

STRUCTURAL NOTES

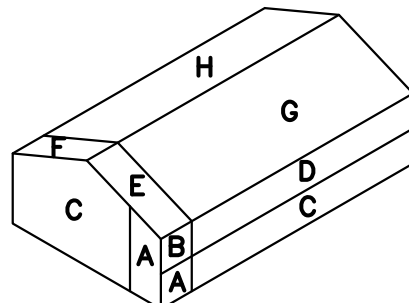
BASIS OF DESIGN

IBC 2015
ASCE 7-10

DESIGN LOADS

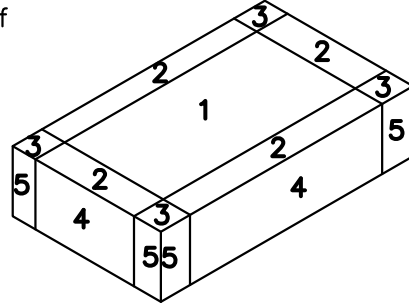
DL1. LIVE LOADS	
a.FIRST FLOOR	100 psf
b.TYPICAL FLOOR	40 psf
c.CORRIDORS SERVING ROOMS	40 psf
d.MECHANICAL ROOMS	
EQUIPMENT WEIGHT NOT LESS THAN	125 psf
DL2. ROOF	20 psf
DL3. GROUND SNOW LOAD	10 psf
a.SNOW EXPOSURE FACTOR	Ce 0.9
b.THERMAL FACTOR	Ct 1.0
c.SNOW IMPORTANCE FACTOR	I 1.0
d.FLAT ROOF SNOW LOAD	0.7*pg*Ce*Ct*I 6.3 psf

DL4. WIND LOAD	
a.WIND SPEED	90 mph
b.WIND SPEED	115 mph ULTIMATE
c.WIND IMPORTANCE FACTOR	1.00
d.RISK CATEGORY	II
e.WIND EXPOSURE	C
f.INTERNAL PRESSURE COEFFICIENT – ASD	
ZONE A	19.9 psf
ZONE B	-10.4 psf
ZONE C	13.2 psf
ZONE D	-10.0 psf
ZONE E	-24.0 psf
ZONE F	-13.7 psf
ZONE G	-16.7 psf
ZONE H	-10.6 psf



OVERHANG	Eoh -33.6 psf	Goh -26.3 psf
PARAPET LOAD	-38.2 psf	

g.COMPONENTS AND CLADDING	
ZONE 1	10.0 psf -22.7 psf
ZONE 2	10.0 psf -38.0 psf
ZONE 3	10.0 psf -57.3 psf
ZONE 4	22.7 psf -24.6 psf
ZONE 5	22.7 psf -30.4 psf
ROOF OVERHANG	ZONE 2 -32.7 psf
	ZONE 3 -55.9 psf



h.WIND BASE SHEAR	
Vxult= 105 kips	Vyult = 249 kips
Vxasd= 64 kips	Vyasd =152 kips

DL5. SEISMIC DESIGN DATA

a.SEISMIC IMPORTANCE FACTOR	1.00
b.MAPPED SPECTRAL RESPONSE ACCELERATION	Ss=0.156 S1=0.086
c.SITE CLASS	
d.SPECTRAL RESPONSE COEFFICIENTS	Sds=0.167 Sd1=0.138 C
e.SEISMIC DESIGN CATEGORY	
f.BASIC SEISMIC FORCE RESISTING SYSTEM	
LIGHT FRAMED WALLS SHEATHED WITH WOOD	
g.DESIGN BASE SHEAR	42 KIPS
h.RESPONSE MODIFICATION FACTORS	R=6.5
i. SEISMIC RESPONSE COEFFICIENT	Cd=4.0
j. ANALYSIS PROCEDURE -- EQUIVALENT FORCE METHOD	

