRETE TOPPING ON 3/8" T & G PLYWD. DECKING ON 11 1/8" FLOOR JOISTS. GLUE & NAIL DECKING w/ 10d NAILS AT 6" o.c. AT SUPPORTS & 12" o.c. IN THE FIELD.
2. T/3rd FLOOR EL.=120.67'; T/4th: 129'-11" FOR REFERENCE.
3. COORDINATE FLOOR PENETRATIONS WITH PLUMBING AND MECHANICAL DRAWINGS, TYP.

TYPICAL FLOOR FRAMING PLAN
SCALE: 1/8"=1'-0"





NOTES:

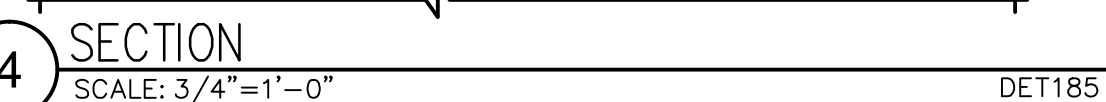
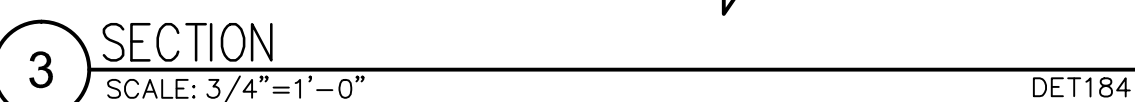
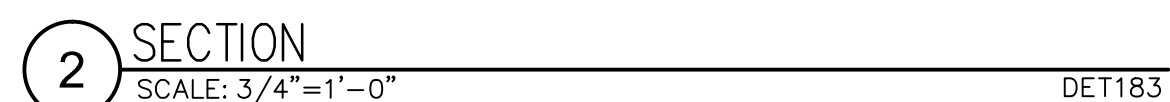
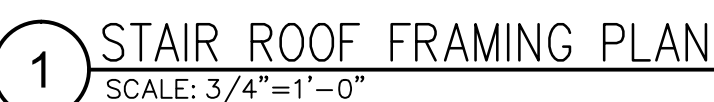
1. PLYWOOD SPACERS SHALL BE INSTALLED BETWEEN HEADERS AS REQUIRED TO ACHIEVE WALL WIDTH. REFER TO GENERAL NOTES FOR FASTENING REQUIREMENTS, TYP.