DL6. DESIGN DEAD LOADS

a.FLOOR DEAD LOAD	
WOOD FRAMING	= 5 psf
3/4" T&G PLYWOOD	= 2 psf
1 1/2" GYPSUM TOPPING	= 14 psf
5/8" DRYWALL CEILING	= 2 psf
WALLS	= 8 psf
MECHANICAL DUCTS & PIPING	= 4 psf
TOTAL DEAD LOAD	= 35 psf

b.ROOF DEAD LOAD	
WOOD FRAMING	= 5 psf
5/8" PLYWOOD	= 2 psf
5/8" DRYWALL CEILING	= 2 psf
MECHANICAL DUCTS & LIGHTING	= 4 psf
6" INSULATION AVERAGE	= 2 psf
ROOFING	= 10 psf
TOTAL DEAD LOAD	= 25 psf

FOUNDATIONS

FD1. SOIL BEARING PRESSURE	2,100 psf WALL FOOTINGS 2,400 psf COLUMN FOOTINGS
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GEOTECHNICAL INVESTIGATION REPORT PREPARED BY:
GEOTECHNICAL ASSOCIATES NETWORK, LLC
110 BEECHTREE ROAD
VICKSBURG, MISSISSIPPI 39183-7464

FD2. THE CONTRACTOR SHALL REFER TO THE GEOTECHNICAL INVESTIGATION REPORT FOR RECOMMENDATIONS FOR SUB-GRADE BUILDING PAD AND FOUNDATION BEARING. CONTRACTOR SHALL FOLLOW REPORTS RECOMMENDATIONS FOR BUILDING PAD AND OPEN FOUNDATION PROTECTION FROM WEATHER. EXTERIOR AND PERIMETER BUILDING FOUNDATION SHALL NOT BEAR ABOVE THE LOCAL FROST LINE. REFER TO ARCHITECTURAL DRAWINGS FOR PERIMETER SLAB/FOUNDATION INSULATION REQUIREMENTS.

FD3. BACK FILL

CONTRACTOR SHALL TAKE ALL NECESSARY PRECAUTIONS TO BRACE WALLS DURING BACKFILLING. CARE SHALL BE TAKEN DURING PLACEMENT OF BACKFILL ALONG THE WALL SO AS TO NOT OVERLOAD THE WALL DUE TO HEAVY EQUIPMENT. ONLY LIGHTWEIGHT (A MAXIMUM OF ONE TON TOTAL WEIGHT) EQUIPMENT SHALL BE PERMITTED WITHIN THE CRITICAL ZONE DEFINED AS BEGINNING AT THE BASE OF THE WALL ON A 1:1 SLOPE.

STRUCTURAL CONCRETE

CO1. CONCRETE SHALL BE IN ACCORDANCE WITH ACI MANUAL 315 AND STANDARD 318. CONCRETE SHALL BE OF REGULAR AGGREGATE AND SHALL HAVE DESIGN COMPRESSIVE STRESS AT 28 DAYS AS FOLLOWS:

a.f'c = 3,000 psi FOR FOUNDATIONS AND FOUNDATION WALLS.
b.f'c = 3,000 psi FOR SLAB ON GRADE
c.f'c = 3,500 psi AIR ENTRAINED FOR EXTERIOR CONCRETE
d.f'c = 4,000 psi FOR ELEVATED SLABS

CO2. PROVIDE AIR ENTRAINED CONCRETE FOR CONCRETE EXPOSED TO WEATHER.

CO3. SLAB ON GRADE

a.PROVIDE A 5" CONCRETE SLAB WITH #4@12" o.c. EACH WAY OVER 8 MIL POLY VAPOR RETARDER.

CO4. REFER TO ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS FOR ALL MOLDS, GROOVES, ETC. AND FOR LOCATIONS OF SLEEVED AND INSERTS TO BE CAST IN CONCRETE SLABS AND FLOORS.

CO5. FOR SIZE, NUMBER AND LOCATIONS OF ALL SLAB OPENINGS AND MECHANICAL HOUSEKEEPING PADS SEE ARCHITECTURAL AND MECHANICAL DRAWINGS. PROVIDE MECHANICAL HOUSEKEEPING PADS AS REQUIRED AND REINFORCE WITH #4@12" EACH WAY U.N.O. DOWEL PADS INTO SUPPORTING SLAB.

CO6. REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60. DETAILING SHALL BE IN ACCORDANCE WITH ACI MANUAL 315 AND STANDARD 318. LAP SPLICES IN CONCRETE SHALL BE IN ACCORDANCE WITH CHAPTER 12 OF ACI 318-08. REINFORCING STEEL SHALL HAVE A MINIMUM CONCRETE COVER AS TABULATED BELOW UNLESS OTHERWISE NOTED:

a.WALLS	1 1/2"
b.FOUNDATIONS2" FOR FORMED CONCRETE	
c.3" WHERE CONCRETE IS CAST AGAINST GROUND	

CO7. MINIMUM SPLICE & EMBEDMENT LENGTHS

BAR SIZE	MIN EMBEDMENT, (IN)		STD HOOK ALL BARS
	MIN LAP (IN)	STRAIGHT OTHER--TOP	
#3	12 - 14	12 - 14	6
#4	15 - 19	15 - 19	7
#5	18 - 24	18 - 23	9
#6	22 - 28	22 - 28	10
#7	25 - 33	25 - 33	12
#8	29 - 37	29 - 37	14

MASONRY

MA1. REINFORCED MASONRY SHALL BE IN ACCORDANCE WITH THE SPECIFICATIONS FOR DESIGN AND CONSTRUCTION FOR LOAD BEARING CONCRETE MASONRY PUBLISHED BY THE NATIONAL CONCRETE MASONRY ASSOCIATION.

MA2. REINFORCED BRICK LINTELS SHALL BE IN ACCORDANCE WITH THE TECHNICAL NOTES 17TH PUBLISHED BY THE BRICK INSTITUTE OF AMERICA, AND "BUILDING CODE REQUIREMENTS FOR ENGINEERED BRICK MASONRY", BY STRUCTURAL CLAY PRODUCTS INSTITUTE.

MA3. ALL CONCRETE HOLLOW BLOCK UNITS SHALL BE NORMAL WEIGHT 2 CELL UNITS AND CONFORM TO ASTM C-90. ALL UNITS SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF f'm = 1,350 psi.

MA4. COURSE GROUT SHALL CONFORM TO THE REQUIREMENTS OF THE CONTRACT SPECIFICATIONS AND SHALL DEVELOP A MINIMUM COMPRESSIVE STRESS OF 3000 psi AT 28 DAYS.

MA5. MORTAR SHALL BE TYPE M OR TYPE S. MORTAR SHALL NOT BE USED FOR GROUT FILL.

MA6. ALL CONCRETE HOLLOW BLOCK UNITS SHALL BE LAID IN A RUNNING BOND PATTERN.

MA7. LAP ALL REINFORCING BARS IN MASONRY 40 BAR DIAMETERS AT SPLICES.

MA8. HORIZONTAL REINFORCING BARS SHALL BE IN BOND BEAM BLOCKS AT FLOORS AND ROOF. WHERE HORIZONTAL REINFORCING BARS ARE REQUIRED BETWEEN FLOORS, BARS SHALL BE IN INTERMEDIATE (OPEN BOTTOM) BOND UNITS.

MA9. HORIZONTAL JOINT REINFORCING SHALL BE CONTINUOUS AROUND ALL CORNERS AND INTERSECTIONS AND SHALL BE LAPPED 8" AT SPLICES. HORIZONTAL JOINT REINFORCING SHALL BE TRUSS TYPE NO. 9 WIRE, GALVANIZED AND SPACED AT 16" o.c.

MA10. FILL ALL CMU AND CAVITY BETWEEN INTERIOR AND EXTERIOR WYTHE BELOW FINISH FLOOR WITH GROUT OR MORTAR.