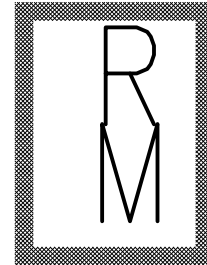
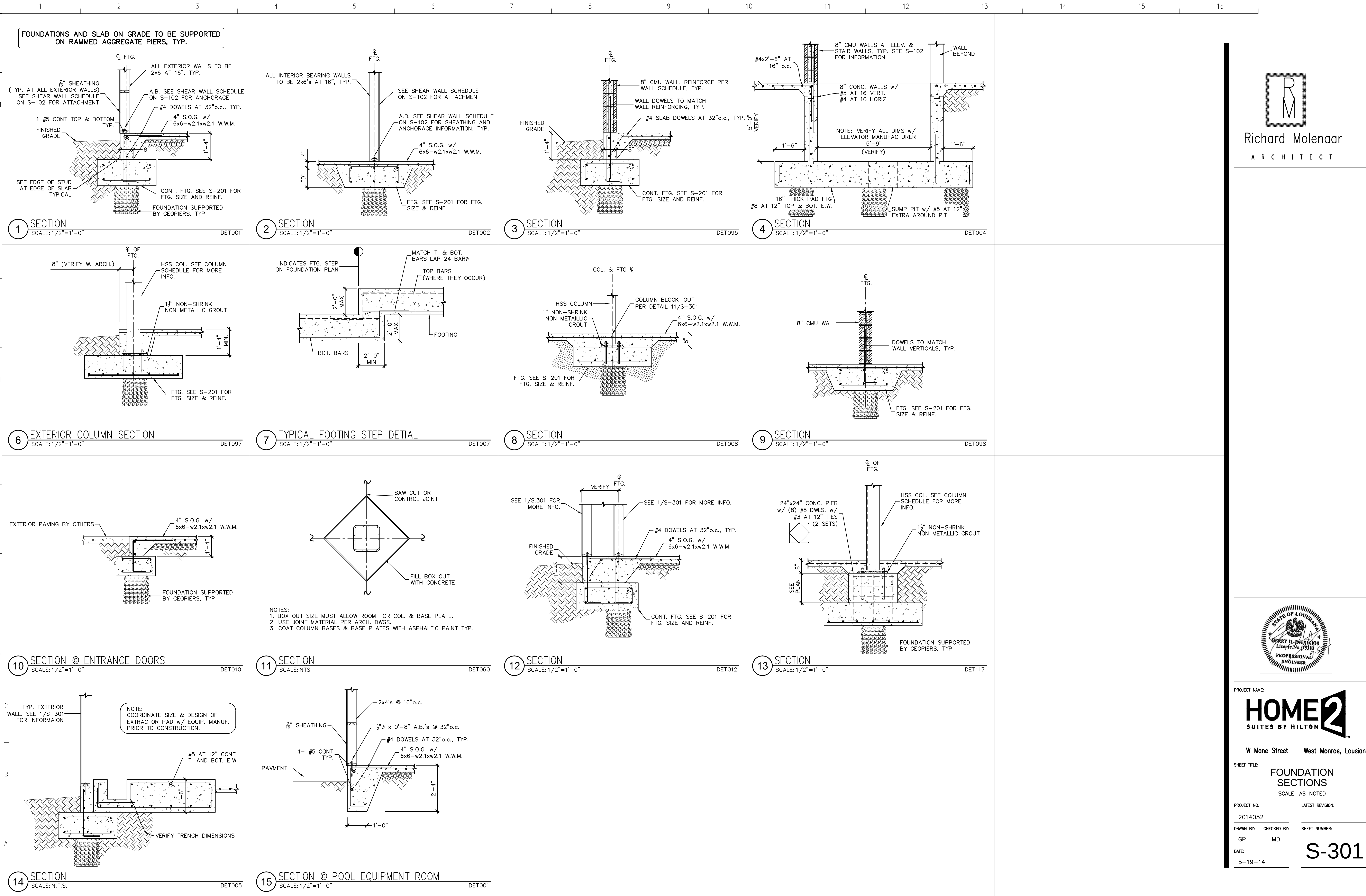