MA11. PROVIDE BOND BEAMS AT TOPS OF ALL WALLS WITH 2 #5 CONT. AND WHERE WALLS ARE TO UNDERSIDE OF ROOF, GROUT TIGHT TO ROOF.

MA12. FOR WALLS GREATER THAN 12'-0" HIGH PROVIDE BOND BEAMS AT 8'-0" ON CENTER WITH 2 #5 CONT.

STRUCTURAL STEEL

ST1. STRUCTURAL STEEL SHALL BE IN ACCORDANCE AISC 360-10:

a. ALL STRUCTURAL	A992
b. MISC STEEL, CHANNELS, ANGLES, PLATES	A36
c. STEEL TUBING	ASTM A500 GRADE B
d. SHOP AND FIELD WELDS	E70XX ELECTRODES
e. BOLTS (3/4" DIAMETER MIN)	A325 HIGH STRENGTH
f. ANCHOR BOLTS	F1554 GR. 36
g. COLUMN BASE PLATES	3/4" BENT UNO
5" PROJECTION + 9" EMBED + 3" BEND	
h. WOOD SILL PLATES LOAD BEARING & SHEARWALLS	5/8" BENT UNO
5" PROJECTION + 7" EMBED + 3" BEND	
WITH 1/4"x3"x3" PLATE WASHER	
i. NON-LOAD BEARING	5/8" BENT UNO
5" PROJECTION + 7" EMBED + 3" BEND	
WITH STANDARD WASHER	

ST2. CONTRACTOR MAY USE EXPANSION BOLTS OR EPOXY BOLTS IN LIEU OF ANCHOR BOLTS FOR WOOD SILL PLATES. USE 5/8" EXPANSION BOLTS w/ 4" EMBED w/ 1/4"x3"x3" PLATE WASHER AT SAME SPACING.

ST3. CONTRACTOR MAY EPOXY ALL THREAD BOLTS FOR COLUMN BASE PLATE ANCHOR BOLTS. EMBEDMENT TO BE 9".

ST4. EPOXY SHALL BE HILTI HY-200, SIMPSON STRONG TIE ET--HP OR SIMPSON STRONG TIE SET OR PRE-APPROVED EQUAL.

ST5. GROUT UNDER BASE AND BEARING PLATES SHALL BE NON-SHRINK, NON-METALLIC.

ST6. ANCHOR BOLTS, BASE PLATES AND COLUMNS SHALL BE PROTECTED FROM DIRECT CONTRACT w/ THE GROUND. COAT HEAVILY AND VOID FREE WITH ASPHALTIC MASTIC.

ST7. ALL WELDING SHALL BE IN ACCORDANCE WITH THE AMERICAN WELDING SOCIETY (AWS) BY CERTIFIED WELDERS.

ST8. HOLES SHALL NOT BE CUT THROUGH BEAMS UNLESS INDICATED OR APPROVED BY THE ENGINEER.

ST9. PRIME STRUCTURAL AND MISC. STEEL WITH MANUFACTURES STANDARD IRON OXIDE PRIMER -- COLOR GRAY. PRIMER SHALL BE COMPATIBLE WITH FINISH COAT OF PAINT WHEN PROVIDED.

ST10.STRUCTURAL STEEL DETAILING, FABRICATION, AND ERECTION SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE AISC "MANUAL OF STEEL CONSTRUCTION".

ST11.HSS TUBES SHALL HAVE ONE-QUARTER INCH END PLATES.

ST12.FIELD TORCH CUTTING OF COLUMN BASE PLATES FOR MISS LOCATED. ANCHOR BOLTS IS NOT ALLOWED. HOLES CAN BE DRILLED IN THE PROPER LOCATIONS OR NEW STRAIGHT ANCHOR BOLTS CAN BE DRILLED AND EPOXIED INTO PLACE SEE NOTE ST3 ABOVE.

ST13.WHERE ANCHOR BOLT EXTENSION IS INSUFFICIENT TO FULLY ENGAGE THE BOLT THREADS THE ANCHOR BOLT MAY BE WELDED TO THE BASE PLATE WITH 1/4" WELD ALL ROUND.

WOOD FRAMING

WD1. WOOD CONSTRUCTION SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION.

WD2. STRUCTURAL LUMBER SHALL BE HEM FIR, SPRUCE PINE IFIR OR DOUGLAS FIR NO.2 OR PRE-APPROVED EQUAL. FLOOR SHEATHING TO BE GLUED (PL400) AND FASTENED TO FRAMING LUMBER. FINGER JOINTED MEMBERS ARE ALLOWED IF MATERIAL PROPERTIES ARE EQUAL OR BETTER.

WD3. HOLES IN FRAMING (JOISTS AND WALL STUDS) SHALL NOT EXCEED 1/3 TO DEPTH OF THE MATERIAL AND LOCATED IN THE MIDDLE THIRD. HOLES SHALL BE AT LEAST 2" APART. NOTCHES IN FRAMING LUMBER SHALL NOT EXCEED 1/8 OF THE DEPTH NOR LONGER THAN 1/3 OF THE DEPTH AND SHALL NEVER BE LOCATED IN THE MIDDLE THIRD OF FRAMING MEMBERS. NOTCHES AT THE END OF FRAMING MEMBERS SHALL NOT EXCEED 1/4 THE DEPTH.

WD4. FRAMING LUMBER FRAMING SHALL BEAR A MINIMUM OF 1 1/2" AND HAVE SOLID BLOCKING BETWEEN FRAMING.

WD5. PROVIDE SOLID BLOCKING IN FLOOR AND ROOF FRAMING AS REQUIRED BY CODE.

WD6. ALL LUMBER IN CONTACT WITH MASONRY OR CONCRETE SHALL BE PRESSURE TREATED.

WD7. ALL METAL CONNECTORS, TIES, AND STRAPS IN CONTACT WITH CONCRETE, MASONRY OR TREATED LUMBER SHALL BE G185 HOT DIPPED GALVANIZED.

WD8. PROVIDE SOLID HORIZONTAL BLOCKING AT SHEATHING JOINTS IN EXTERIOR WALLS.

WD9. PROVIDE SOLID HORIZONTAL BLOCKING FOR ALL FIRST FLOOR BEARING AND EXTERIOR WALLS (1st TO ROOF) AT 48" o.c.

WOOD I-JOIST FRAMING

W1. ENGINEERED WOOD PRODUCTS (WOOD I-JOISTS & LAMINATED VENEER LUMBER -- LVL SHOWN ON THE DRAWINGS ARE MANUFACTURED BY AN APPROVE MANUFACTURER FOR THE DESIGN LOADS INDICATED, WHETHER SHOWN OR NOT, PROVIDE ACCESSORY ITEMS (BLOCKS, CLIPS, STIFFENERS, STRAPS, ETC.) DESIGNED BY THE MANUFACTURER, FOR A COMPLETE SYSTEM. REFER TO ARCHITECTURAL DRAWINGS FOR FIRE RATING REQUIREMENTS AND PROVIDE I-JOISTS ACCORDINGLY. PROVIDE I-JOISTS THAT COMPLY WITH UL570 FOR MINIMUM CHORD AND WEB SIZES FOR RATED ASSEMBLY. FOLLOW ALL MANUFACTURER'S RECOMMENDATIONS FOR INSTALLATION AND USE.

W2. FRAMING CONNECTORS, ANCHORS, AND HANGERS SHOWN ON THE DRAWINGS ARE PRODUCTS OF SIMPSON STRONG-TIE AND ARE DESIGNATED BY MANUFACTURER'S STANDARD PRODUCT NUMBERS. FOLLOW ALL MANUFACTURER'S RECOMMENDATIONS FOR INSTALLATION AND USE.

W3. CUTTING I-JOISTS ABOVE DEMISING WALLS OR CORRIDOR WALLS AND WHERE JOISTS ARE IN NON-BENDING INSTALLATIONS IS PERMITTED PROVIDED THE DRAFT STOPPING IF REQUIRED IS RESTORED.