ROOF FRAMING PLAN

S-204

NORTH





Richard Molenaar
ARCHITECT

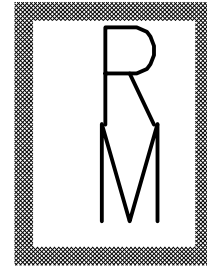
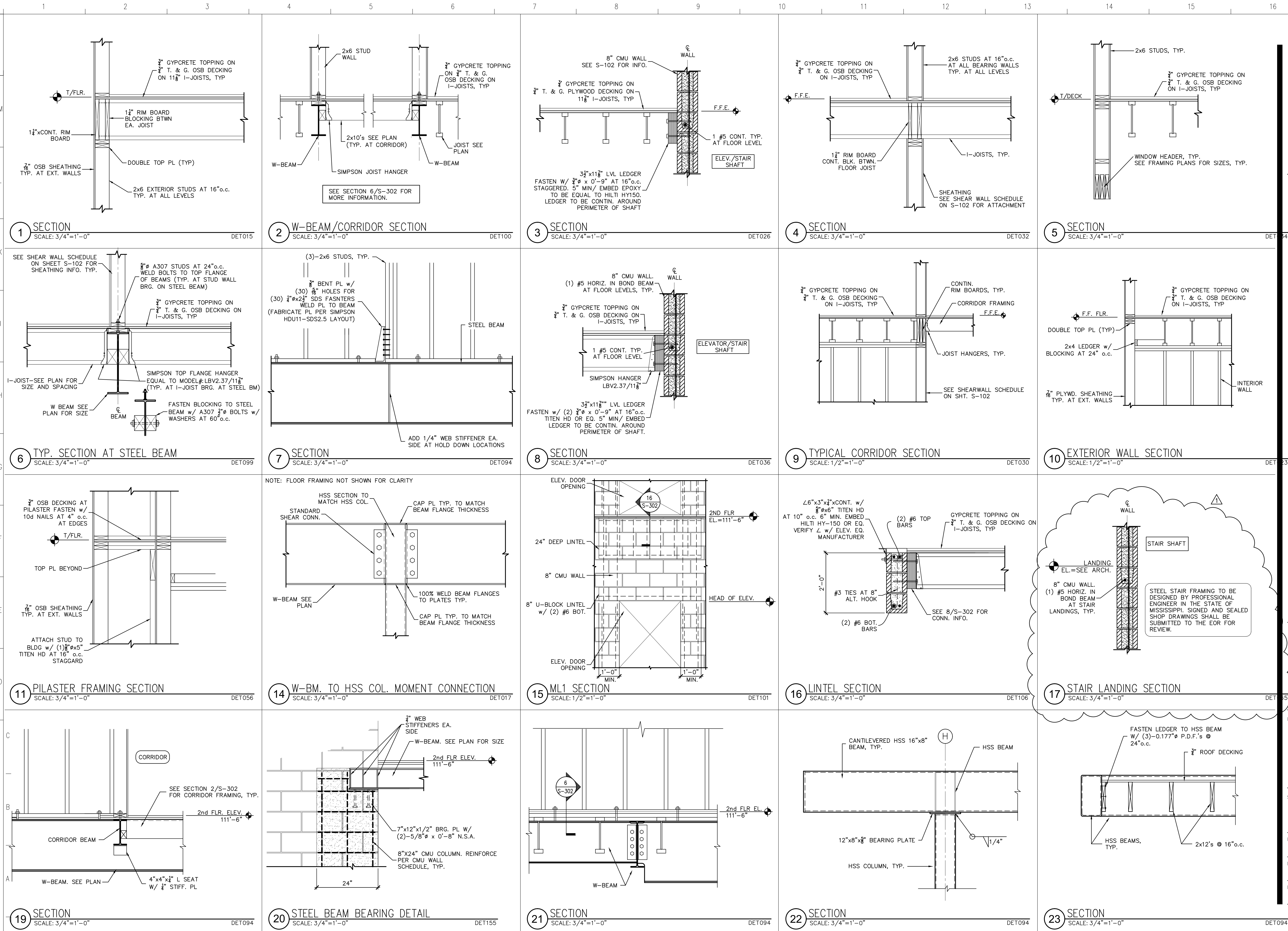