W4. FLOOR SHEATHING TO BE GLUED (PL400) AND FASTENED TO I-JOIST.

W5. DEFLECTION CRITERIA L/480 LIVE LOAD, L360 TOTAL LOAD.

W6. MANDATORY PRE-CONSTRUCTION MEETING -- PRIOR TO I-JOIST INSTALLATION THE GENERAL CONTRACTOR SHALL HOLD A PRE-CONSTRUCTION MEETING WITH ALL TRADES TO COORDINATE PLACEMENT OF I-JOISTS TO AVOID INTERFERENCES AND LIMITATIONS ON CUTTING HOLES IN JOISTS WEBS. MINUTES OF THE MEETING SHALL BE SUBMITTED TO THE EOR.

WOOD TRUSS FRAMING

WT1. WOOD TRUSS FRAMING SHALL BE DESIGNED IN ACCORDANCE WITH TPI DESIGN SPECIFICATIONS FOR METAL PLATE CONNECTED WOOD TRUSSES.

WT2. LIMIT TOTAL LOAD DEFLECTION TO L/360.

WT3. FLOOR SHEATHING TO BE GLUED (PL400) AND FASTENED TO FLOOR TRUSSES.

WT4. MANDATORY PRE-CONSTRUCTION MEETING -- PRIOR TO TRUSS INSTALLATION THE GENERAL CONTRACTOR SHALL HOLD A PRE-CONSTRUCTION MEETING WITH ALL TRADES TO COORDINATE PLACEMENT OF TRUSSES TO AVOID INTERFERENCES. MINUTES OF THE MEETING SHALL BE SUBMITTED TO THE EOR.

GENERAL

GN1. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT THE JOBSITE WITH ARCHITECTURAL AND OTHER TRADE DRAWINGS.

GN2. UNLESS OTHERWISE SHOWN, ALL TYPICAL DETAILS (WHERE APPLICABLE) SHALL BE USED.

GN3. THIS PROJECT HAS BEEN DESIGNED FOR THE WEIGHTS OF THE MATERIALS INDICATED ON THE DRAWINGS FOR THE LIVE LOADS INDICATED IN THE DESIGN LOADS. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ALLOWABLE CONSTRUCTION LOADS AND TO PROVIDE PROPER DESIGN AND CONSTRUCTION OF FALSEWORK, FORMWORK, STAGING, BRACING, SHEETING AND SHORING, ETC

GN4. WATERPROOFING, FLASHING, CAULKING AND FIREPROOFING REQUIREMENTS ARE NOT THE RESPONSIBILITY OF THESE STRUCTURAL DRAWINGS. ANY REFERENCE OR NOTES RELATED TO THESE MATERIALS ARE FOR INFORMATION ONLY AND THE GENERAL CONTRACTOR SHALL REFER TO OTHER PLANS AND SPECIFICATIONS FOR THESE MATERIALS.

SUBMITTALS

SB1. SUBMITTALS SHALL BE PROVIDED THROUGH THE ARCHITECT IN ACCORDANCE WITH THE GENERAL CONDITIONS.

SB2. GENERAL CONTRACTOR TO SCHEDULE SUBMITTALS TO ALLOW TIME FOR REVIEW WITHOUT IMPEDING CONSTRUCTION. SCHEDULE A MINIMUM OF FOURTEEN CALENDAR DAYS FOR THE RETURN OF SUBMITTALS.

SB3. SHOP DRAWINGS ARE THE GENERAL CONTRACTORS MEANS AND METHODS OF PROVIDING WHAT IS INDICATED ON THESE STRUCTURAL DRAWINGS THEREFORE ARE NOT SUBJECT TO APPROVAL BY THE EOR. THE SHOP DRAWINGS LISTED BELOW ARE SUBJECT TO REVIEW BY THE EOR.