PROJECT NAME:
HOME2
SUITES BY HILTON
W Mane Street West Monroe, Louisiana

SHEET TITLE:
FOUNDATION SECTIONS
SCALE: AS NOTED

PROJECT NO. 2014052
LATEST REVISION:
DRAWN BY: GP
CHECKED BY: MD
SHEET NUMBER:
S-301

DATE: 5-19-14



Richard Molenaar
ARCHITECT

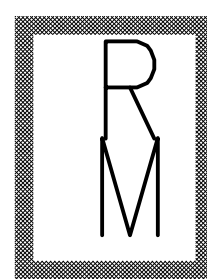
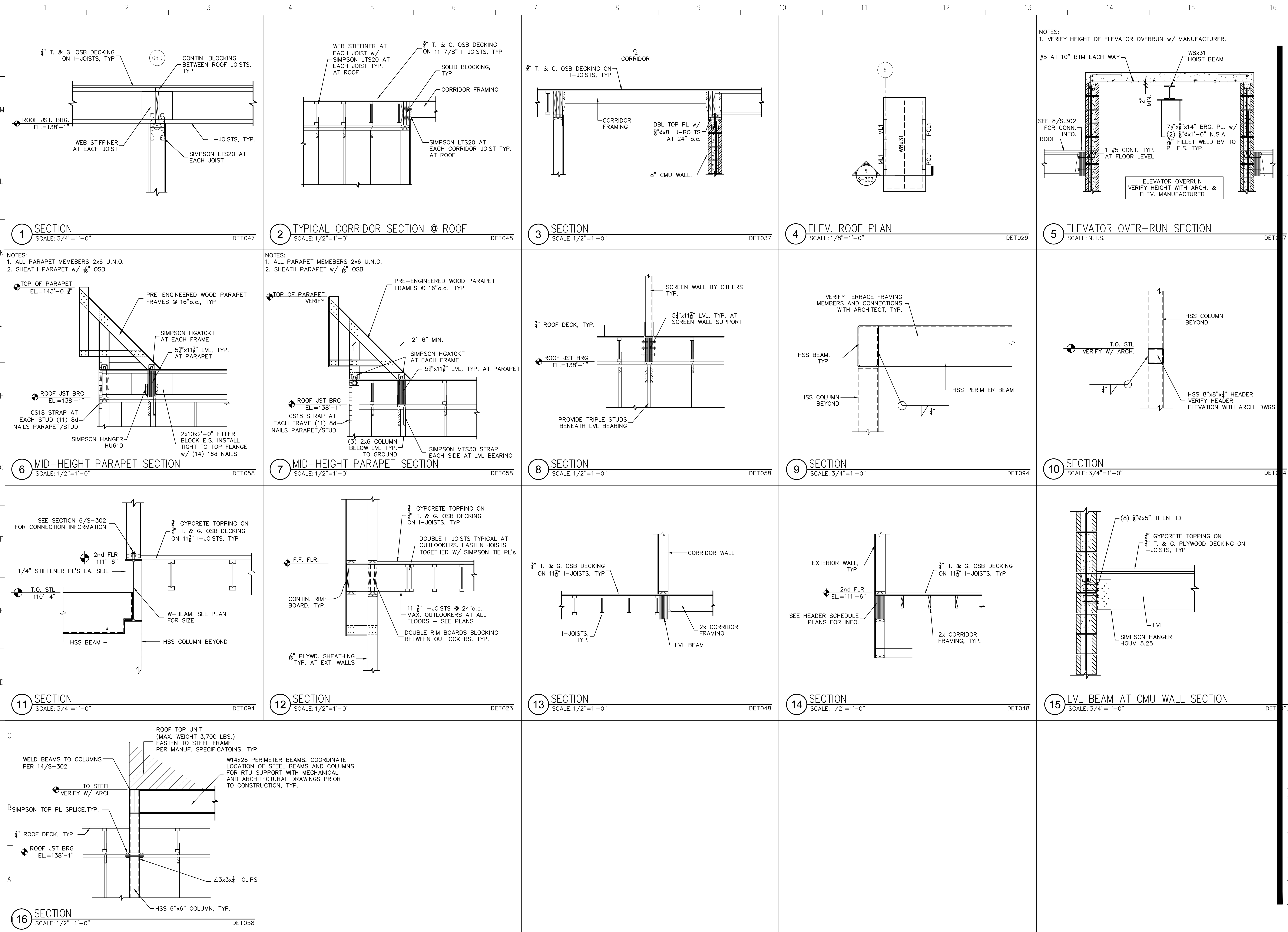


PROJECT NAME:
HOME2 SUITES BY HILTON
W Mane Street West Monroe, Louisiana

SHEET TITLE:
FRAMING SECTIONS

SCALE: AS NOTED

PROJECT NO. 2014052
LATEST REVISION:
DRAWN BY: GP
CHECKED BY: MD
SHEET NUMBER:
DATE: 5-19-14
S-302



Richard Molenaar
ARCHITECT



PROJECT NAME:
HOME2 SUITES BY HILTON
W Mane Street West Monroe, Louisiana

SHEET TITLE:
FRAMING SECTIONS

SCALE: AS NOTED

PROJECT NO. 2014052 LATEST REVISION:

DRAWN BY: GP CHECKED BY: MD SHEET NUMBER:

DATE: 5-19-14

S-303