SB4. REVIEW IS ONLY FOR GENERAL CONFORMANCE WITH THE DESIGN CONCEPT AND THE INFORMATION GIVEN IN THE CONSTRUCTION DOCUMENTS. CORRECTIONS OR COMMENTS MADE ON THE SHOP DRAWINGS DURING THE REVIEW DO NOT RELIEVE THE GENERAL CONTRACTOR FROM COMPLIANCE WITH THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. REVIEW OF A SPECIFIC ITEM SHALL NOT INCLUDE REVIEW OF AN ASSEMBLY OF WHICH THE ITEM IS A COMPONENT. THE CONTRACTOR IS RESPONSIBLE FOR DIMENSIONS TO BE CONFIRMED AND CORRELATED AT THE JOBSITE; INFORMATION THAT PERTAINS SOLELY TO THE FABRICATION PROCESSES AND PROCEDURES OF THE CONSTRUCTION; COORDINATION OF THE WORK WITH THAT OF ALL OTHER TRADES AND PERFORMING ALL WORK IN A SAFE AND SATISFACTORY MANNER.

SB5. REVIEW OF SHOP DRAWINGS DOES NOT WARRANT OR REPRESENT THAT THE INFORMATION WITHIN THE SUBMITTAL IS EITHER ACCURATE OR COMPLETE. SOLE RESPONSIBILITY FOR CORRECT DESIGN, DETAILS, DIMENSIONS AND QUANTITIES SHALL REMAIN WITH THE GENERAL CONTRACTOR.

SB6. CHANGES/SUBSTITUTIONS SHOWN ON SHOP DRAWINGS AND SUBMITTED TO AND REVIEWED BY THE EOR REMAIN NON-COMPLIANT WITH THE CONTRACT DOCUMENTS. CHANGES/SUBSTITUTIONS TO THE CONTRACT DOCUMENTS MUST BE DOCUMENTED SEPARATELY FROM SHOP DRAWINGS.

SB7. REQUIRED SUBMITTALS -- (SUBMITTALS OTHER THAN THOSE LISTED SHALL NOT BE REVIEWED):

SB8. SUBMIT FOR REVIEW:

- CONCRETE DESIGN MIX -- EACH TYPE
- MORTAR DESIGN MIX -- EACH TYPE
- GROUT MIX -- COURSE AND FINE
- STRUCTURAL STEEL ERECTION DRAWINGS
- METAL DECK LAYOUT PLANS
- WOOD TRUSS

- TRUSS LAYOUT INDICATING DESIGN LOADS
- STATEMENT OF DEFLECTION CRITERIA COMPLIANCE
- STATEMENT THAT TRUSS LAYOUT HAS BEEN COORDINATED WITH PLUMBING AND HVAC FLOOR/ROOF PENETRATIONS

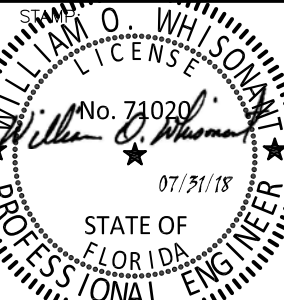
- JOIST LAYOUT INDICATING DESIGN LOADS
- STATEMENT OF DEFLECTION CRITERIA COMPLIANCE
- STATEMENT THAT I-JOIST LAYOUT HAS BEEN COORDINATED WITH PLUMBING AND HVAC FLOOR/ROOF PENETRATIONS

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WES WHISONANT
ENGINEERING
SERVICES, LLC
122 Nut Tree Court
Lexington, South Carolina 29072
(803) 957-4008
Email: bill@weslex.com

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- ☐ PRELIMINARY ONLY
☐ BIDDING/PRICING
☒ PERMIT
☒ CONSTRUCTION
DATE : 07-31-18

DRAWING TITLE:

STRUCTURAL NOTES

SCALE:

PROJECT NO: 057N2C

DATE: 07-31-18

DRAWN BY: HVS

CHECKED BY: WCVW

SHEET NO:

S001